

Stonestreet Green Solar
Environmental Statement
Volume 4: Appendices
Chapter 10: Water Environment
Appendix 10.2: Flood Risk Assessment Part 2 of 3

PINS Ref: EN010135

Doc Ref. 5.4(A)

Version 2

Deadline 1

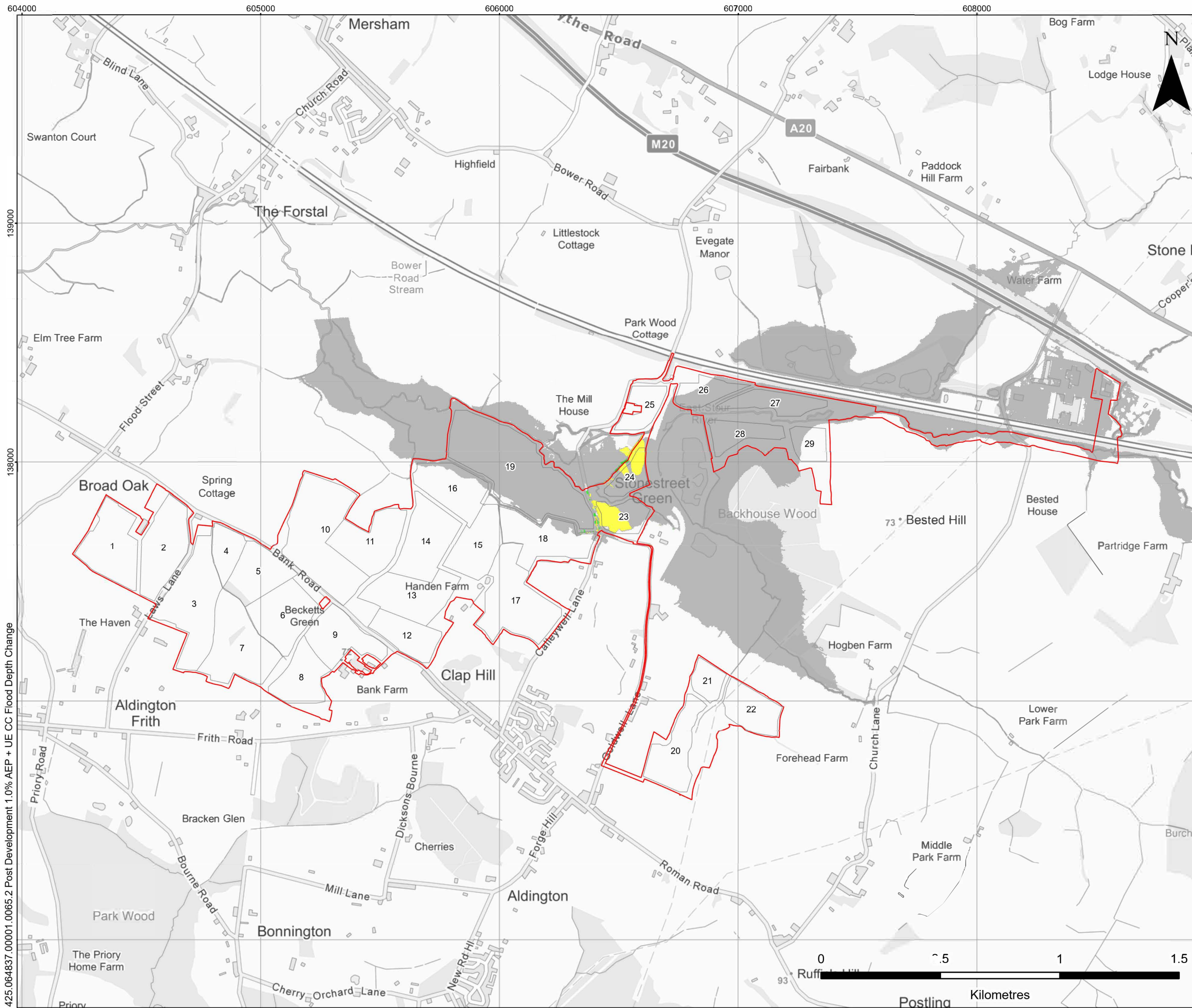
December 2024

APFP Regulation 5(2)(a)

Planning Act 2008

The Infrastructure Planning (Applications: Prescribed Forms and Procedure) Regulations 2009





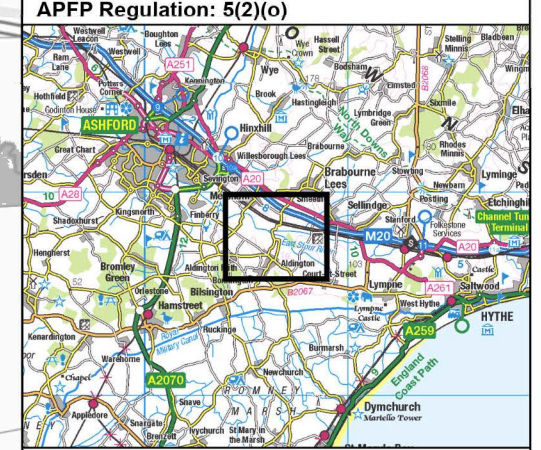
LEGEND

- Order limits
- Field

Depth Change (mm)

- <= -40
- 20 to -40
- 10 to -20
- 10 to -5
- 5 to -1
- 1 to 1
- 1 to 5
- 5 to 10
- 10 to 20
- 20 to 40
- >= 40

*Baseline vs Post Development Defended 1:100 - 36.25 hr Rainfall Event - 2 m Grid



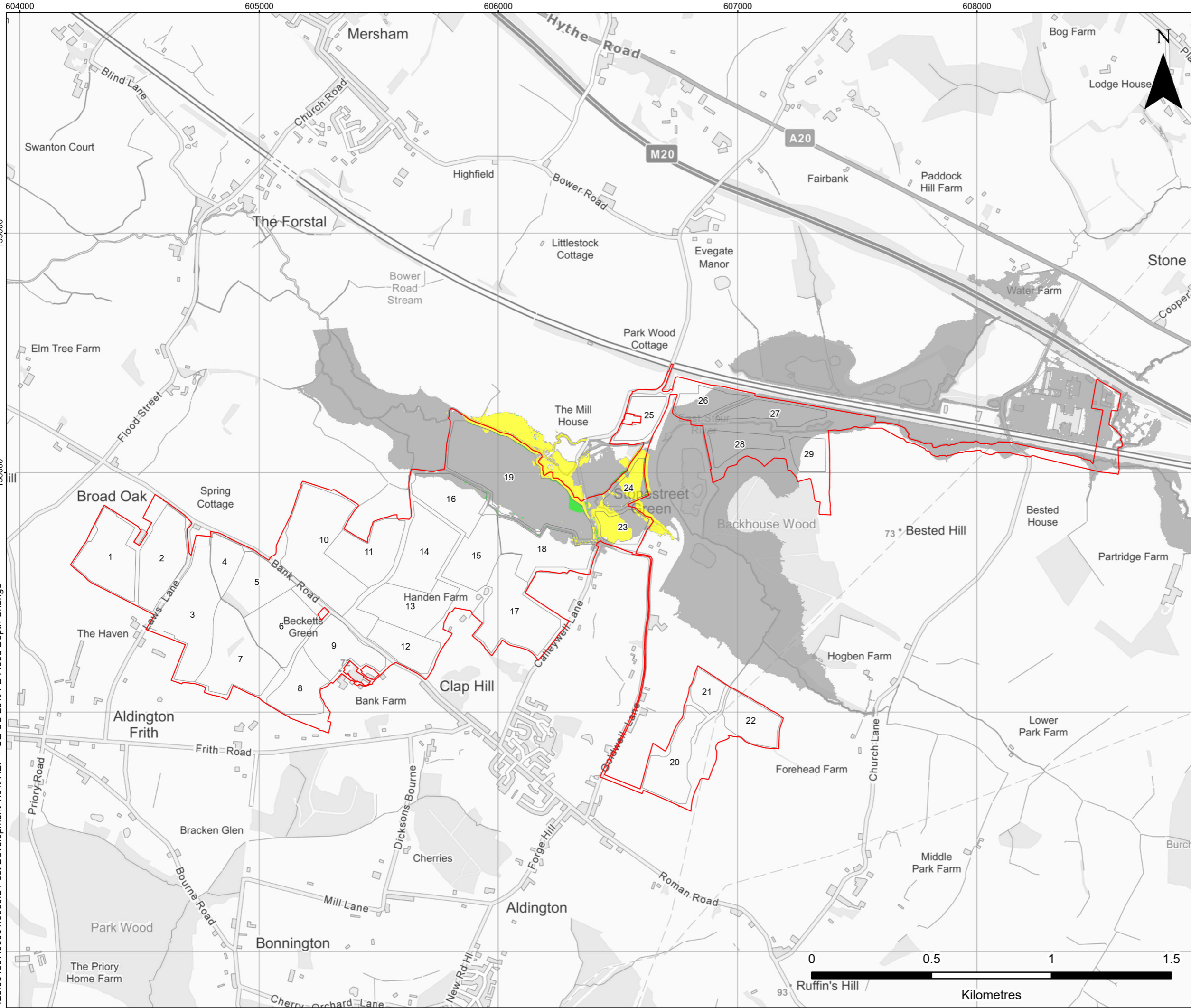
**STONESTREET GREEN SOLAR
HYDRAULIC MODELLING REPORT**

POST DEVELOPMENT 1.0% ANNUAL EXCEEDANCE PROBABILITY AND UPPER END CLIMATE CHANGE FLOOD DEPTHS

HMR Figure 24

Scale: 1:15,000 @ A3 Date: MAY 2024

425.064837.00001.0065.2 Post Development 1.0% AEP + UE CC Flood Depth Change



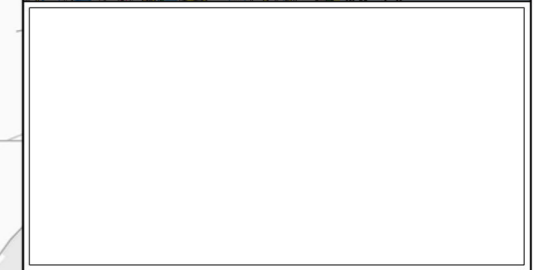
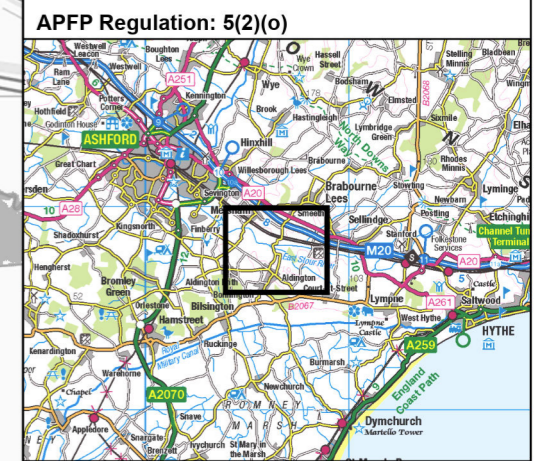
LEGEND

- Order limits
- Field

Depth Change (mm)

- <= -40
- 20 to -40
- 10 to -20
- 10 to -5
- 5 to -1
- 1 to 1
- 1 to 5
- 5 to 10
- 10 to 20
- 20 to 40
- >= 40

*Baseline vs Post Development 25% Fence Blockage
1:100 - 36.25 hr Rainfall Event - 2 m Grid



STONESTREET GREEN SOLAR
HYDRAULIC MODELLING REPORT
POST DEVELOPMENT 1.0% ANNUAL
EXCEEDANCE PROBABILITY AND UPPER
END CLIMATE CHANGE
25% FENCE BLOCKAGE FLOOD DEPTHS

HMR Figure 25

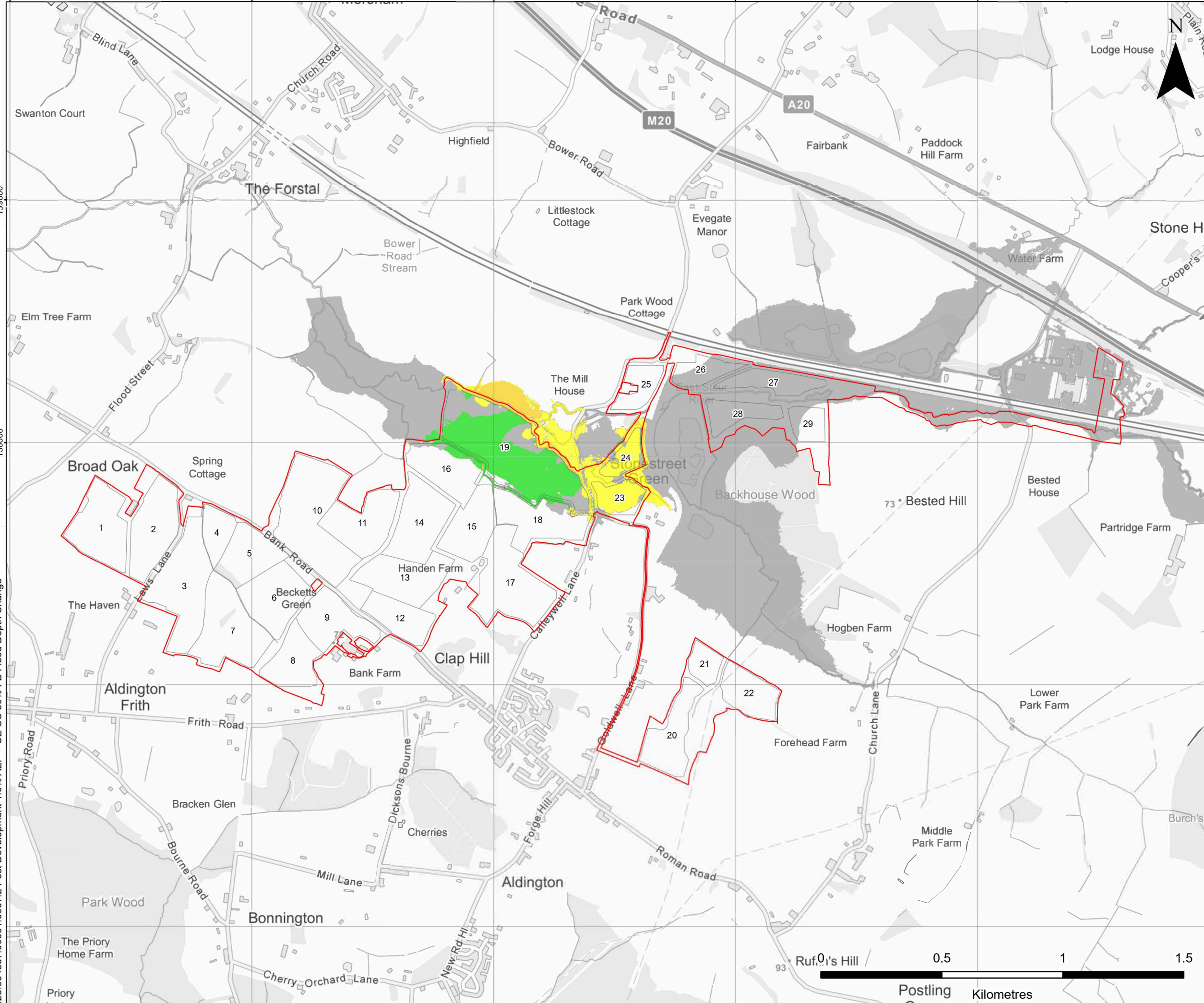
Scale: 1:15,000 @ A3 Date: MAY 2024

425.064837.00001.0066.2 Post Development 1.0% AEP + UE CC 25% FB Flood Depth Change

604000 605000 606000 607000 608000

139000
138000

425.064837.00001.0067.2 Post Development 1.0% AEP + UE CC 50% FB Flood Depth Change



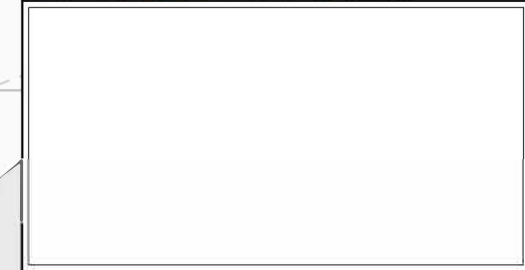
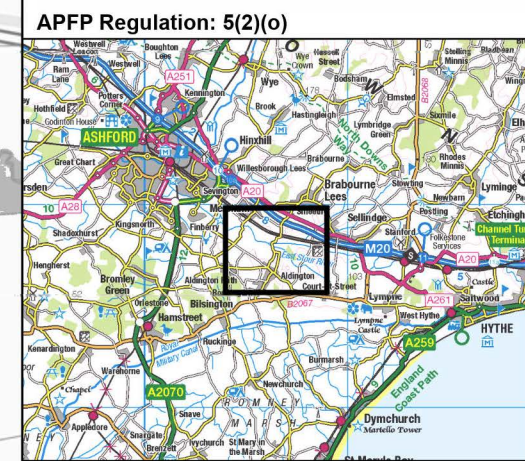
LEGEND

- Order limits
- Field

Depth Change (mm)

- <= -40
- 20 to -40
- 10 to -20
- 10 to -5
- 5 to -1
- 1 to 1
- 1 to 5
- 5 to 10
- 10 to 20
- 20 to 40
- >= 40

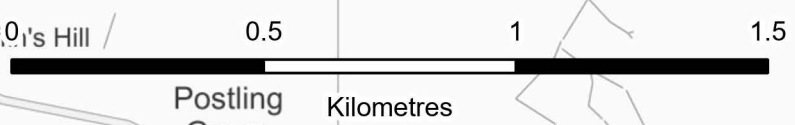
*Baseline vs Post Development 50% Fence Blockage
1:100 - 36.25 hr Rainfall Event - 2 m Grid



STONESTREET GREEN SOLAR
HYDRAULIC MODELLING REPORT
POST DEVELOPMENT 1.0% ANNUAL
EXCEEDANCE PROBABILITY AND UPPER
END CLIMATE CHANGE
50% FENCE BLOCKAGE FLOOD DEPTHS

HMR Figure 26

Scale 1:15,000 @ A3 Date MAY 2024





Making Sustainability Happen

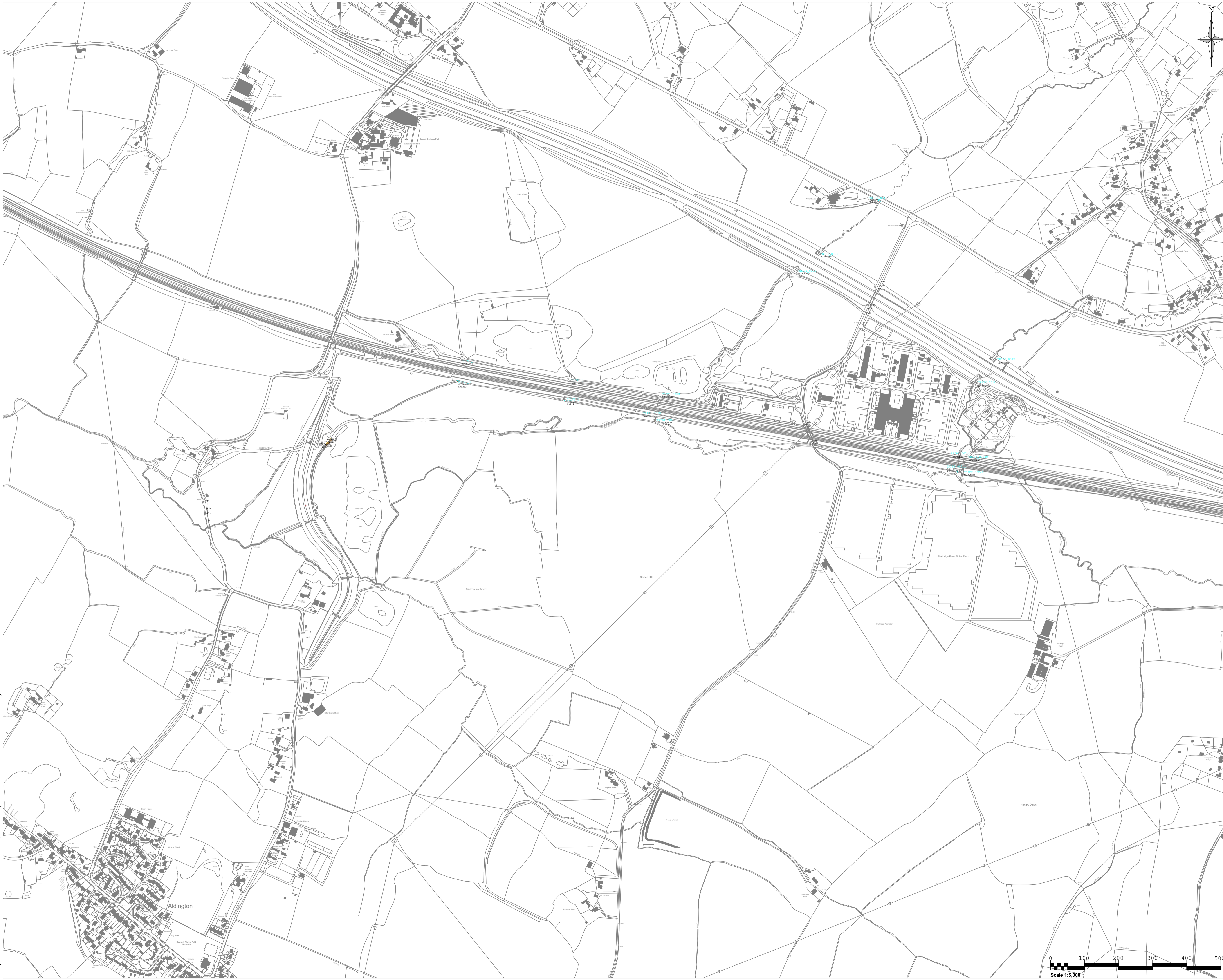
Annex A **SLR Topographic Survey**

Annex B: East Stour Hydraulic Modelling Report

Stonestreet Green Solar Farm

EPL 001 Limited





- Notes:**
1. TOPOGRAPHICAL SURVEY CARRIED OUT BY SLR CONSULTING 24th-26th JULY 2023.
 2. THIS PLAN IS ORIENTATED TO ORDNANCE SURVEY NATIONAL GRID PLAN AND HEIGHT DATUM DERIVED FROM THE ORDNANCE SURVEY ACTIVE NETWORK; OSGB36(OSGM15/OSTN15).
 3. WHILST EVERY EFFORT HAS BEEN MADE TO INCLUDE ALL ACCESSIBLE DETAIL, SOME FEATURES MAY NOT BE SHOWN IF OBTAINED AT THE TIME OF SURVEY (e.g. PARKED VEHICLES/BOATS/OVERGROWN AREAS). ONLY MAJOR TREES SURVEYED IN WOODED/OVERGROWN AREAS.
 4. THE ACCURACY OF THIS SURVEY IS COMMENSURATE WITH THE DRAWING SCALE SPECIFIED WITHIN THE TITLE BLOCK. ALL CRUCIAL DIMENSIONS SHOULD BE CHECKED ON-SITE.
 5. THIS PLAN IS TO OSGB36 DATUM.

- Legend:**
- SPOT LEVEL
 - SURVEY CONTROL STATION WITH ID
 - KERB BOTTOM
 - KERB TOP
 - CHANGE IN SURFACE
 - WALL BOTTOM
 - FENCE
 - GATE
 - BUILDING HATCH
 - FOLIAGE
 - CANOPY

BB	Belted Beacon	LR	Life Ring
BC	Building Contour	LM	Marker
BD	Borehole	MOR	Marker
BE	Bench Mark	ND	Not
BF	Boundary	PTH	Door Threshold Level
BGL	Belted	PI	Pipe Invert
BT	British Telecom Manhole	RE	Rodding Eye
CTV	Cable Television	RL	Ridge Level
CP	Chamber Pit	RNP	Road Name Plate
CTV	Cable Television Manhole	RS	Sign Post
CUL	Culvert	SL	Spot Light
DK	Drop Kerb	SO	Soft Level
DP	Drain Pipe	ST	Stop Top
DPC	Damp Proof Course	ST	Stop Top
DWB	Dog Waste Bin	SV	Stop Valve
EB	Electricity Box	SVP	Stop Valve Pipe
EL	Eaves Level	SW	Storm Water
EP	Electricity Pole	SY	Stop (i.e. Pylon, TP)
ER	Earthing Rod	TAP	Stand Pipe
ER	Earthing Rod Level	TGB	Telephone Call Box
FL	Internal Floor Level	TCL	Tactile
FW	Fire Post	TLE	Threshold Level
FA	Fire Alarm	TL	Traffic Light
GB	Gas Box	TOD	Top of Deck Level
GV	Gas Valve	TW	Top of Wall
GP	Gate Post	TP	Telegraph Pole
GRT	Gate	TPT	Tree Pit
G	Gully	UB	Under Beam
PH	Hydrant	UTL	Unable To LIT
IC	Inspection Chamber	UP	Up Pipe
IT	Intercom	WEL	Window Sill Level
KO	Kerb Outlet	WHL	Window Head Level
LB	Life Buoy	WL	Water Level
LP	Lamp Post	WM	Water Meter
		WVO	Wash Out

0		28/07/23	JL	GJH	VQ
Rev	Amendments	Date	By	Chk	Auth



Drawing Status & Suitability Code

Client
EPL 001 LIMITED

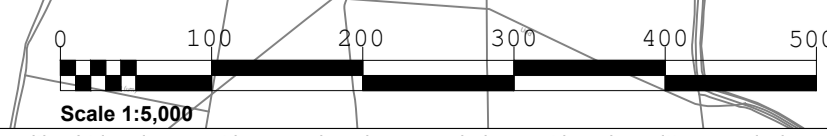
Project
**STONESTREET GREEN
ASHFORD**

Drawing Title
**TOPOGRAPHICAL SURVEY
JULY 2023**

Scale
1:5000 @ A1 SLR Project No.
425.064837.00001

Designed	Drawn	Checked	Authorised
	JL	GJH	VQ
Date	Date	Date	Date
	28/07/2023	28/07/2023	28/07/2023

Drawing Number
425.064837.00001.09.01



H:\Projects\425.064837.00001.Stonestreet.Green_Solar_Farm\input\SLR_Survey\425.064837.00001.09.01.STONESTREET_CE.dwg 22/01/2024 Clement Elhal

Annex B **Environment Agency Supplied Topographic Survey**

Annex B: East Stour Hydraulic Modelling Report

Stonestreet Green Solar Farm

EPL 001 Limited



J01058 – East Stour, Ashford to Stanford Channel Survey - C00322
Maltby Land Surveys Ltd - 17_322.
December 2017 - March 2018

J01058 - East Stour, Ashford to Stanford Channel Survey – C00322
December 2017 - March 2018

1) WRITTEN REPORT

- A) Objectives**
- B) Survey Control**
- C) Topography**
- D) Digital Data**
- E) General Comments**

2) STATION DESCRIPTIONS

3) EQUIPMENT LIST

4) DELIVERABLES

5) SELF CERTIFICATION SHEETS

1. WRITTEN REPORT

To: [REDACTED], Orchard House, Endeavour Park, London Road,
Addington, West Malling, Kent, ME19 5SH

From: Maltby Land Surveys Limited

Our Reference: 17_322

Survey Site: East Stour and Tributaries

Date of Survey: January-March 2018

A) Objectives

Further to your instruction, a river channel survey was carried out on the above watercourse in accordance with your survey specification.

Survey Extents

East Stour River (ESTO01) – TQ 0160 4286 Open Channel, Confluence with Great Stour to TQ 1316 3824 Culvert Exit, B2068.

East Stour River Tributary (ESTO02) – TQ 0235 4001 Open Channel, Confluence with ESTO01 to TQ 0298 3966 Open Channel, Confluence with ESTO01.

East Stour River Tributary (ESTO03) – TQ 0340 3938 Open Channel, Confluence with ESTO01 to TQ 0388 3883 Mill Face, Confluence with ESTO01.

East Stour River Tributary (ESTO04) – TQ 0379 3923 Open Channel, Confluence with ESTO01 to TQ 0432 3897 Sluice, Confluence with ESTO01.

East Stour River Tributary (ESTO05) – TQ 0492 3909 Open Channel, Confluence with ESTO01 to TQ 0496 3909 Culvert Entrance, Confluence with ESTO01.

East Stour River Tributary (ESTO05) – TQ 0492 3909 Open Channel, Confluence with ESTO01 to TQ 0496 3909 Culvert Entrance, Confluence with ESTO01.

East Stour River Tributary (ESTO06) – TQ 0620 3806 Open Channel, Confluence with ESTO01 to TQ 0643 3811 Open Channel, Confluence with ESTO01.

East Stour River Tributary (ESTO07) – TQ 0616 3804 Open Channel, Confluence with ESTO01 to TQ 0671 3814 Weir, Confluence with ESTO01.

East Stour River Tributary (ESTO08) – TQ 0471 3898 Open Channel, Confluence with ESTO01 to TQ 0475 3902 Culvert Entrance, Confluence with ESTO01.

Rail Culvert (ESTO09) – TQ 1116 3762 Culvert Entrance to TQ 1115 3756 Open Channel.

East Stour River Tributary (ESTO10) – TQ 0616 3804 Open Channel, Confluence with ESTO01 to TQ 0639 3810 Footbridge, Confluence with ESTO06.

Avocet Way Flood Relief Culvert (AFRC01-AFRC07) - Avocet Way

4) Specified Requirements

The *services* include, but are not limited to, the provision of the items specified in Section 2.

Project Management

Day to day management of project delivery shall be the responsibility of the *Consultant*.

In managing the *services* the *Consultant* shall undertake the following:

1. Maintain weekly contact with the *Employer's* Project Manager such that the *Employer* is fully informed of progress and issues.

Product Delivery

In delivering the *services* the *Consultant* shall:

1. Ensure all deliverables and products show evidence of a quality control system. General quality tolerances should consist of:
 - a) All first draft deliverables shall be free from significant error and be consistent with other documents, avoiding repetition.
 - b) All first draft products shall tell a logical story with a clear audit trail of consultation and decision making processes, supported by robust evidence, which can be easily transferred and interpreted by future users.
 - c) All first draft deliverables shall be grammatically correct and clearly presented, with consistent formatting.
 - d) All first draft deliverables shall satisfy the relevant latest guidance and legislative requirements.
 - e) All first draft deliverables shall conform to the latest and necessary guidance and meet the template requirements, unless otherwise agreed with the *Employer* in advance of submission.
 - f) Products shall be suitable for the audience and written using 'plain English'.
2. Ensure all drawings are produced at the scale described below in Section 2.4.

Previous Studies and Data Sources

The *Consultant* shall obtain Ordnance Survey mapping data and other relevant data products from the Partner Data Portal website and comply with the licensing of that site.

Survey

The *Consultant* shall:

1. Obtain all consents and approvals required for the survey to be undertaken.

2. When on site, carry with them at all times the letter of introduction, or similar, supplied by the *Employer*.
3. Deliver the survey as described in this Scope.
4. Ensure the *Employer's* estates lead is given 4 weeks advance notice of any land entry required.
5. Provide final outputs of the survey to the Employer in a digital format.
6. Ensure that all survey results including the survey report are to be delivered as a zipped file email attachment if the zipped file is smaller than 10Mb or via DEFRA Sharefile (link supplied by Employer upon request).

General Requirements applicable to all surveys, (Section I of Specifications)

The following is to be copied and pasted into the survey report as a record of the survey requirement.

The *Consultant* shall:

Submit all documents with the EA job number in the file name.

Name drawing files A-XXXXXX-BB.dwg where A may be:

Letter	Meaning
L	Long section
X	Cross sections
T	Topographic map
H	Hydrographic chart

BB is a sequential sheet number – 01, 02 etc.

And where XXXXXX is the job number - J01058

Submit weekly progress reports to EA Project Manager by email / telephone.

2.4.1 Survey Control (Section II of specifications)

The *Consultant* shall:

Take GNSS observations at existing stations shown on the contract map to E6 standard.

Establish permanent survey control stations in the approximate locations shown on C00322_Sheet1 to C00322_sheet5.pdf. When it is not possible to take GNSS observations at a permanent station, the surveyor shall establish a temporary station for the GNSS observations and a permanent E5 station on a suitable stable structure. He shall level from the GNSS station to the E5 station to determine the height.

GNSS observations shall be to E6 standard

Number EACS using the numbering system in Paragraph 3.2.5 of the National Technical Survey Specification.

Provide EACS description cards as .doc format files.

2.4.3 Channel Surveys (Section IV of specifications)

The *Consultant* shall:

Allow within their pricing for one day walkover of the site with the EA project manager / consultant.

Use the channel centreline as shown on the contract map.

Supply channel survey data in ISIS, formats. **Tenderers to state in the methodology if they will be supplying structure data automatically from survey data in ISIS format.**

Supply data in EACSD V3.2 format and include CES1 and CES2 data. EACSD shall be passed through the Environment Agency's on-line validator program (www.eacsd.co.uk) and the validation certificate presented in the survey report.

Survey open channel and structure cross sections in approximate locations indicated on the contract mapping.

Survey channel hard bed levels. Separate ISIS digital data files are to be provided for hard and for soft bed data.

Extend cross sections 5m beyond the river bank into the flood plain.

Number cross-sections in the format AAAABBCCCC where AAAA is an alphanumeric watercourse identifier, BB is braid or side channel number and CCCCC is the cross-section chainage. These are marked on the contract mapping. Chainage for the East Stour is as shown on the contract mapping. For the side channels, chainage shall be 0+000 at the downstream confluence and increase upstream.

Present cross-sections at 1:100 natural scale, longitudinal sections at 1:2500 horizontal and 1:100 vertical scale or as agreed with the EA project manager.

Survey weirs and drop structures if their head of water is 0.2m or greater.

Survey a structure section for all pipes crossing the water course which have a diameter greater than 0.35m.

Survey and show on the long profile outfalls entering the watercourse of 350mm diameter or greater (re: para 9.3.5).

Produce gaugeboard description cards using template gaugeboard_blank.doc.

Survey surveyed in accordance with Clause 9.5 of the specification river banks which are raised above the floodplain.

B) Survey Control

Control for the survey was derived using Global Navigation Satellite Systems (GNSS) utilising Leica ‘SmartNet’, a Network Real Time Kinematic (RTK) survey method.

Two new Environment Agency Grade Control Stations (EACS) E50733004 & E50733005 were surveyed to E5 grade specification. Seventeen various previously installed EACS have been surveyed to various EA grades, for values relating to checks please see *J01058 – EACS Checks.xls and table B.2 for OSTN15 Check Values*. SmartNet was used to install control throughout the survey catchment where necessary, installing a minimum of two reference pegs for each set up. All supplied data is to the OSTN15.

GNSS techniques were deployed in accordance with the EA specification. Observations have been taken using Leica GS08+ receivers utilising the OSGM15 geoid model and OSTN15 transformation model to obtain OSGB36 co-ordinates. The data is imported into Leica Geo Office so a Mean Co-ordinates and Difference report can be created. The weighted average coordinates from the field are then used to derive the height of newly installed EACS with best fit levelling. See folder ‘13. Survey Control’ for processing data.

Table B.1 –Details of New EACS.

EACS	Easting (NGm)	Northing (NGm)	Hgt (NGm)
E50733004	605042	139000	43.712
E50733005	610656	137517	60.587

Table B.2 –OSTN15 Values of existing EACS.

OSTN15 Values				Comments
Station	Easting	Northing	Hgt	
E20730012	601035.741	141073.248	35.975	
E20730401	601495.721	142538.993	35.267	
E20730403	602222.987	140275.473	38.489	
E20731536			51.806	Levelled only (No Position from Historic Data (OSTN02))
E50730013	601326.653	142026.358	38.489	Position Value Derived from 2 x 3min observation
E50730020	601592.794	140613.787	36.314	Position Derived from Section Resection
E50730021			36.337	Levelled only (No Position from Historic Data (OSTN02))
E50730500			38.486	Levelled only (No Position from Historic Data (OSTN02))
E50730503	603435.819	139359.525	40.259	Position Value Derived from 2 x 3min observation
E50731495	601526.000	140706.000	37.864	Levelled only (Position from Historic Data (OSTN02))
E50731496	601288.000	141992.000	37.732	Levelled only (Position from Historic Data (OSTN02))
E50731537	606676.066	137917.409	47.711	Position Value Derived from 2 x 3min observation
E50731538	606712.362	138424.528	63.022	Position Value Derived from 2 x 3min observation
E50731571	606662.807	138137.856	50.976	Levelled only (Position from Historic Data (OSTN02))
E60731483	601512.490	140701.055	35.916	
E60731484	601332.236	142006.215	37.463	
E60731539	606717.365	138286.468	52.251	

C) Topography

The topography has been surveyed using a Leica TS06 Total Station in accordance with the specification.

D) Digital Data

For list of data provided please see section 4. Deliverables

All data supplied has been run through the latest edition of AVG Anti – Virus software and is, to the best of our knowledge, virus – free.

E) General Comments

As per the contract mapping, reaches ESTO01 to ESTO07 have been completed. Addition reaches ESTO08 to ESTO10 have been created where single sections have been requested. As previously stated, requested reach ESTO05 located at Hanover Mill has been surveyed as ESTO01 with the ESTO05 now going through the Mill House. See relevant long section drawings.

Numerous additional Structures surveyed to specification

Flood Modeller data which contains structure information is still being completed and will be delivered in due course.

East Stour River (ESTO01)

Requested Crest levels at TQ 0152 4224 have been surveyed and added to drawing L-J01058-01, with XYZ data (EMB01) supplied.

Due to access (Network Rail) historic data has been utilised on section ESTO01_01022, the main opening was surveyed with non-contact survey methods with the historic data referenced to these levels

Requested wall levels at TQ 0125 4164 have been surveyed and added to drawing L-J01058-01, with XYZ data (TOW01) supplied.

A ponding area was located at TQ 0323 3941, additional data has been surveyed to show area, see drawing L-J01058-03 and accompanying XYZ data (EMB02).

Unable to survey full extent of culvert exit at location TQ 1097 3761, this is due to culvert being located on network Rail land, non-contact survey methods have been utilised to collect data on structure with an open channel surveyed at the network rail boundary.

Unable to survey culvert entrance at location TQ 1144 3752, this is due to culvert being located on Network Rail land, non-contact survey methods have been utilised to collect long section structure data.

Unable to survey culvert entrance at TQ 1318 3826, this was due to dense foliage. MLS have surveyed the culvert exit (ESTO01_20131).

East Stour River Tributary (ESTO02))

Additional Access Bridge section ESTO02_0999 has been surveyed, no opening was found, and it appears the bridge is made up of earth which has been dropped in the channel to create an access point between ESTO01 and ESTO02.

East Stour River Tributary (ESTO03)

The water source is from the pipe on the left bank downstream of section ESTO03_0788. We were unable to survey the dimensions of Swanton Mill, due to confined space.

East Stour River Tributary (ESTO04)

Surveyed passed with-out incident.

East Stour River Tributary (ESTO05)

Reach has been surveyed through Hanover Mill.

East Stour River Tributary (ESTO06)

Surveyed passed with-out incident.

East Stour River Tributary (ESTO07)

Water levels were high during the survey. Due to this a large amount of debris were present on the trash screen on section ESTO07_0741. A silt trap and overflow structure were located between section ESTO07_0741 and ESTO07_0750, this is shown on long section and ESTO07_0750 Through Section.

East Stour River Tributary (ESTO08)

Additional reach surveyed to show full channel.

Rail Culvert (ESTO09)

Surveyed passed with-out incident.

East Stour River Tributary (ESTO10)

Surveyed passed with-out incident.

Avocet Way Flood Relief Culvert (AFRC01-AFRC07)

All data has been supplied in a separate folder 11. Flood Relief Culverts.

3 .EQUIPMENT LIST

- Leica GS08+ Series Dual Frequency Geodetic, RTK Receiver (x2)
Serial numbers: 2884265,2884490
- Leica TS06 Total Station, serial number: 1391964,1392085

J01058 – East Stour, Ashford to Stanford Channel Survey - C00322
Maltby Land Surveys Ltd - 17_322.
December 2017 - March 2018

4. DELIVERABLES

		Format	File	Contents
1. ESTO01	1. Cross Sections	AutoCAD	X-J01058-01-30.dwg X-J01058-01-30.pdf	Cross Section Drawings
	2. Long Section & Location Plan	AutoCAD	L-J01058-01-09.dwg L-J01058-01-09.pdf	Long Section & Location plan
	3. Flood Modeller	.dat	J01058 - ESTO01_FMod_Hard.dat J01058 - ESTO01_FMod_Soft.dat	Flood Modeller
	4. XYZ	Excel	J01058 - ESTO01_XYZ_HardBed.xls J01058 - ESTO01_XYZ_SoftBed.xls J01058 - Additional XYZ Data.xls	XYZ Data
	5. EACSD	Various	J01058 - ESTO01_EACSD.txt J01058 - ESTO01_EACSD_Validator.pdf	EACSD & Validator
	6. Gauge Board	Word	Various	Gauge Board Description Sheet
	7. Photos	JPEG Excel	Various	Photos & Photo Schedule
1. ESTO02	1. Cross Sections	AutoCAD	X-J01058-31-32.dwg X-J01058-31-32.pdf	Cross Section Drawings
	2. Long Section & Location Plan	AutoCAD	L-J01058-10.dwg L-J01058-10.pdf	Long Section & Location plan
	3. Flood Modeller	.dat	J01058 - ESTO02_FMod_Hard.dat J01058 - ESTO02_FMod_Soft.dat	Flood Modeller
	4. XYZ	Excel	J01058 - ESTO02_XYZ_HardBed.xls J01058 - ESTO02_XYZ_SoftBed.xls	XYZ Data
	5. EACSD	Various	J01058 – EST002_EACSD.TXT J01058 – EST002_EACSD_Validator.pdf	EACSD & Validator
	6. Photos	JPEG Excel	Various	Photos & Photo Schedule
3. ESTO03	1. Cross Sections	AutoCAD	X-J01058-33.dwg X-J01058-33.pdf	Cross Section Drawings
	2. Long Section & Location Plan	AutoCAD	L-J01058-11.dwg L-J01058-11.pdf	Long Section & Location plan
	3. Flood Modeller	.dat	J01058 - ESTO03_FMod_Hard.dat J01058 - ESTO03_FMod_Soft.dat	Flood Modeller
	4. XYZ	Excel	J01058 - ESTO03_XYZ_HardBed.xls J01058 - ESTO03_XYZ_SoftBed.xls	XYZ Data

J01058 – East Stour, Ashford to Stanford Channel Survey - C00322
Maltby Land Surveys Ltd - 17_322.
 December 2017 - March 2018

	5. EACSD	Various	J01058 – EST003_EACSD.TXT J01058 – EST003_EACSD_Validator.pdf	EACSD & Validator
	6. Photos	JPEG Excel	Various	Photos & Photo Schedule
4. ESTO04	1. Cross Sections	AutoCAD	X-J01058-34.dwg X-J01058-34.pdf	Cross Section Drawings
	2. Long Section & Location Plan	AutoCAD	L-J01058-12.dwg L-J01058-12.pdf	Long Section & Location plan
	3. Flood Modeller	.dat	J01058 - ESTO04_FMod_Hard.dat J01058 - ESTO04_FMod_Soft.dat	Flood Modeller
	4. XYZ	Excel	J01058 - ESTO04_XYZ_HardBed.xls J01058 - ESTO04_XYZ_SoftBed.xls	XYZ Data
	5. EACSD	Various	J01058 – EST004_EACSD.TXT J01058 – EST004_EACSD_Validator.pdf	EACSD & Validator
	6. Photos	JPEG Excel	Various	Photos & Photo Schedule
5. ESTO05	1. Cross Sections, Long Section & Location Plan	AutoCAD	X-J01058-35.dwg X-J01058-35.pdf	Cross Section, Long Section & Location plan
	2. Flood Modeller	.dat	J01058 - ESTO05_FMod_Hard.dat J01058 - ESTO05_FMod_Soft.dat	Flood Modeller
	3. XYZ	Excel	J01058 - ESTO05_XYZ_HardBed.xls J01058 - ESTO05_XYZ_SoftBed.xls	XYZ Data
	4. EACSD	Various	J01058 – EST005_EACSD.TXT J01058 – EST005_EACSD_Validator.pdf	EACSD & Validator
	5. Photos	JPEG Excel	Various	Photos & Photo Schedule
6. ESTO06	1. Cross Sections	AutoCAD	X-J01058-36.dwg X-J01058-36.pdf	Cross Section Drawings
	2. Long Section & Location Plan	AutoCAD	L-J01058-13.dwg L-J01058-13.pdf	Long Section & Location plan
	3. Flood Modeller	.dat	J01058 - ESTO06_FMod_Hard.dat J01058 - ESTO06_FMod_Soft.dat	Flood Modeller
	4. XYZ	Excel	J01058 - ESTO06_XYZ_HardBed.xls J01058 - ESTO06_XYZ_SoftBed.xls	XYZ Data
	5. EACSD	Various	J01058 – EST006_EACSD.TXT J01058 – EST006_EACSD_Validator.pdf	EACSD & Validator

J01058 – East Stour, Ashford to Stanford Channel Survey - C00322
Maltby Land Surveys Ltd - 17_322.

December 2017 - March 2018

	6. Photos	JPEG Excel	Various	Photos & Photo Schedule
7. ESTO07	1. Cross Sections	AutoCAD	X-J01058-37-38.dwg X-J01058-37-38.pdf	Cross Section Drawings
	2. Long Section & Location Plan	AutoCAD	L-J01058-14.dwg L-J01058-14.pdf	Long Section & Location plan
	3. Flood Modeller	.dat	J01058 - ESTO07_FMod.dat	Flood Modeller
	4. XYZ	Excel	J01058 - ESTO07_XYZ.xls	XYZ Data
	5. EACSD	Various	J01058 - ESTO07_EACSD.txt J01058 - ESTO07_EACSD_Validator.pdf	EACSD & Validator
	6. Gauge Board	Word	Various	Gauge Board Description Sheet
	7. Photos	JPEG Excel	Various	Photos & Photo Schedule
8. ESTO08	1. Cross Sections	AutoCAD	X-J01058-39.dwg X-J01058-39.pdf	Cross Section Drawings
	2. Long Section & Location Plan	AutoCAD	L-J01058-15.dwg L-J01058-15.pdf	Long Section & Location plan
	3. Flood Modeller	.dat	J01058 - ESTO08_FMod.dat	Flood Modeller
	4. XYZ	Excel	J01058 - ESTO08_XYZ.xls	XYZ Data
	5. EACSD	Various	J01058 – EST008_EACSD.TXT J01058 – EST008_EACSD_Validator.pdf	EACSD & Validator
	6. Photos	JPEG Excel	Various	Photos & Photo Schedule
9. ESTO09	1. Cross Sections, Long Section & Location Plan	AutoCAD	X-J01058-40.dwg X-J01058-40.pdf	Cross Section, Long Section & Location plan
	2. Flood Modeller	.dat	J01058 - ESTO09_FMod.dat	Flood Modeller
	3. XYZ	Excel	J01058 - ESTO09_XYZ.xls	XYZ Data
	4. EACSD	Various	J01058 – EST009_EACSD.TXT J01058 – EST009_EACSD_Validator.pdf	EACSD & Validator
	5. Photos	JPEG Excel	Various	Photos & Photo Schedule

J01058 – East Stour, Ashford to Stanford Channel Survey - C00322

Maltby Land Surveys Ltd - 17_322.

December 2017 - March 2018

<p>10. ESTO10</p>	<p>1. Cross Sections, Long Section & Location Plan</p> <p>2. Flood Modeller</p> <p>3. XYZ</p> <p>4. EACSD</p> <p>5. Photos</p>	<p>AutoCAD</p> <p>.dat</p> <p>Excel</p> <p>Various</p> <p>JPEG Excel</p>	<p>X-J01058-41.dwg X-J01058-41.pdf</p> <p>J01058 – ESTO10_FMod_Hard.dat J01058 – ESTO10_FMod_Hard.dat</p> <p>J01058 – ESTO10_XYZ_HardBed.xls J01058 – ESTO10_XYZ_SoftBed.xls</p> <p>J01058 – ESTO10_EACSD.TXT J01058 – ESTO10_EACSD_Validator.pdf</p> <p>Various</p>	<p>Cross Section, Long Section & Location plan</p> <p>Flood Modeller</p> <p>XYZ Data</p> <p>EACSD & Validator</p> <p>Photos & Photo Schedule</p>
<p>11. Flood Relief Culverts</p>	<p>1. Cross Sections</p> <p>2. Location Plan</p> <p>3. Flood Modeller</p> <p>4. XYZ</p> <p>5. Photos</p> <p>6.EACSD</p>	<p>AutoCAD</p> <p>AutoCAD</p> <p>.dat</p> <p>Excel</p> <p>JPEG Excel Various</p>	<p>X-J01058-42-43.dwg X-J01058-42-43.pdf</p> <p>L-J01058-16.dwg L-J01058-16.pdf</p> <p>AFRC01-07_Mod.dat</p> <p>J01058_AFRC_XYZ</p> <p>Various</p> <p>J01058 – AFRC01-07_EACSD.TXT J01058 – AFRC01-07_EACSD_Validator.pdf</p>	<p>Cross Section Drawings</p> <p>Location Plan</p> <p>Flood Modeller</p> <p>XYZ Data</p> <p>Photos & Photo Schedule</p> <p>EACSD & Validator</p>
<p>12. Survey Control</p>	<p>1. Level Runs & Adjustments</p> <p>2. Description Sheets</p> <p>3. Temporary E6 Photos</p> <p>4. GNSS Report</p>	<p>Excel</p> <p>Word</p> <p>.JPG</p> <p>Word</p>	<p>J01058 – Permanent Control Schedule.xls J01058 - Level Run.xls J01058 - EACS Adjustments.xls J01058 - Temporary Control Schedule.xls J01058 - EACS Checks.xls</p> <p>Various</p> <p>Various</p> <p>J01058 – GNSS Report (Existing).docx J01058 – GNSS Report (New).docx</p>	<p>E6 Control Data</p> <p>Description Sheets</p> <p>Photos</p> <p>GNSS Report</p>
<p>13. Report</p>		<p>Word</p>	<p>J01058 East Stour Survey Report.doc</p>	<p>Written Survey Report</p>

5. Self Certification Forms

ENVIRONMENT AGENCY ISSUE 03/03.2

SELF CERTIFICATION CHANNEL SURVEY CHECK LIST

Job number	J01058	Company	Maltby Land Surveys		
Task	East Stour				
Surveyor	JB-RC	Checker	its	Date	29/03/2018

COPIES OF THE FOLLOWING WILL BE PROVIDED TO THE SURVEYOR'S CHECKER
YES

Survey Brief & mapping	x	Equipment list	x
OS & EA Control Sheets	x	GNSS Report	x
Vertical Control Field Sheets	x		
Channel Profile Sheets	x		

VERTICAL CONTROL
NO YES

Check control values against Control Data		x
Check level comparisons between heighting by GNSS and from BMs		x
Check other level misclosures and Two Peg Test results		x

5)

PLOTS
NO YES

Does survey meet specification? IS IT DOWNSTREAM?		x
Check Title Box is complete and contains the correct Agency Region		x
Check control values and descriptions on longitudinal sections		x
Check for incomplete features		x
Check for correct sequence of section numbers on longitudinal section		x
Check height labels on cross-section for gross errors		x
Check cross-section feature heights with longitudinal section heights		x

CROSS SECTION LABELLING

SHEET

Fence type/height	XJ01058 01-43
Bridge widths	XJ01058 01-43
Soffits/Invert heights	XJ01058 01-43
Skew Diagrams	XJ01058 01-43

Bed and Bank Material	XJ01058_01-43
--------------------------	---------------

EA Control Stations

Check originating control values _____ and EA CS value
Check grid square _____ Check diagram

CHECK KEY PLAN

NO YES

Correct sequence of sections		X
Do the sections plotted from co-ordinate data fall in the right place on the OS background		X
Co-ordinates, scale, north point		X
Check digital location plan.		X
Legend on longitudinal section		

REPORT

YES

NO

Standard written format used		X
Landowner plan or schedule		
EACS, TBM and Structure photographs adequate		X
All required appendices present		X

REMEDIAL ACTION

Please action the following:

I CERTIFY THAT ALL THE REQUIRED CHECKS HAVE BEEN CARRIED OUT AND ALL
REMEDIAL ACTIONS HAVE BEEN COMPLETED.

SIGNED:

PRINT NAME:



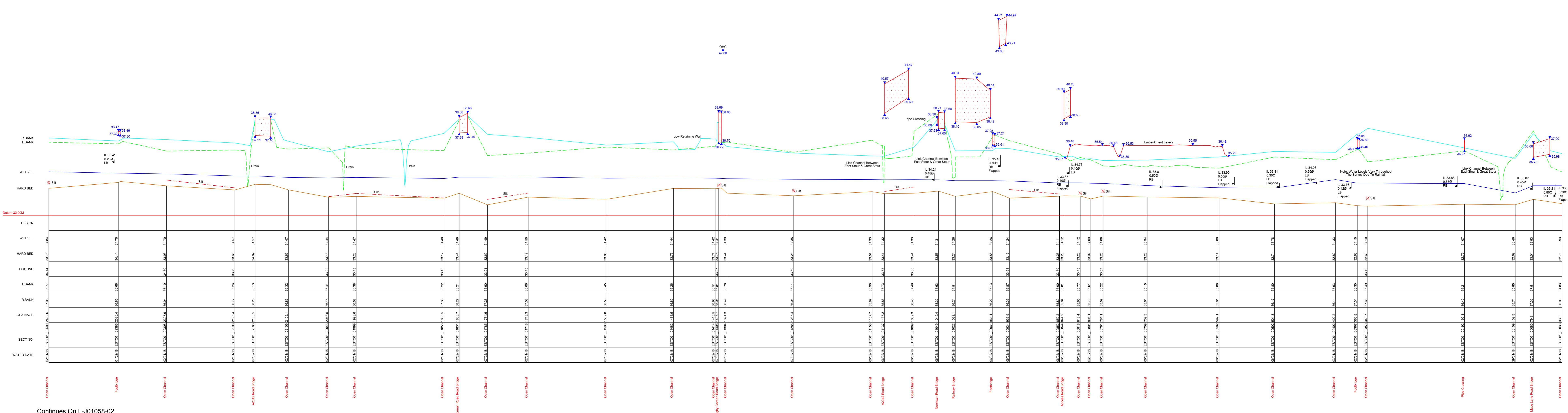
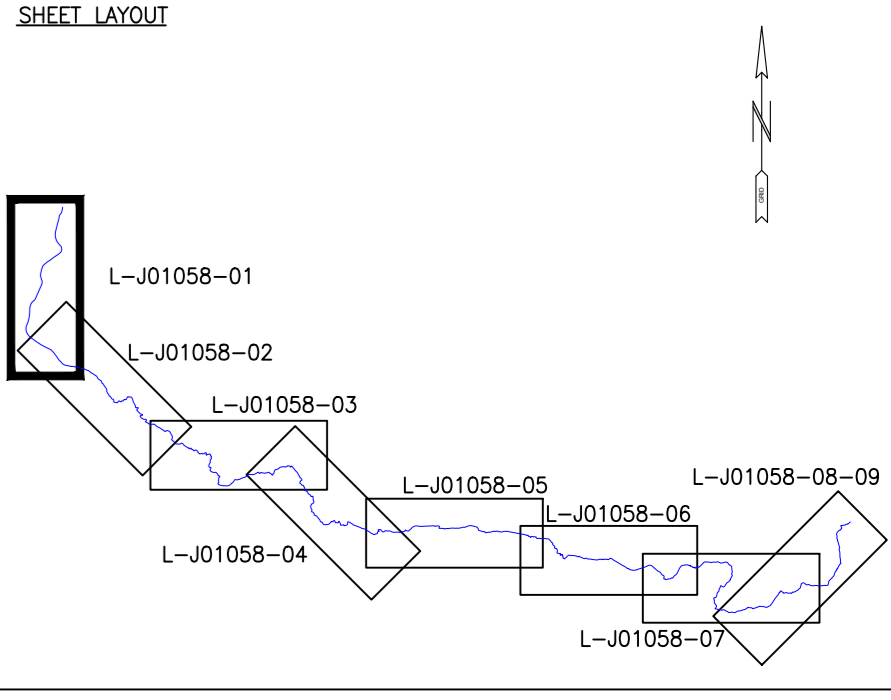
POSITION: COMMERCIAL DIRECTOR

DATE: 29/03/2018

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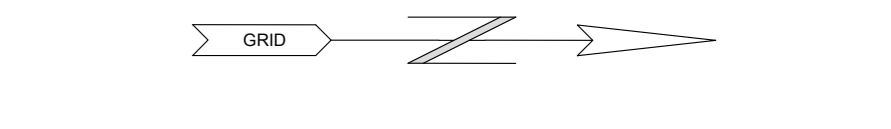
Original Drawing Size: A0

- KEY TO SECTIONS:
- WATER LEVEL
 - VISIBLE BED (TOP OF SILT) AND GROUND
 - HARD BED (DETERMINED BY PROBING)
 - BANK CREST
- KEY TO LONGITUDINAL SECTION ONLY:
- LEFT BANK CREST
 - RIGHT BANK CREST
 - GAUGE BOARD LOCATION
 - TOP OF WALL
 - TOP OF EMBANKMENT
- VIEWED LOOKING DOWNSTREAM
POINTS INDICATED BY 'C' ON
CROSS SECTIONS AND ADDITIONAL
POINTS BETWEEN SECTIONS



Continues On L-J01058-02

LOCATION PLAN ORIENTATION



NOTES:

1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
2. THIS MAP IS REPRODUCED FROM THE OS MAP BY THE ENVIRONMENT AGENCY WITH PERMISSION OF ORDNANCE SURVEY ON BEHALF OF THE CONTROLLER OF HER MAJESTY'S STATIONERY OFFICE. © CROWN COPYRIGHT LICENCE. ALL RIGHTS RESERVED. UNAUTHORISED REPRODUCTION INFRINGES CROWN COPYRIGHT AND MAY LEAD TO PROSECUTION OR CIVIL PROCEEDINGS. LICENCE NO. 100026380.
3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

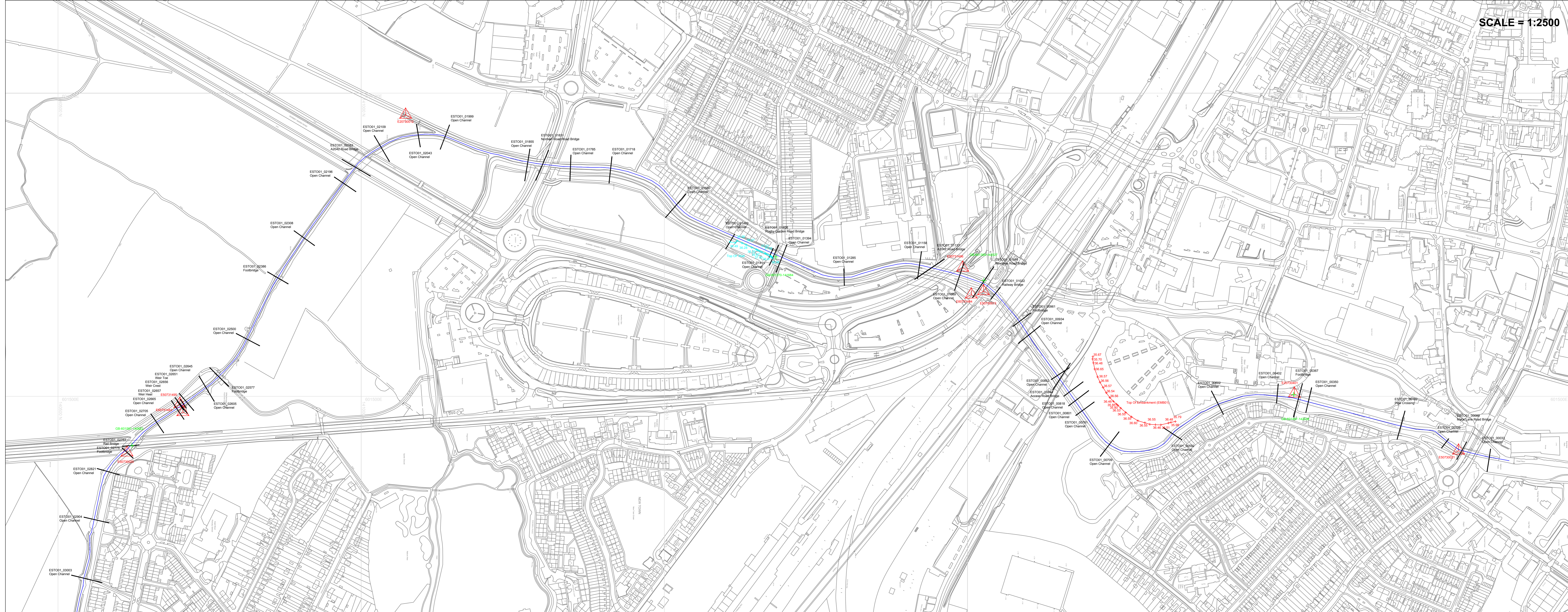
SURVEY LEGEND

SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
...

AMENDMENT	NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E20730012	TR 0103 4107	36.925
E20730013	TR 0103 4108	36.925
E20730014	TR 0103 4109	36.925
E20730015	TR 0103 4110	36.925
E20730016	TR 0103 4111	36.925
E20730017	TR 0103 4112	36.925
E20730018	TR 0103 4113	36.925
E20730019	TR 0103 4114	36.925
E20730020	TR 0103 4115	36.925
E20730021	TR 0103 4116	36.925
E20730022	TR 0103 4117	36.925
E20730023	TR 0103 4118	36.925
E20730024	TR 0103 4119	36.925
E20730025	TR 0103 4120	36.925
E20730026	TR 0103 4121	36.925
E20730027	TR 0103 4122	36.925
E20730028	TR 0103 4123	36.925
E20730029	TR 0103 4124	36.925
E20730030	TR 0103 4125	36.925
E20730031	TR 0103 4126	36.925
E20730032	TR 0103 4127	36.925
E20730033	TR 0103 4128	36.925
E20730034	TR 0103 4129	36.925
E20730035	TR 0103 4130	36.925
E20730036	TR 0103 4131	36.925
E20730037	TR 0103 4132	36.925
E20730038	TR 0103 4133	36.925
E20730039	TR 0103 4134	36.925
E20730040	TR 0103 4135	36.925
E20730041	TR 0103 4136	36.925
E20730042	TR 0103 4137	36.925
E20730043	TR 0103 4138	36.925
E20730044	TR 0103 4139	36.925
E20730045	TR 0103 4140	36.925
E20730046	TR 0103 4141	36.925
E20730047	TR 0103 4142	36.925
E20730048	TR 0103 4143	36.925
E20730049	TR 0103 4144	36.925
E20730050	TR 0103 4145	36.925



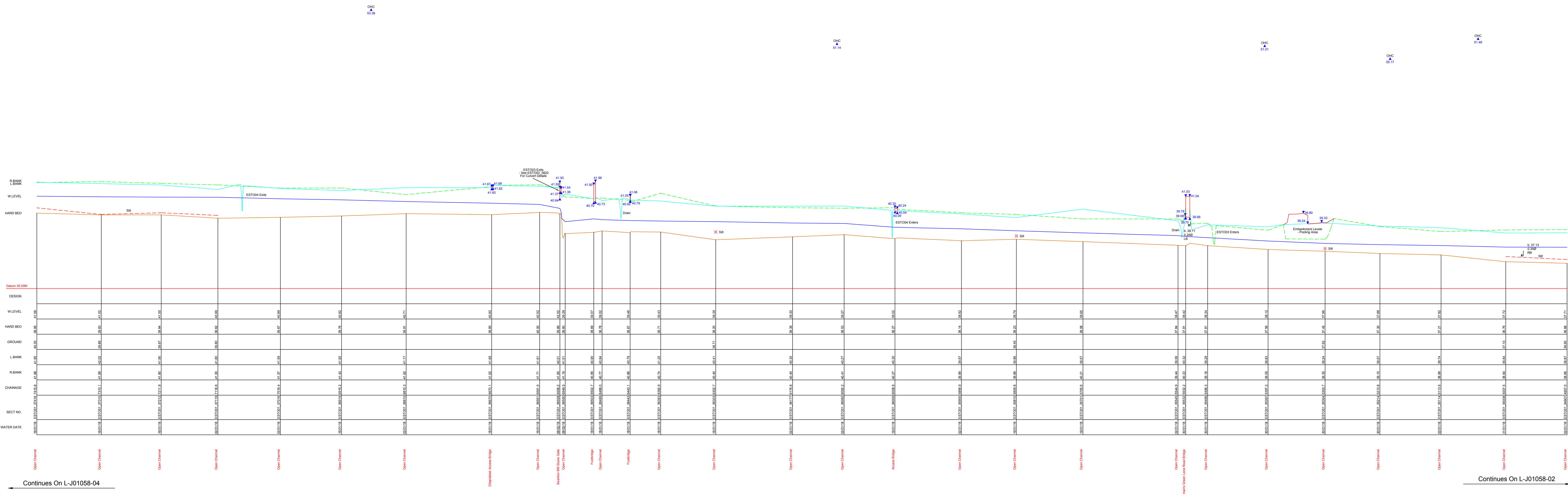
Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Ordnance Park, London Road, Addlestone, West Malling, Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
EAST STOUR (EST001)
LONG SECTION & LOCATION PLAN
EST001_00033 TO EST001_02500

SURVEYED BY: MALTBY LAND SURVEYS LTD *Rev 12_157*
SURVEY DATE: DECEMBER 2018 – MARCH 2018
SCALE: AS SHOWN DRN: RC CHKD: ITS
DATUM: OS GPS ACTIVE DATE: MAR 18 DATE: MAR 18
GRID: NATIONAL GRID DRAWING NO. L-J01058-01

SCALE = 1:2500 H, 1:100 V



Original Drawing Size: A0

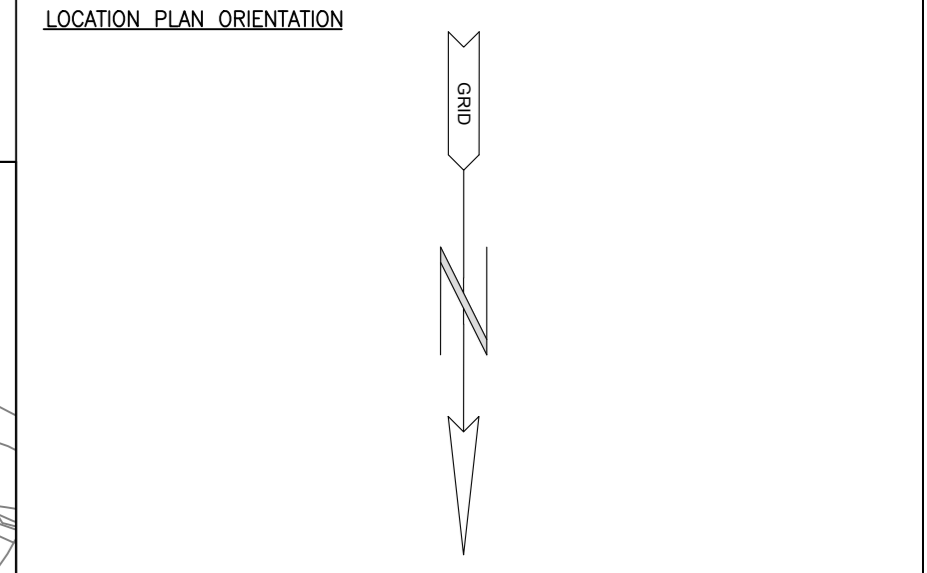
KEY TO SECTIONS:

- Water Level (Blue line)
- Visible Bed (Top of Silt) and Ground (Dashed Red line)
- Hard Bed (Determined by Probing) (Orange line)
- Bank Crest (Dashed Green line)

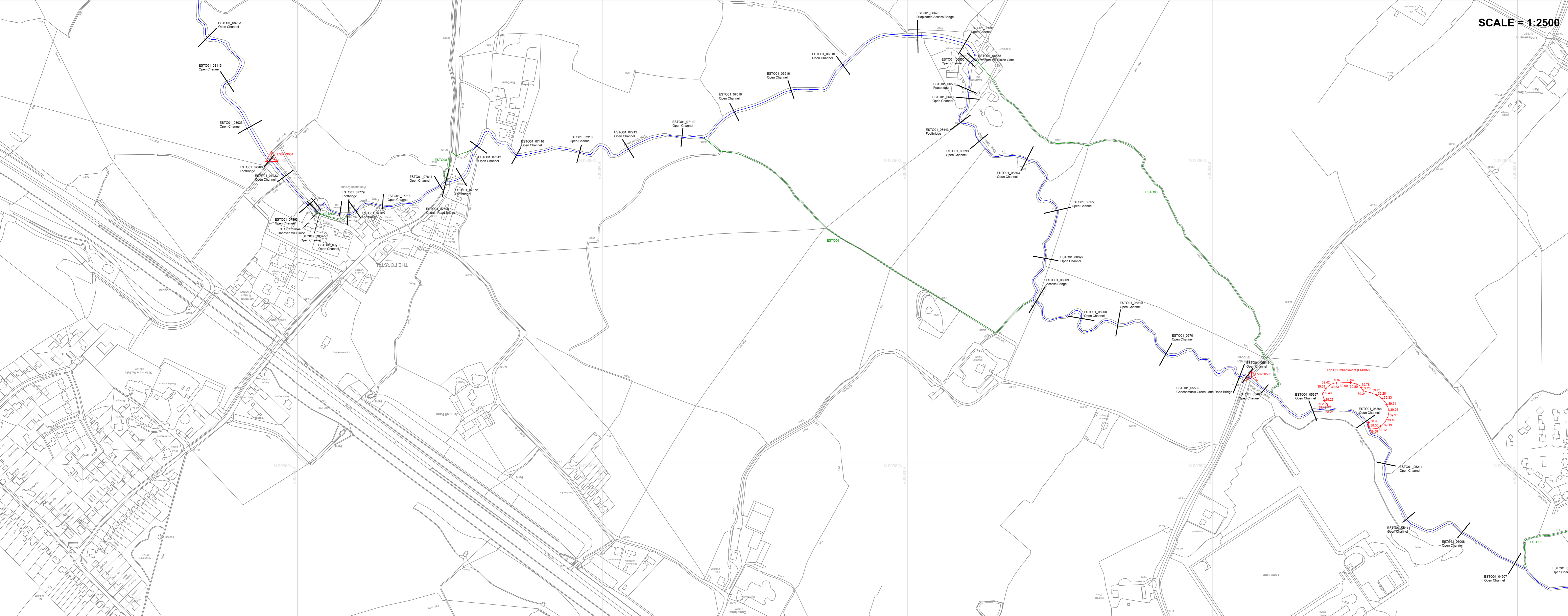
KEY TO LONGITUDINAL SECTION ONLY:

- Left Bank Crest (Dashed Green line)
- Right Bank Crest (Dashed Green line)
- Points Indicated by 'C' on Cross Sections and Additional Points Between Sections (Red '+' symbols)
- Gauge Board Location (Red '+' symbol)
- Top of Embankment (Dashed Green line)

SHEET LAYOUT:



SCALE = 1:2500



NOTES:

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- UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND:

SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
AS	AS BENCH	FB	FIELD BOOK
AW	AW BENCH	FM	FIELD MARK
BR	BROADWAY	FR	FIELD ROAD
BS	BANK SIGN	FS	FIELD SIGN
BT	BANK TOP	FT	FIELD TOP
BU	BANK UNDER	FW	FIELD WALL
BV	BANK VERT	FX	FIELD CROSS
CA	CHANNEL	FY	FIELD YARD
CB	CHANNEL BANK	FZ	FIELD ZONE
CC	CHANNEL CREST	GA	GAUGE
CD	CHANNEL DEPTH	GB	GAUGE BOARD
CE	CHANNEL ELEVATION	GC	GAUGE CHAIN
CF	CHANNEL FLOW	GD	GAUGE DRAIN
CG	CHANNEL GATE	GE	GAUGE ELEVATION
CH	CHANNEL HEDG	GF	GAUGE FLOW
CI	CHANNEL INLET	GG	GAUGE GATE
CJ	CHANNEL JUNCTION	GH	GAUGE HEIGHT
CK	CHANNEL KICK	GI	GAUGE INLET
CL	CHANNEL LIFT	GJ	GAUGE JUNCTION
CM	CHANNEL MOUND	GK	GAUGE KICK
CN	CHANNEL NARROW	GL	GAUGE LEVEL
CO	CHANNEL OVERTOP	GM	GAUGE MOUND
CP	CHANNEL POINT	GN	GAUGE NARROW
CQ	CHANNEL RAMP	GO	GAUGE OVERTOP
CR	CHANNEL ROAD	GP	GAUGE RAMP
CS	CHANNEL SIGN	GQ	GAUGE ROAD
CT	CHANNEL SIGN	GR	GAUGE SIGN
CU	CHANNEL SIGN	GS	GAUGE SIGN
CV	CHANNEL SIGN	GT	GAUGE SIGN
CW	CHANNEL SIGN	GU	GAUGE SIGN
CX	CHANNEL SIGN	GV	GAUGE SIGN
CY	CHANNEL SIGN	GW	GAUGE SIGN
CZ	CHANNEL SIGN	GX	GAUGE SIGN
DA	DRAIN	GY	GAUGE SIGN
DB	DRAIN	GA	GAUGE SIGN
DC	DRAIN	GB	GAUGE SIGN
DD	DRAIN	GC	GAUGE SIGN
DE	DRAIN	GD	GAUGE SIGN
DF	DRAIN	GE	GAUGE SIGN
DG	DRAIN	GF	GAUGE SIGN
DH	DRAIN	GF	GAUGE SIGN
DI	DRAIN	GG	GAUGE SIGN
DJ	DRAIN	GH	GAUGE SIGN
DK	DRAIN	GI	GAUGE SIGN
DL	DRAIN	GJ	GAUGE SIGN
DM	DRAIN	GK	GAUGE SIGN
DN	DRAIN	GL	GAUGE SIGN
DO	DRAIN	GM	GAUGE SIGN
DP	DRAIN	GN	GAUGE SIGN
DQ	DRAIN	GO	GAUGE SIGN
DR	DRAIN	GP	GAUGE SIGN
DS	DRAIN	GQ	GAUGE SIGN
DT <td>DRAIN</td> <td>GR</td> <td>GAUGE SIGN</td>	DRAIN	GR	GAUGE SIGN
DU	DRAIN	GS	GAUGE SIGN
DV	DRAIN	GT	GAUGE SIGN
DW	DRAIN	GU	GAUGE SIGN
DX	DRAIN	GV	GAUGE SIGN
DY	DRAIN	GW	GAUGE SIGN
DZ	DRAIN	GX	GAUGE SIGN

AMENDMENT:

NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E20730012	TR 0103 4107	36.975
E20730013	TR 0103 4108	36.975
E20730014	TR 0103 4109	36.975
E20730015	TR 0103 4110	36.975
E20730016	TR 0103 4111	36.975
E20730017	TR 0103 4112	36.975
E20730018	TR 0103 4113	36.975
E20730019	TR 0103 4114	36.975
E20730020	TR 0103 4115	36.975
E20730021	TR 0103 4116	36.975
E20730022	TR 0103 4117	36.975
E20730023	TR 0103 4118	36.975
E20730024	TR 0103 4119	36.975
E20730025	TR 0103 4120	36.975
E20730026	TR 0103 4121	36.975
E20730027	TR 0103 4122	36.975
E20730028	TR 0103 4123	36.975
E20730029	TR 0103 4124	36.975
E20730030	TR 0103 4125	36.975
E20730031	TR 0103 4126	36.975
E20730032	TR 0103 4127	36.975
E20730033	TR 0103 4128	36.975
E20730034	TR 0103 4129	36.975
E20730035	TR 0103 4130	36.975
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E20730042	TR 0103 4137	36.975
E20730043	TR 0103 4138	36.975
E20730044	TR 0103 4139	36.975
E20730045	TR 0103 4140	36.975
E20730046	TR 0103 4141	36.975
E20730047	TR 0103 4142	36.975
E20730048	TR 0103 4143	36.975
E20730049	TR 0103 4144	36.975
E20730050	TR 0103 4145	36.975

Environment Agency
 KENT & SOUTH LONDON REGION
 Ordovik House, Ordovik Park, London Road, Ashford, Kent, ME19 5QH

PROJECT/WATERCOURSE:
 EAST STOUR, ASHFORD TO STANFORD

SITE/LIMITS:
 EAST STOUR (EST001)
 LONG SECTION & LOCATION PLAN
 EST001_04907 TO EST001_07416

SURVEYED BY: MALTYBY LAND SURVEYS LTD *Ref: 12_157*

SURVEY DATE: DECEMBER 2018 – MARCH 2018

SCALE: AS SHOWN **DRN:** RC **CHKD:** ITS

DATUM: OS GPS ACTIVE **DATE:** MAR 18 **DATE:** MAR 18

GRID: NATIONAL GRID **DRAWING NO.:** L-J01058-03 **REV.:** 1

CAD FILENAME: L-J01058-03.dwg

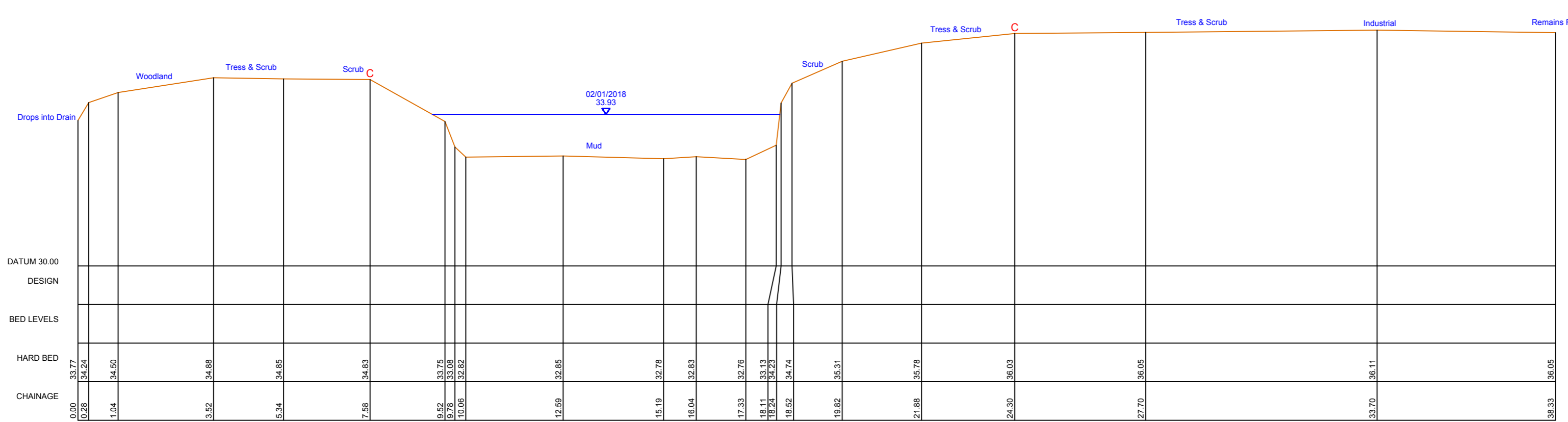
KEY TO SECTIONS:

- WATER LEVEL
- VISIBLE BED (TOP OF SILT) AND GROUND
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

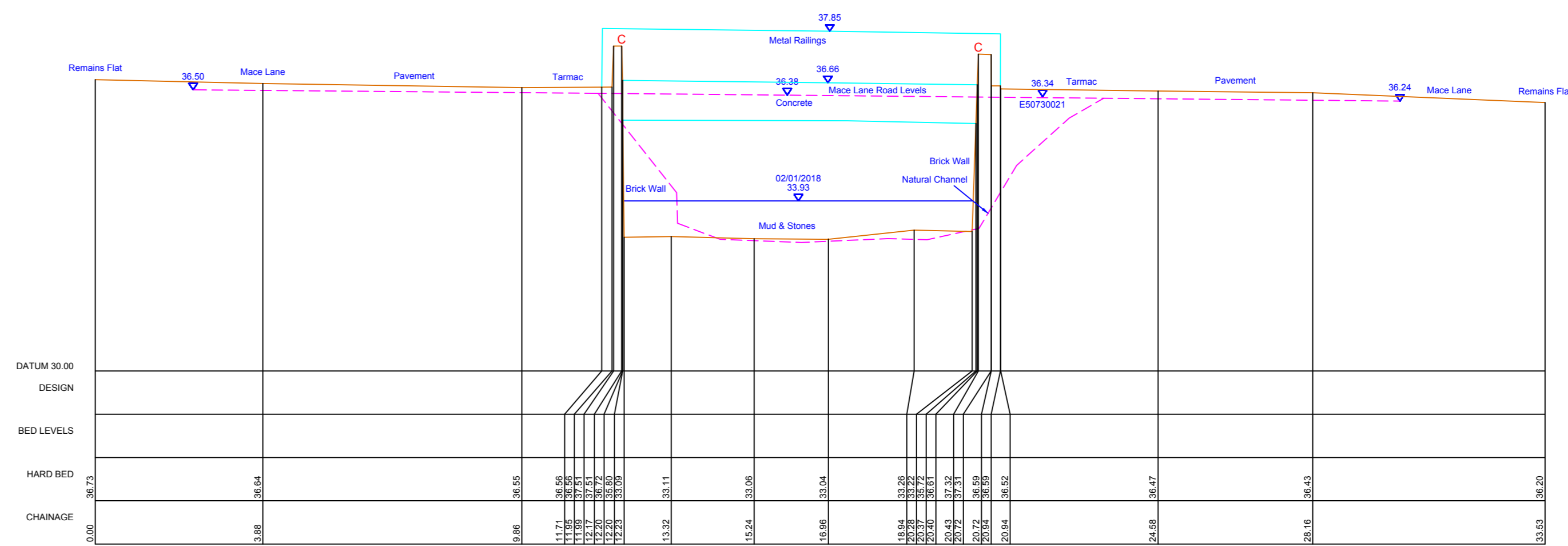
KEY TO LONGITUDINAL SECTION ONLY:

- VIEWED LOOKING DOWNSTREAM
- LEFT BANK CREST
- RIGHT BANK CREST

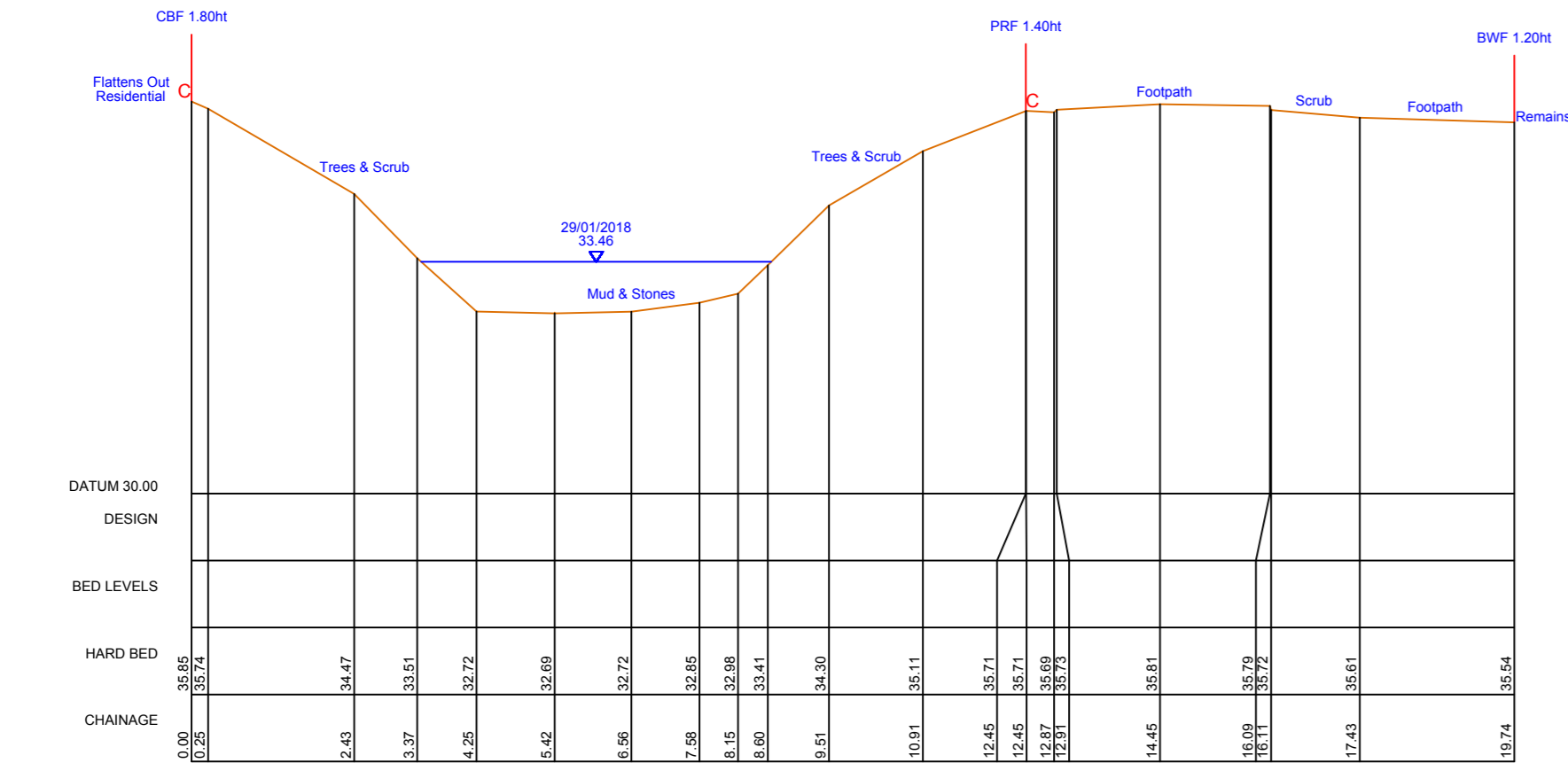
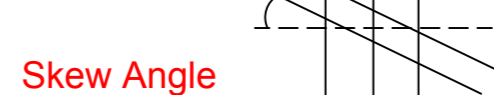
POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



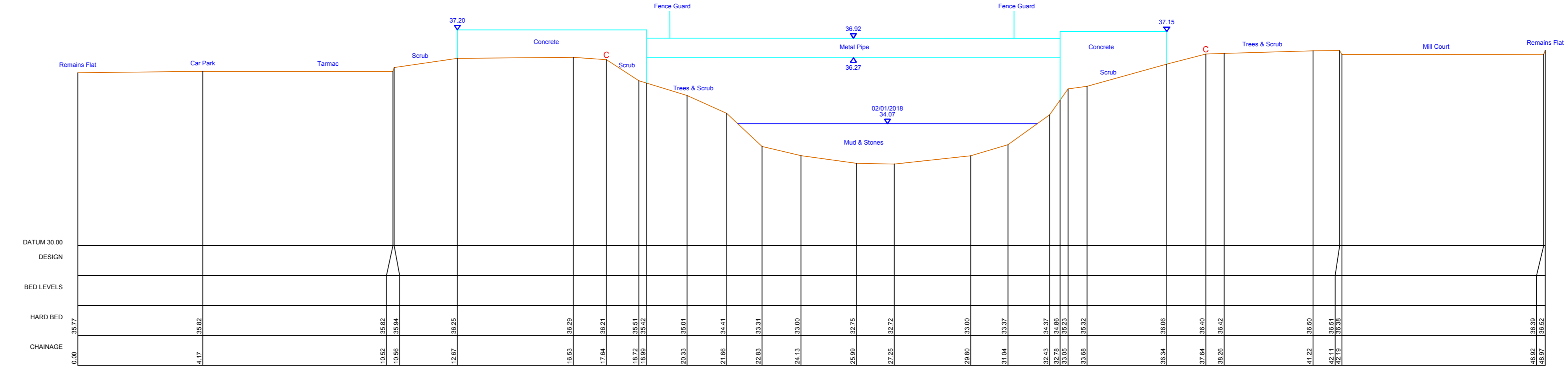
ESTO01_00033
601586.53mE 142862.68mN Brg 99
Open Channel



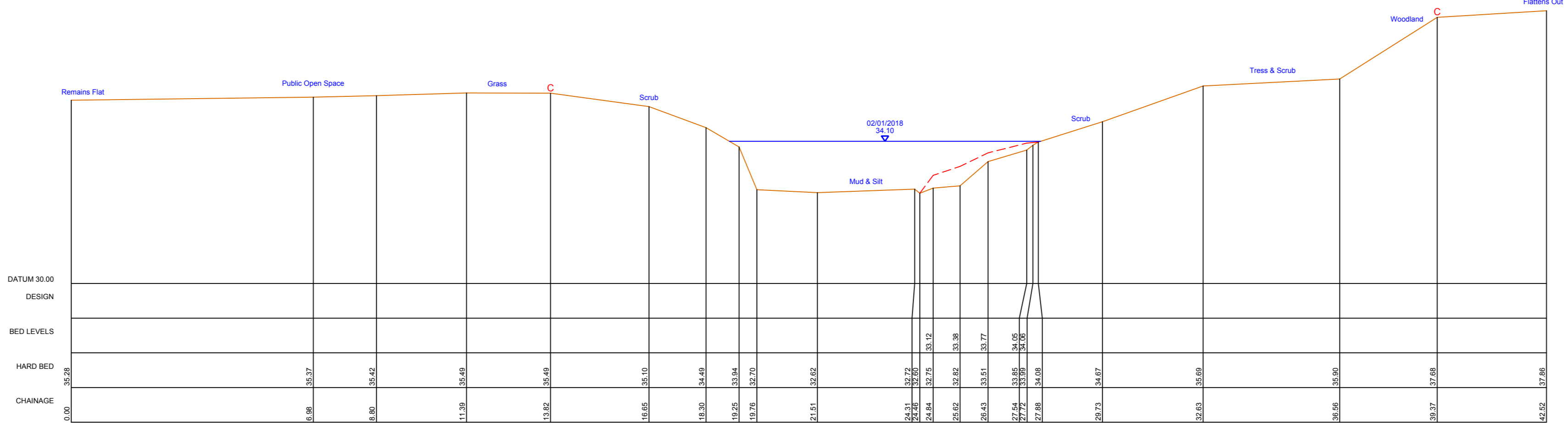
ESTO01_00080
601575.08mE 142823.29mN Brg 119
Mace Lane Road Bridge
Tunnel Length = 26.80m
Skew Angle = 15°



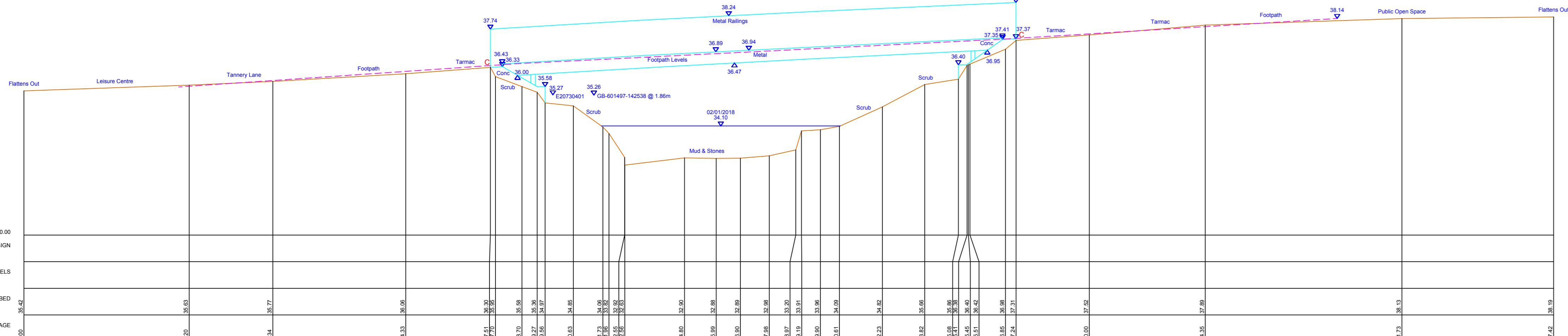
ESTO01_00109
601569.7mE 142793.75mN Brg 128
Open Channel



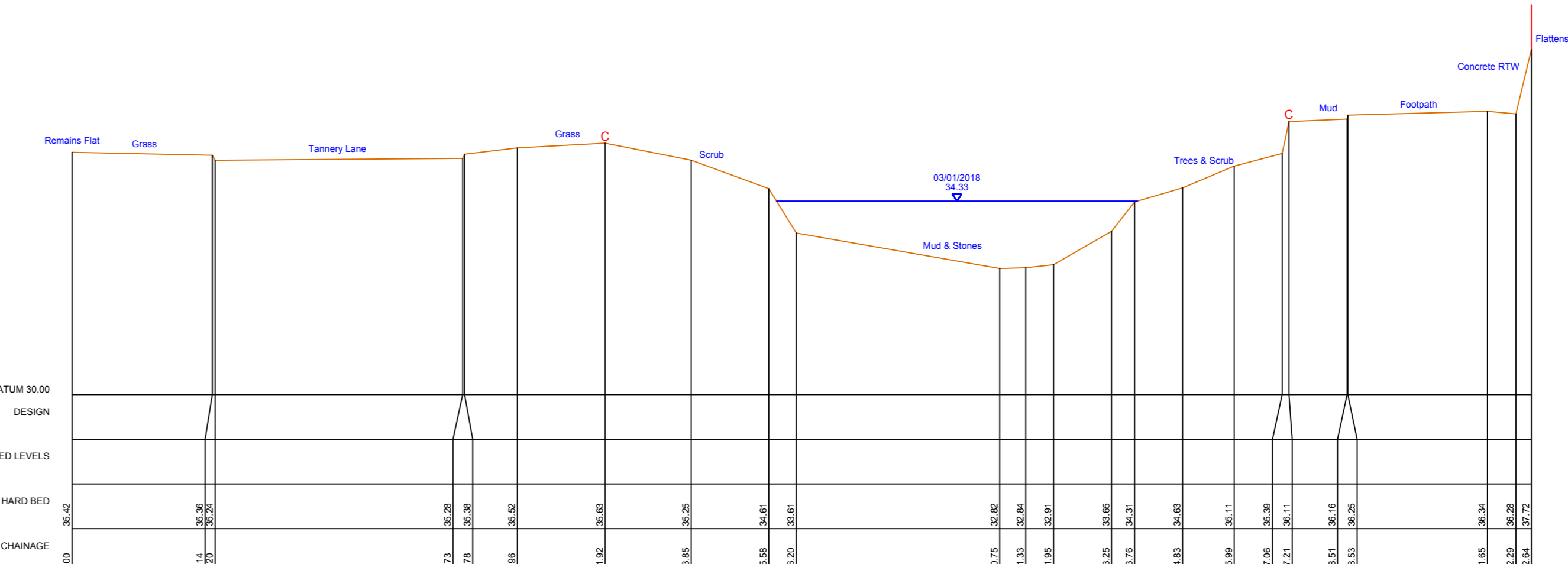
ESTO01_00192
601523.91mE 142721.49mN Brg 106
Pipe Crossing
Tunnel Length = 0.65m



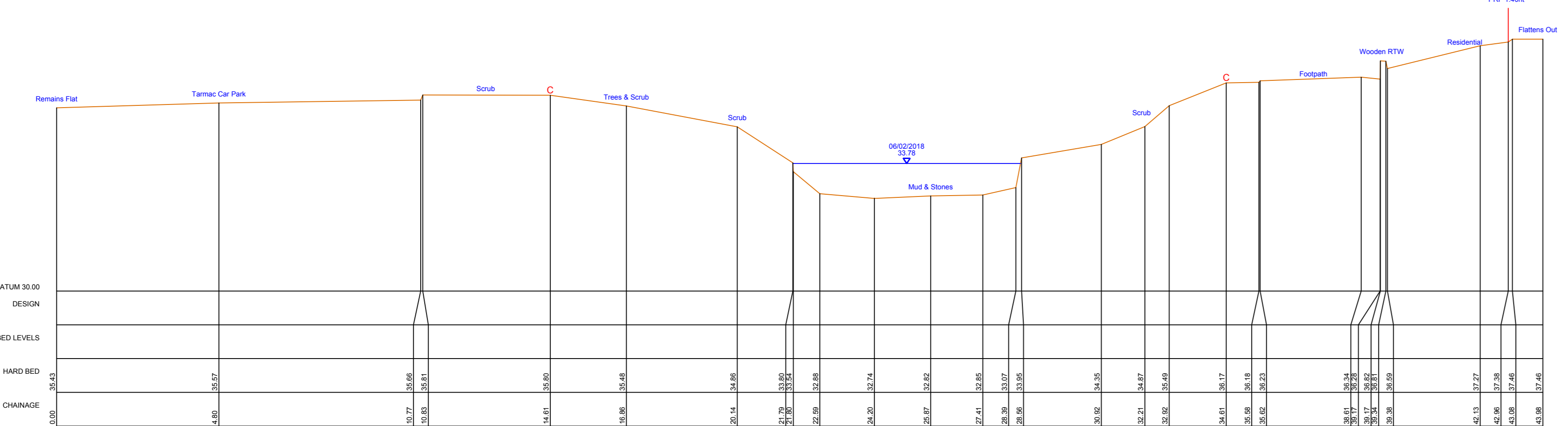
ESTO01_00350
601485.52mE 142567.55mN Brg 103
Open Channel



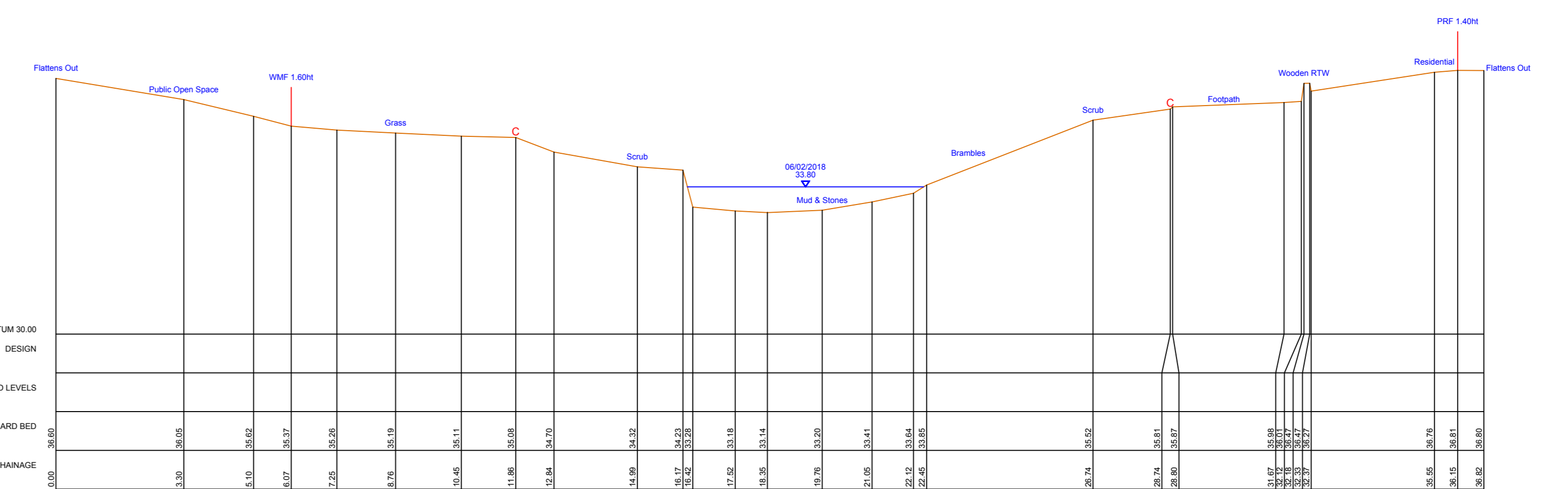
ESTO01_00367
601473.72mE 142552.38mN Brg 105
Footbridge
Tunnel Length = 3.18m



ESTO01_00402
601479.13mE 142512.41mN Brg 95
Open Channel



ESTO01_00502
601490.83mE 142402.14mN Brg 63
Open Channel



ESTO01_00592
601551.61mE 142322.86mN Brg 33
Open Channel

NOTES:

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- UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

AS	AS BENCH	FW	THE CHANNEL	FW	THE CHANNEL	FW	THE CHANNEL	FW	THE CHANNEL
...

AMENDMENT

NO	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E0720012	TR 0103 4107	35.9275
E0720405	TR 0299 4227	36.2890
E0720019	TR 0199 4202	36.4890
E0720001	TR 0199 4201	36.4890
E0720002	TR 0199 4202	36.4890
E0720003	TR 0199 4203	36.4890
E0720004	TR 0199 4204	36.4890
E0720005	TR 0199 4205	36.4890
E0720006	TR 0199 4206	36.4890
E0720007	TR 0199 4207	36.4890
E0720008	TR 0199 4208	36.4890
E0720009	TR 0199 4209	36.4890
E0720010	TR 0199 4210	36.4890

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, London Road, Addington, West Malling, Kent, ME19 5QH

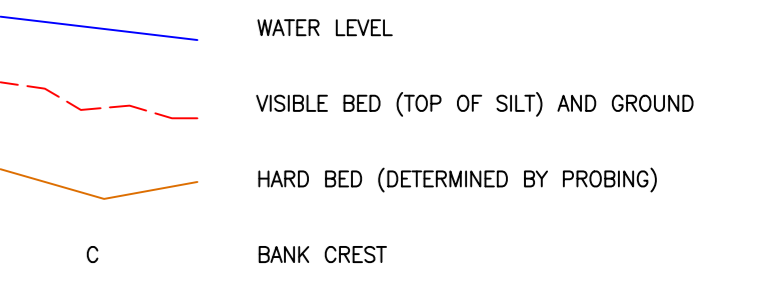
PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS:
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_00033 TO ESTO01_00592

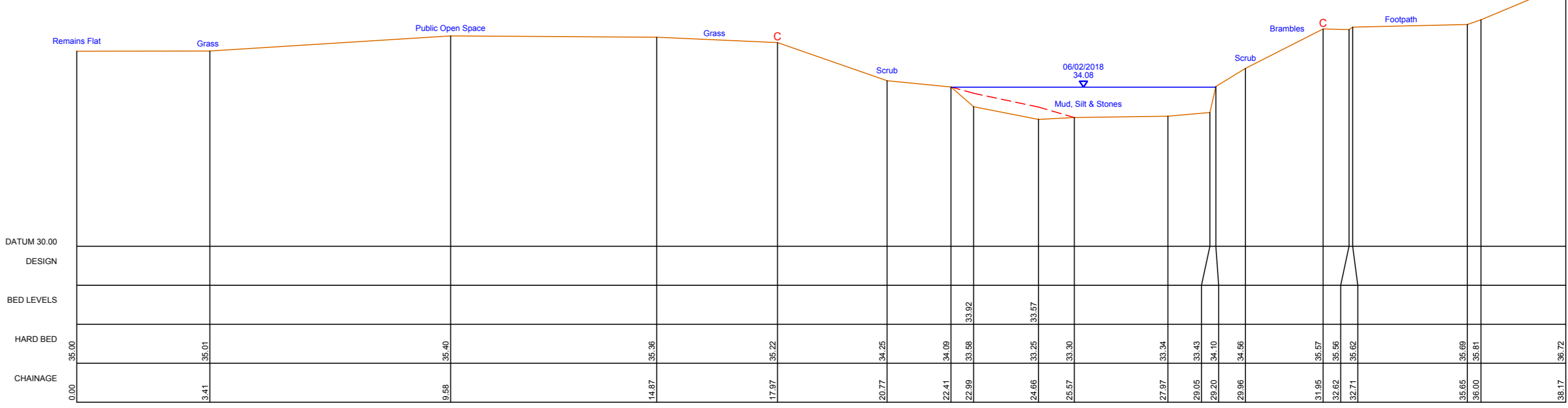
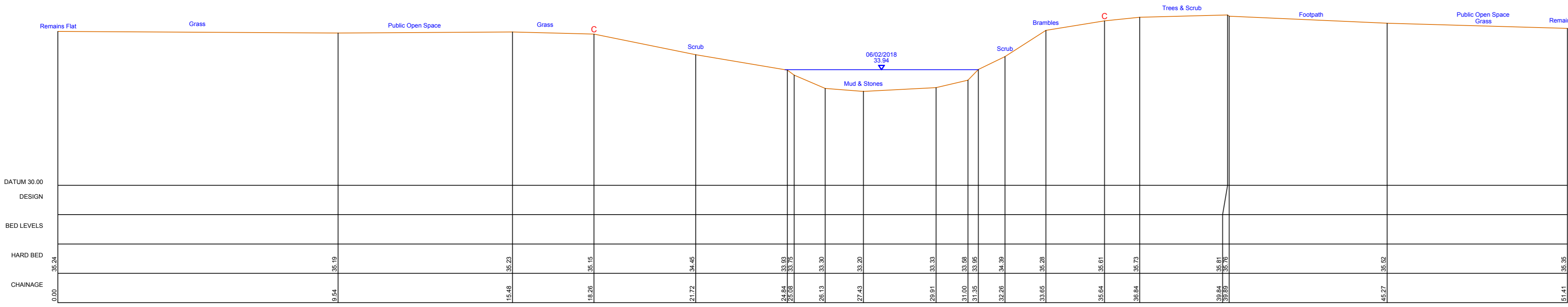
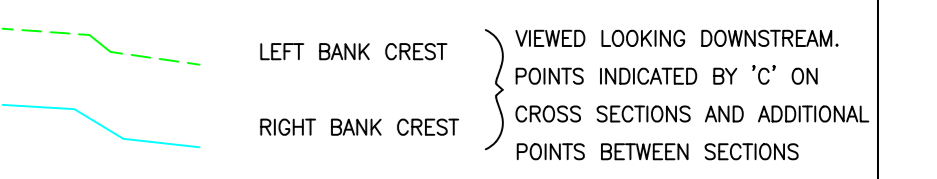
SURVEYED BY: MALTBY LAND SURVEYS LTD *Rev 12_157*
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100
DATUM: OS GPS ACTIVE
GRID: NATIONAL GRID

DRN: RC
CHKD: ITS
DATE: MAR 18
DRAWING NO.: X-J01058-01

KEY TO SECTIONS:

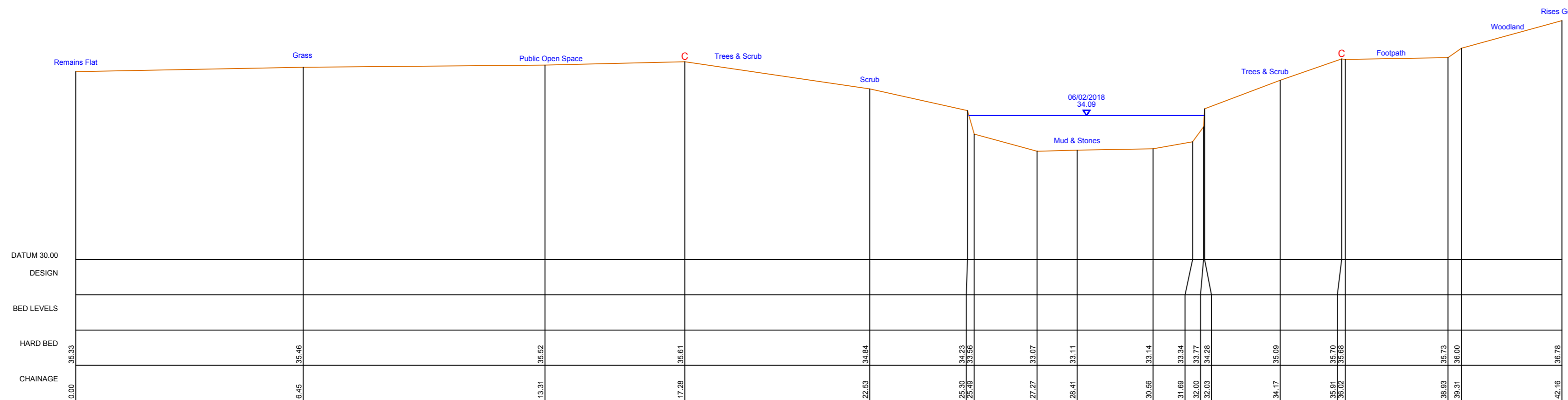


KEY TO LONGITUDINAL SECTION ONLY:

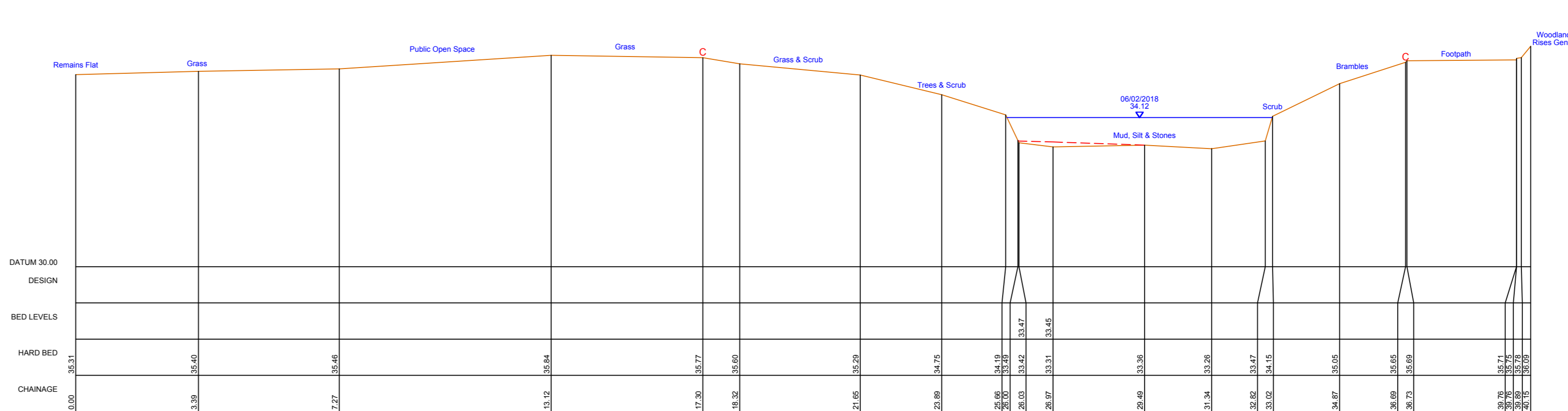


EST001_00709
601558.32mE 142250.02mN Brg 127
Open Channel

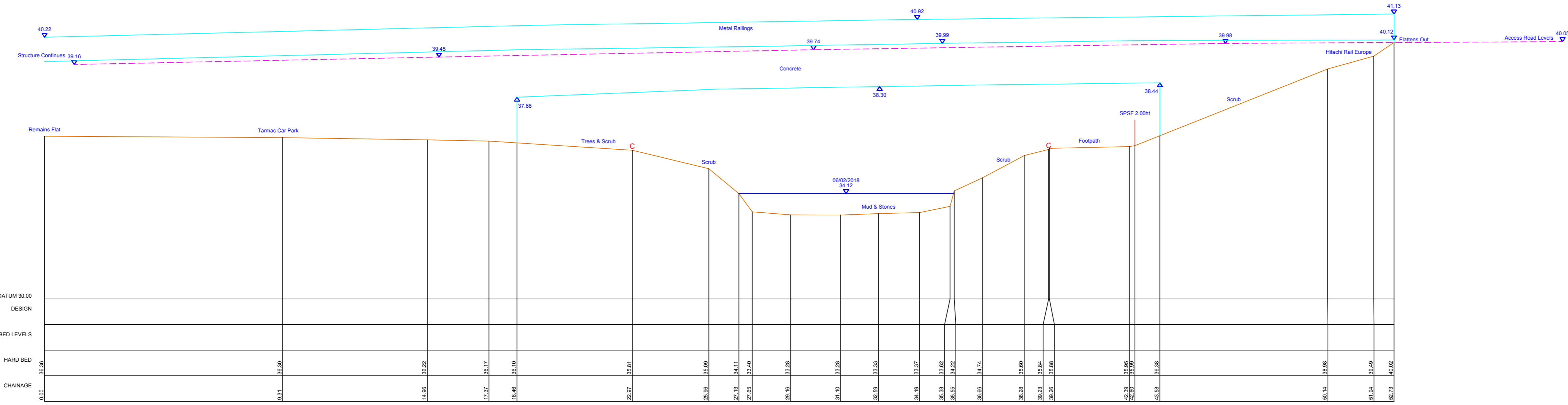
EST001_00781
601509.17mE 142209.15mN Brg 144
Open Channel



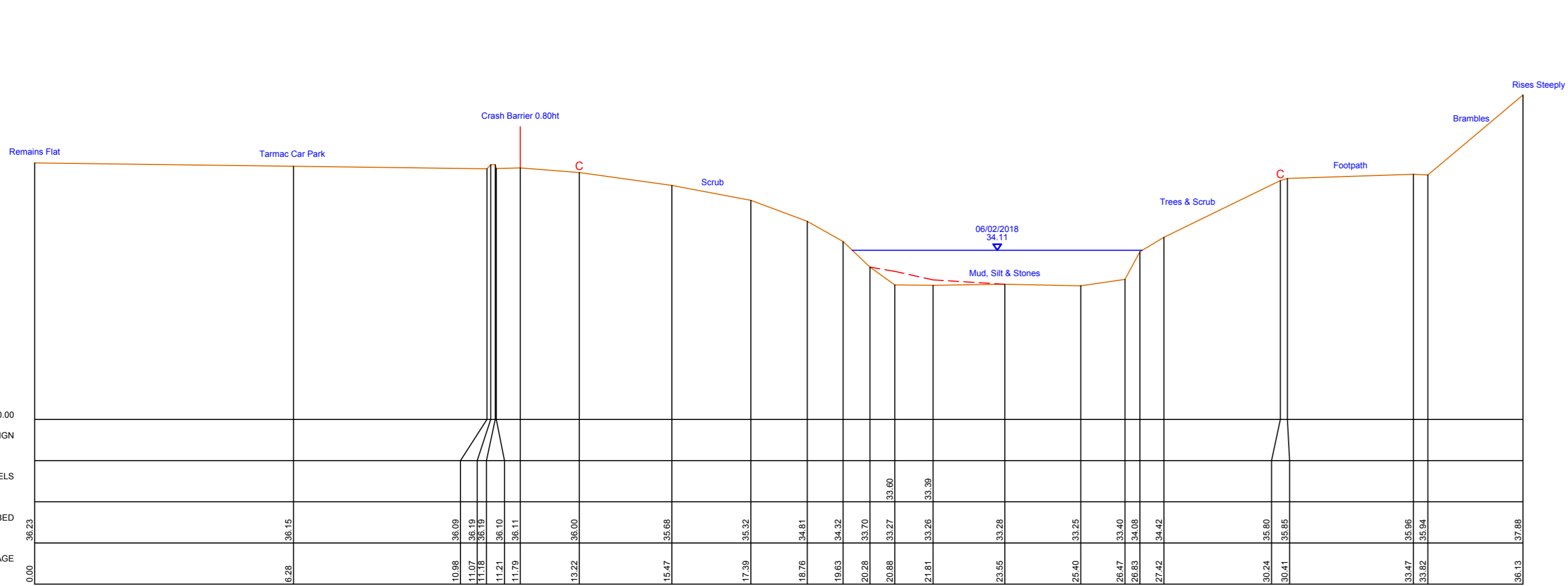
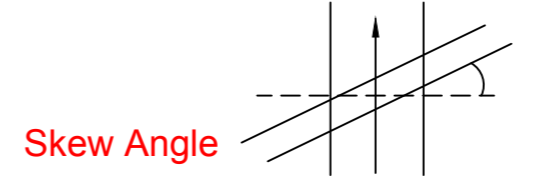
EST001_00801
601490.4mE 142200.03mN Brg 143
Open Channel



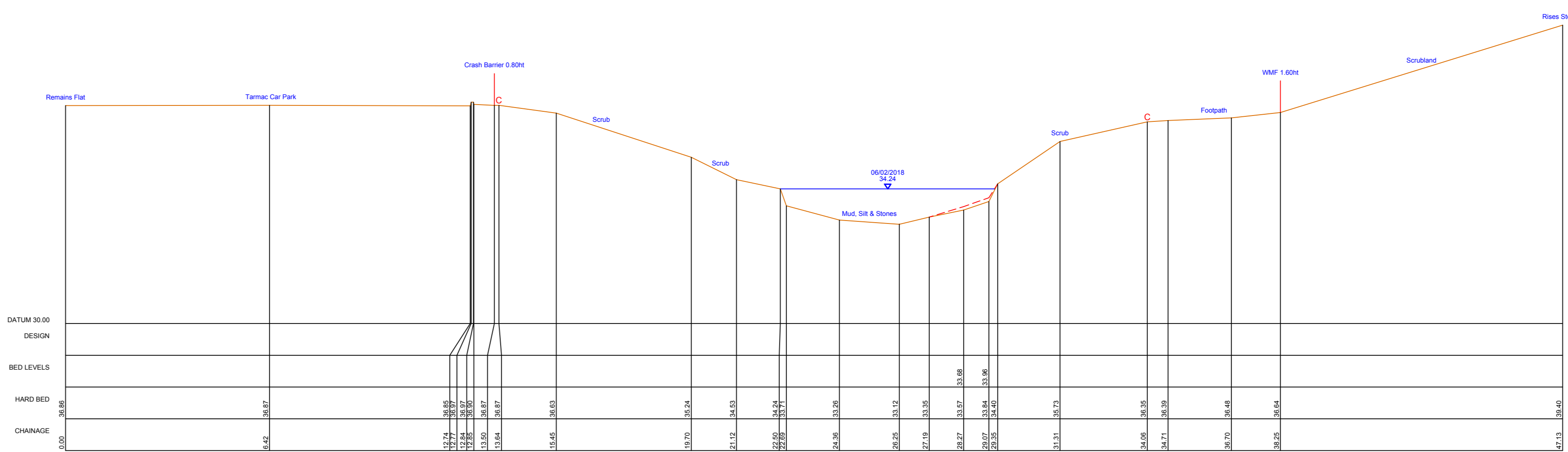
EST001_00818
601475.96mE 142190.76mN Brg 143
Open Channel



EST001_00844
601445.07mE 142169.61mN Brg 123
Access Road Bridge
Tunnel Length = 10.55m
Skew Angle = 20°



EST001_00852
601452.72mE 142168.03mN Brg 147
Open Channel



EST001_00934
601383.31mE 142120.35mN Brg 141
Open Channel

NOTES: 1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

AS	AS BENCH	FM	THE CHANNEL	HW	HIGH WATER
AB	AS BANK	FR	RIVER FRONT	HT	HIGH TIDE
BB	BANK BOUNDARY	FN	RIVER FRONT	HT	HIGH TIDE
BS	BANK SIGN	FW	WATER FLOW	HT	HIGH TIDE
BS	BANK SIGN	FW	WATER FLOW	HT	HIGH TIDE
BS	BANK SIGN	FW	WATER FLOW	HT	HIGH TIDE

AMENDMENT

NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E00230012	TR 0103 4107	35.975
E00230013	TR 0299 4227	36.480
E00230014	TR 0199 4202	36.480
E00230015	TR 0199 4202	36.480
E00230016	TR 0126 4199	36.480
E00230017	TR 0126 4199	36.480
E00230018	TR 0126 4199	36.480
E00230019	TR 0126 4199	36.480
E00230020	TR 0126 4199	36.480
E00230021	TR 0126 4199	36.480
E00230022	TR 0126 4199	36.480
E00230023	TR 0126 4199	36.480
E00230024	TR 0126 4199	36.480
E00230025	TR 0126 4199	36.480
E00230026	TR 0126 4199	36.480
E00230027	TR 0126 4199	36.480
E00230028	TR 0126 4199	36.480
E00230029	TR 0126 4199	36.480
E00230030	TR 0126 4199	36.480
E00230031	TR 0126 4199	36.480
E00230032	TR 0126 4199	36.480
E00230033	TR 0126 4199	36.480
E00230034	TR 0126 4199	36.480
E00230035	TR 0126 4199	36.480
E00230036	TR 0126 4199	36.480
E00230037	TR 0126 4199	36.480
E00230038	TR 0126 4199	36.480
E00230039	TR 0126 4199	36.480
E00230040	TR 0126 4199	36.480
E00230041	TR 0126 4199	36.480
E00230042	TR 0126 4199	36.480
E00230043	TR 0126 4199	36.480
E00230044	TR 0126 4199	36.480
E00230045	TR 0126 4199	36.480
E00230046	TR 0126 4199	36.480
E00230047	TR 0126 4199	36.480
E00230048	TR 0126 4199	36.480
E00230049	TR 0126 4199	36.480
E00230050	TR 0126 4199	36.480

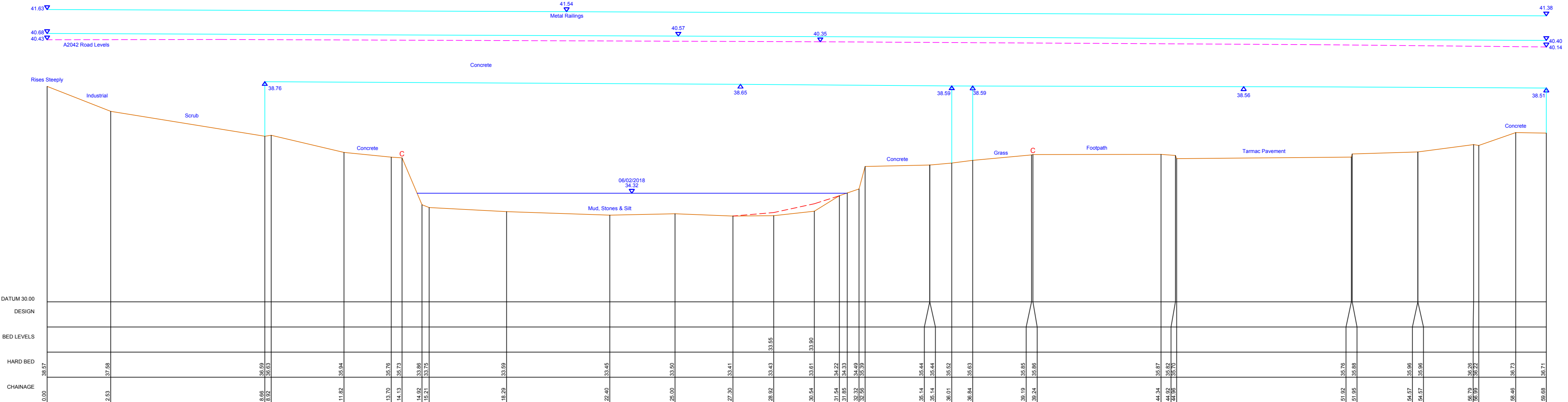
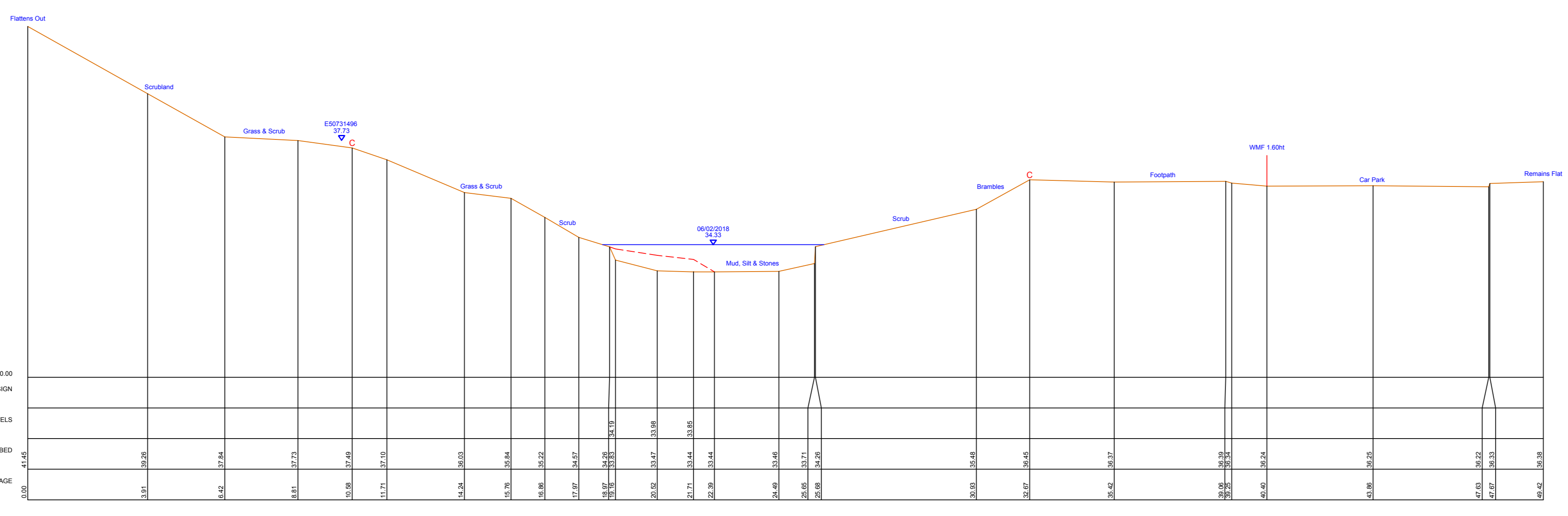
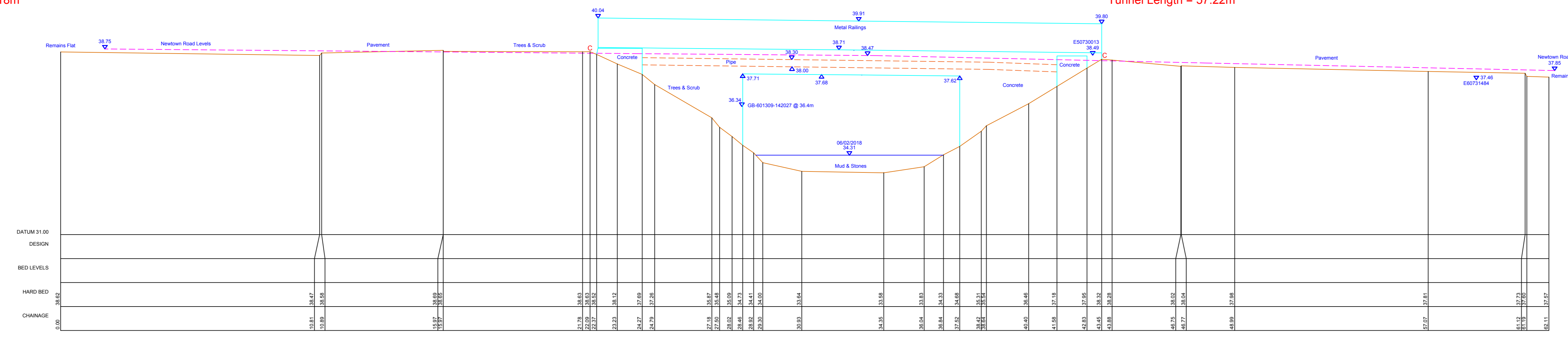
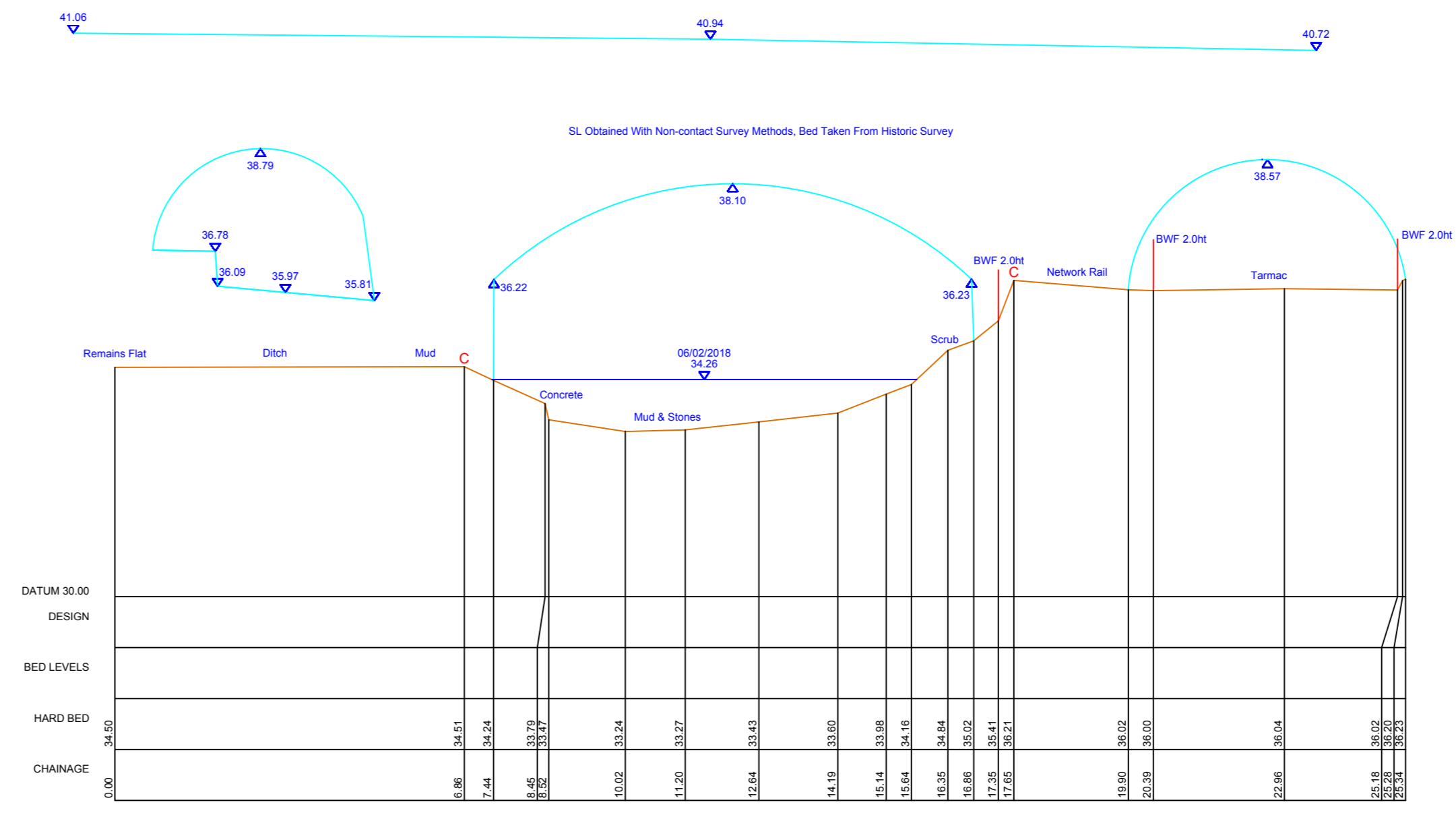
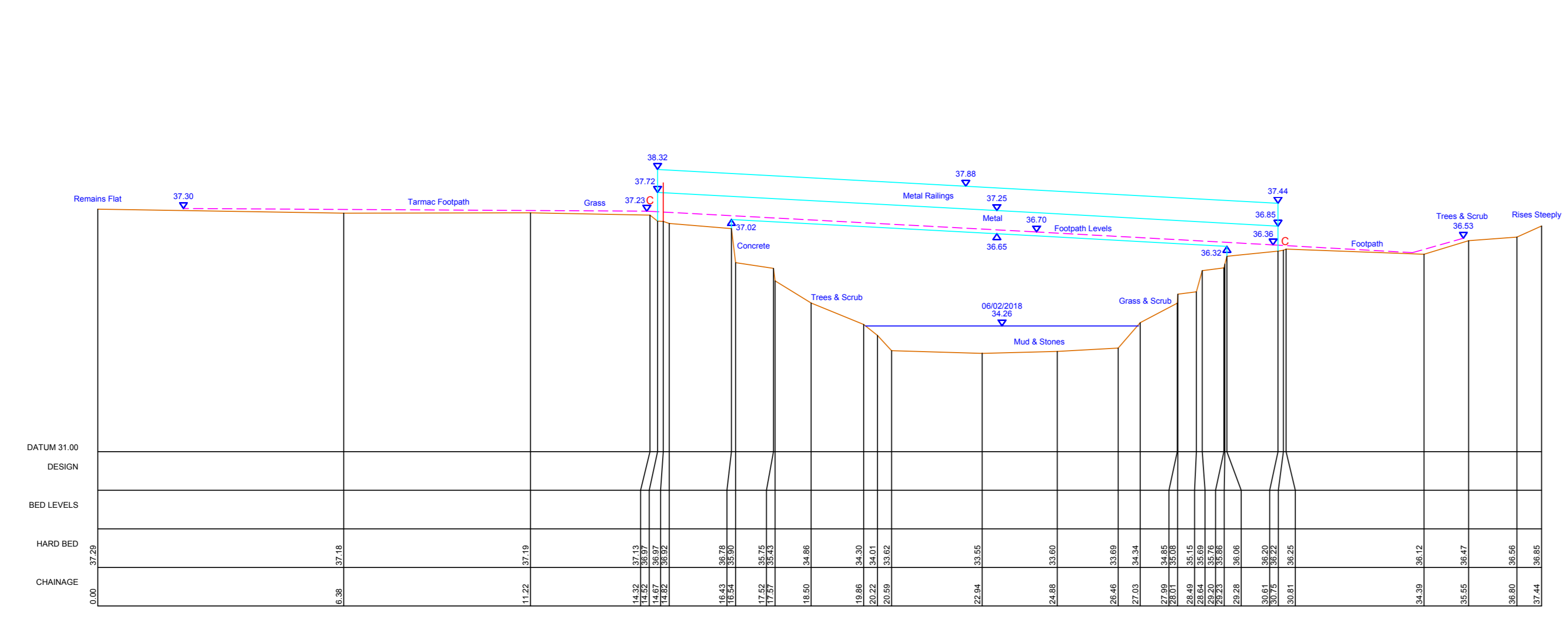
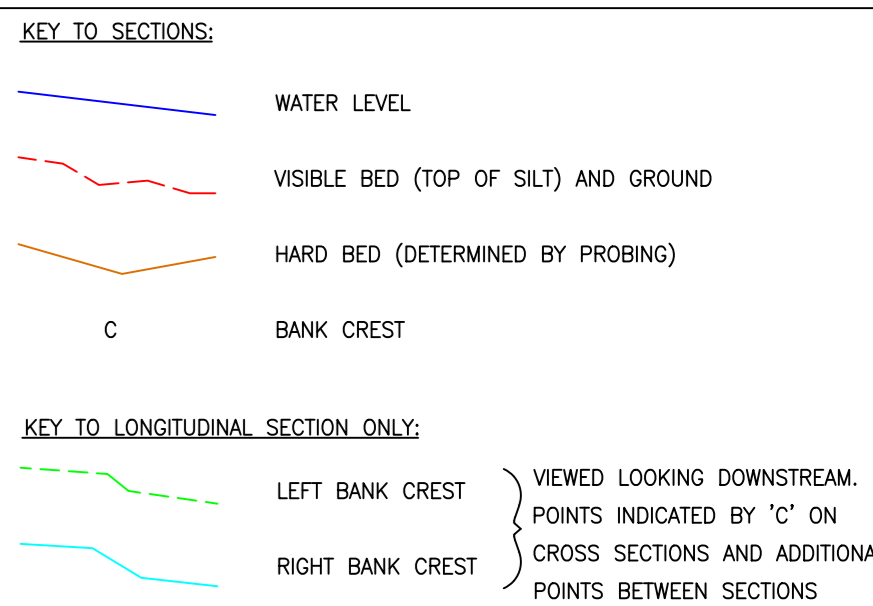
Environment Agency
KENT & SOUTH LONDON REGION
Orford House, Ordeans Park, London Road, Addington, West Malling, Kent, ME19 5SH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
EAST STOUR (EST001)
CROSS SECTIONS
EST001_00709 TO EST001_00934

SURVEYED BY: MALTBY LAND SURVEYS LTD
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100
DATING: OS GPS ACTIVE
GRID: NATIONAL GRID

Rev: 12_157
DRN: RC
CHKD: ITS
DATE: MAR 18
DATE: MAR 18
DRAWING NO.: X-J01058-02
REV.



NOTES:

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- UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

AB	AS BECK	FW	THE CHANNEL	FW	FRONT WALL
AP	AS POND	FW	FRONT WALL	FW	FRONT WALL
AR	AS ROAD	FW	FRONT WALL	FW	FRONT WALL
AS	AS STRUCTURE	FW	FRONT WALL	FW	FRONT WALL
AT	AS TUNNEL	FW	FRONT WALL	FW	FRONT WALL
AV	AS VALVE	FW	FRONT WALL	FW	FRONT WALL
AW	AS WALL	FW	FRONT WALL	FW	FRONT WALL
AX	AS BOX	FW	FRONT WALL	FW	FRONT WALL
AY	AS YARD	FW	FRONT WALL	FW	FRONT WALL
AZ	AS ZONE	FW	FRONT WALL	FW	FRONT WALL
BA	AS BANK	FW	FRONT WALL	FW	FRONT WALL
BB	AS BRIDGE	FW	FRONT WALL	FW	FRONT WALL
BC	AS CREST	FW	FRONT WALL	FW	FRONT WALL
BD	AS DAM	FW	FRONT WALL	FW	FRONT WALL
BE	AS EMBANKMENT	FW	FRONT WALL	FW	FRONT WALL
BF	AS FENCE	FW	FRONT WALL	FW	FRONT WALL
BG	AS GATE	FW	FRONT WALL	FW	FRONT WALL
BH	AS HAULAGE	FW	FRONT WALL	FW	FRONT WALL
BI	AS INFILL	FW	FRONT WALL	FW	FRONT WALL
BJ	AS JUNCTION	FW	FRONT WALL	FW	FRONT WALL
BK	AS KYLE	FW	FRONT WALL	FW	FRONT WALL
BL	AS LIFT	FW	FRONT WALL	FW	FRONT WALL
BM	AS MOUND	FW	FRONT WALL	FW	FRONT WALL
BN	AS NILE	FW	FRONT WALL	FW	FRONT WALL
BO	AS OUPPE	FW	FRONT WALL	FW	FRONT WALL
BP	AS POND	FW	FRONT WALL	FW	FRONT WALL
BQ	AS QUAY	FW	FRONT WALL	FW	FRONT WALL
BR	AS RACE	FW	FRONT WALL	FW	FRONT WALL
BS	AS SLOPE	FW	FRONT WALL	FW	FRONT WALL
BT	AS TUNNEL	FW	FRONT WALL	FW	FRONT WALL
BU	AS UPP	FW	FRONT WALL	FW	FRONT WALL
BV	AS VALVE	FW	FRONT WALL	FW	FRONT WALL
BW	AS WALL	FW	FRONT WALL	FW	FRONT WALL
BX	AS BOX	FW	FRONT WALL	FW	FRONT WALL
BY	AS YARD	FW	FRONT WALL	FW	FRONT WALL
BZ	AS ZONE	FW	FRONT WALL	FW	FRONT WALL

AMENDMENT	DRN	CHD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E07230012	TR 0103 4107	35.925
E07230013	TR 0229 4227	36.280
E07230014	TR 0199 4202	36.480
E07230015	TR 0199 4202	36.480
E07230016	TR 0199 4202	36.480
E07230017	TR 0199 4202	36.480
E07230018	TR 0199 4202	36.480
E07230019	TR 0199 4202	36.480
E07230020	TR 0199 4202	36.480
E07230021	TR 0199 4202	36.480
E07230022	TR 0199 4202	36.480
E07230023	TR 0199 4202	36.480
E07230024	TR 0199 4202	36.480
E07230025	TR 0199 4202	36.480
E07230026	TR 0199 4202	36.480
E07230027	TR 0199 4202	36.480
E07230028	TR 0199 4202	36.480
E07230029	TR 0199 4202	36.480
E07230030	TR 0199 4202	36.480
E07230031	TR 0199 4202	36.480
E07230032	TR 0199 4202	36.480
E07230033	TR 0199 4202	36.480
E07230034	TR 0199 4202	36.480
E07230035	TR 0199 4202	36.480
E07230036	TR 0199 4202	36.480
E07230037	TR 0199 4202	36.480
E07230038	TR 0199 4202	36.480
E07230039	TR 0199 4202	36.480
E07230040	TR 0199 4202	36.480
E07230041	TR 0199 4202	36.480
E07230042	TR 0199 4202	36.480
E07230043	TR 0199 4202	36.480
E07230044	TR 0199 4202	36.480
E07230045	TR 0199 4202	36.480
E07230046	TR 0199 4202	36.480
E07230047	TR 0199 4202	36.480
E07230048	TR 0199 4202	36.480
E07230049	TR 0199 4202	36.480
E07230050	TR 0199 4202	36.480
E07230051	TR 0199 4202	36.480
E07230052	TR 0199 4202	36.480
E07230053	TR 0199 4202	36.480
E07230054	TR 0199 4202	36.480
E07230055	TR 0199 4202	36.480
E07230056	TR 0199 4202	36.480
E07230057	TR 0199 4202	36.480
E07230058	TR 0199 4202	36.480
E07230059	TR 0199 4202	36.480
E07230060	TR 0199 4202	36.480

Environment Agency
KENT & SOUTH LONDON REGION
Orchard House, Endeavour Park, London Road, Ashford, Kent, ME19 5SH

PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS:
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_00961 TO ESTO01_01137

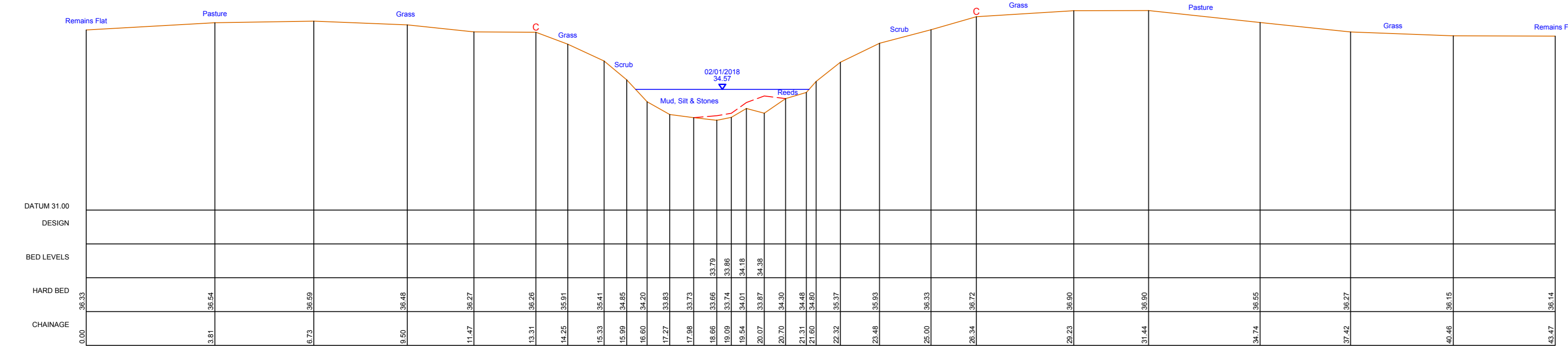
SURVEYED BY: MALBY LAND SURVEYS LTD *Ref: 12_152*
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100 **DRN:** RC **CHKD:** ITS
DATUM: OS GPS ACTIVE **DATE:** MAR 18 **DATE:** MAR 18
GRID: NATIONAL GRID **DRAWING NO.:** X-J01058-03 **REV.:** 1
CAD FILENAME: X-01058-03.dwg

KEY TO SECTIONS:

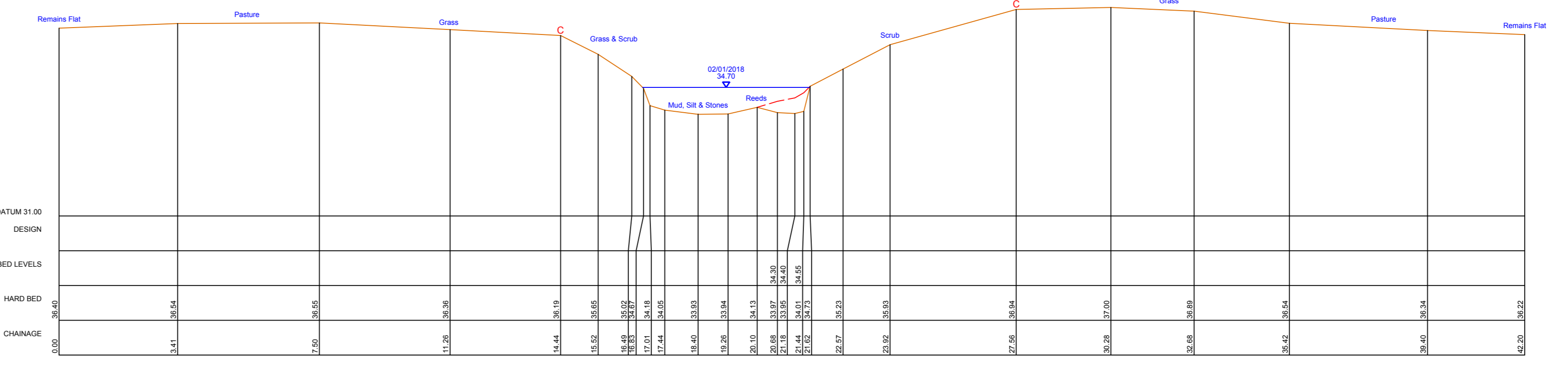
- Water Level
- Visible Bed (Top of Silt) and Ground
- Hard Bed (Determined by Probing)
- Bank Crest

KEY TO LONGITUDINAL SECTION ONLY:

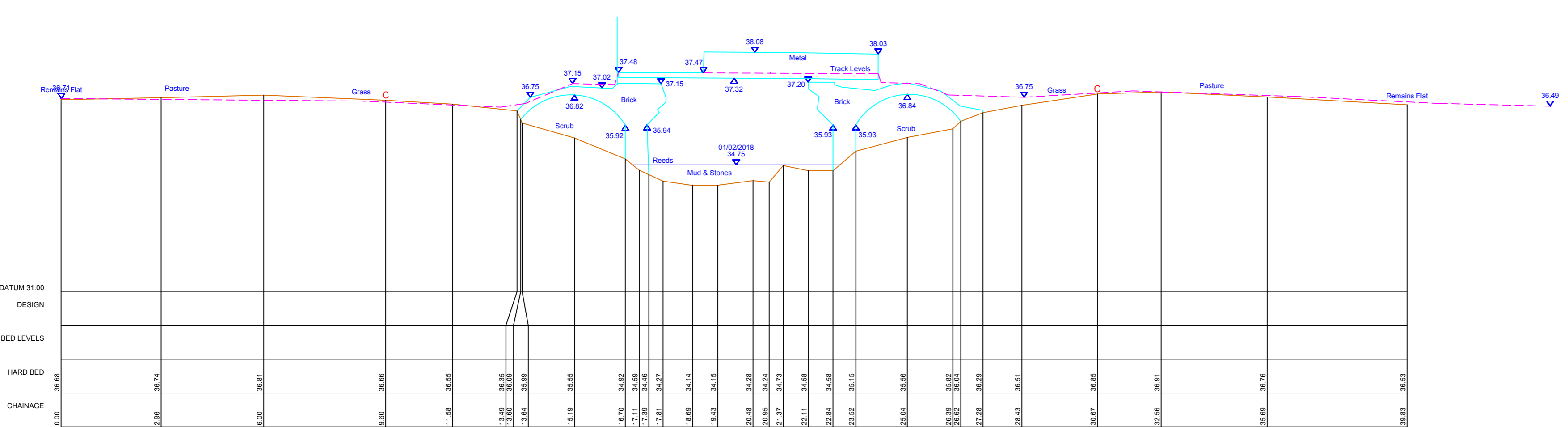
- Left Bank Crest
- Right Bank Crest
- Viewed Looking Downstream
- Points Indicated by 'C' on Cross Sections and Additional Points Between Sections



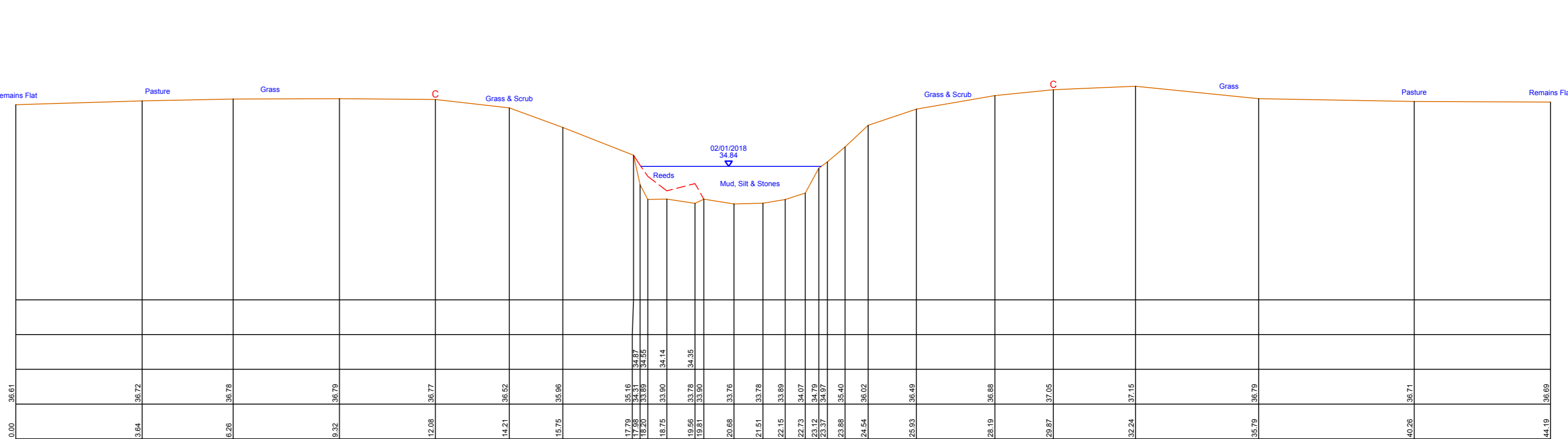
EST001_02196
601138.17mE 140955.54mN Brg 34
Open Channel



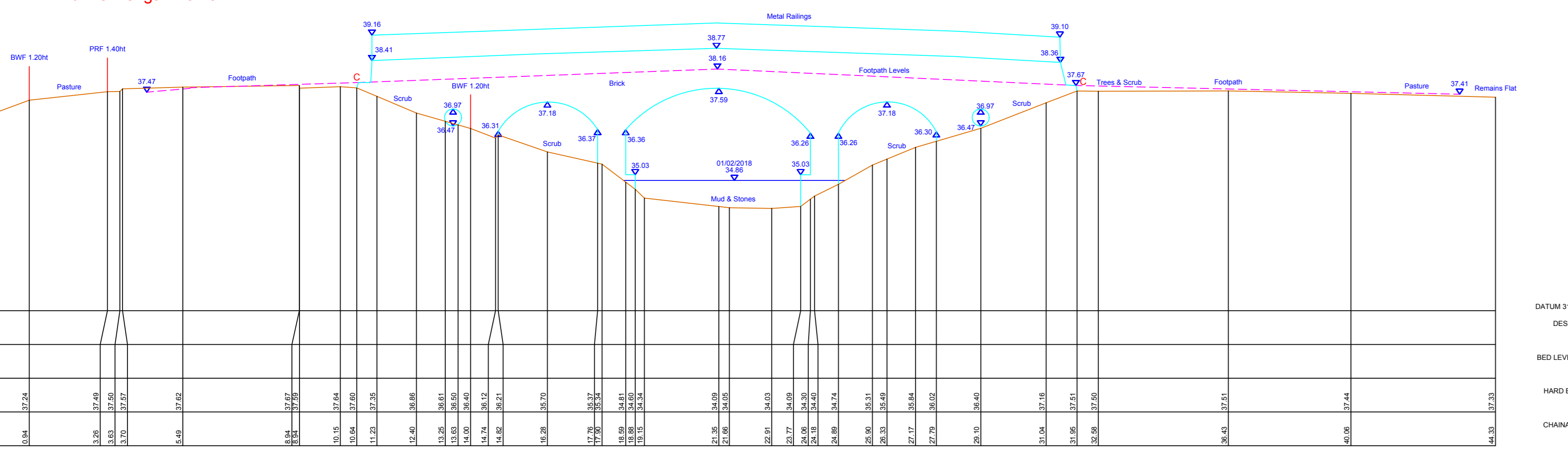
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Open Channel



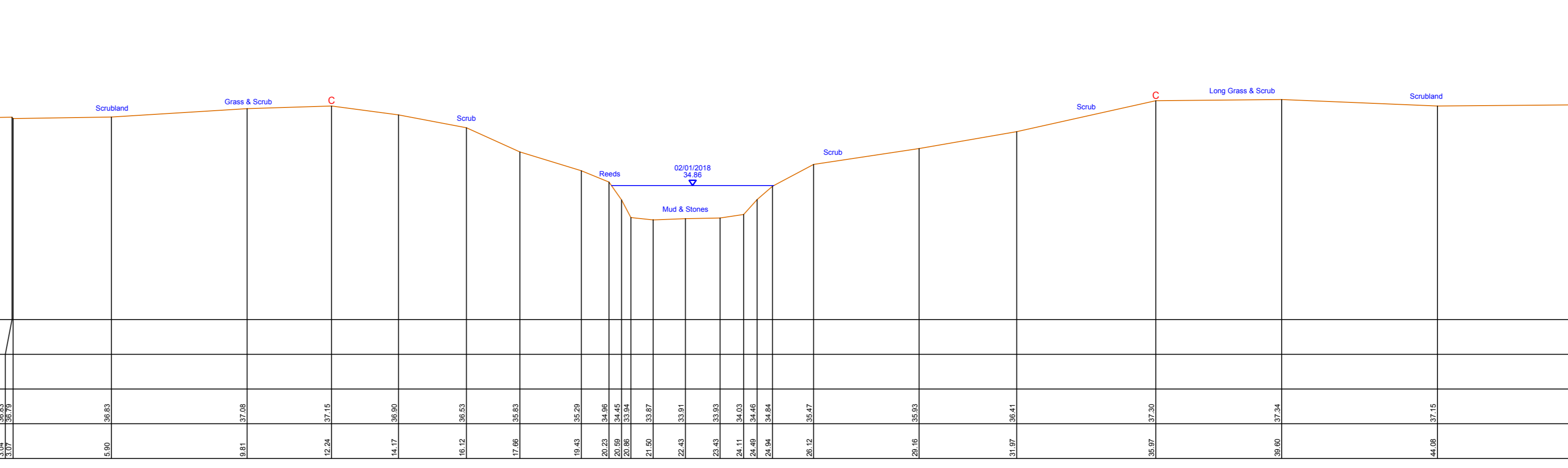
EST001_02386
601294.59mE 140845.38mN Brg 30
Footbridge
Tunnel Length = 3.19m



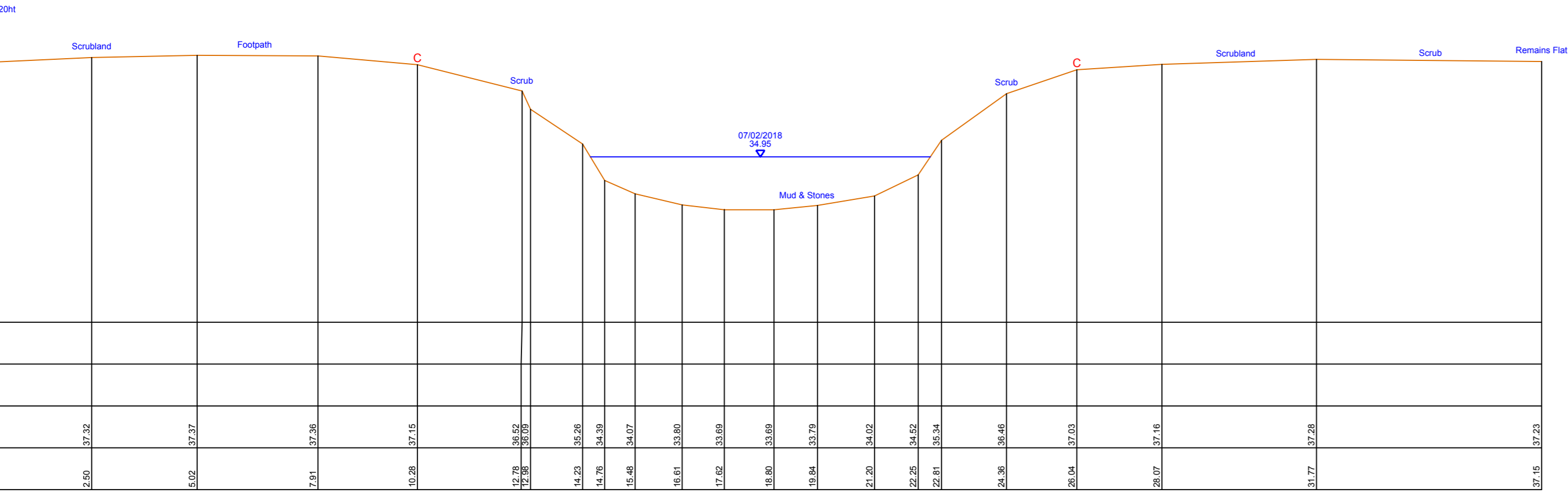
EST001_02500
601396.23mE 140793.64mN Brg 28
Open Channel



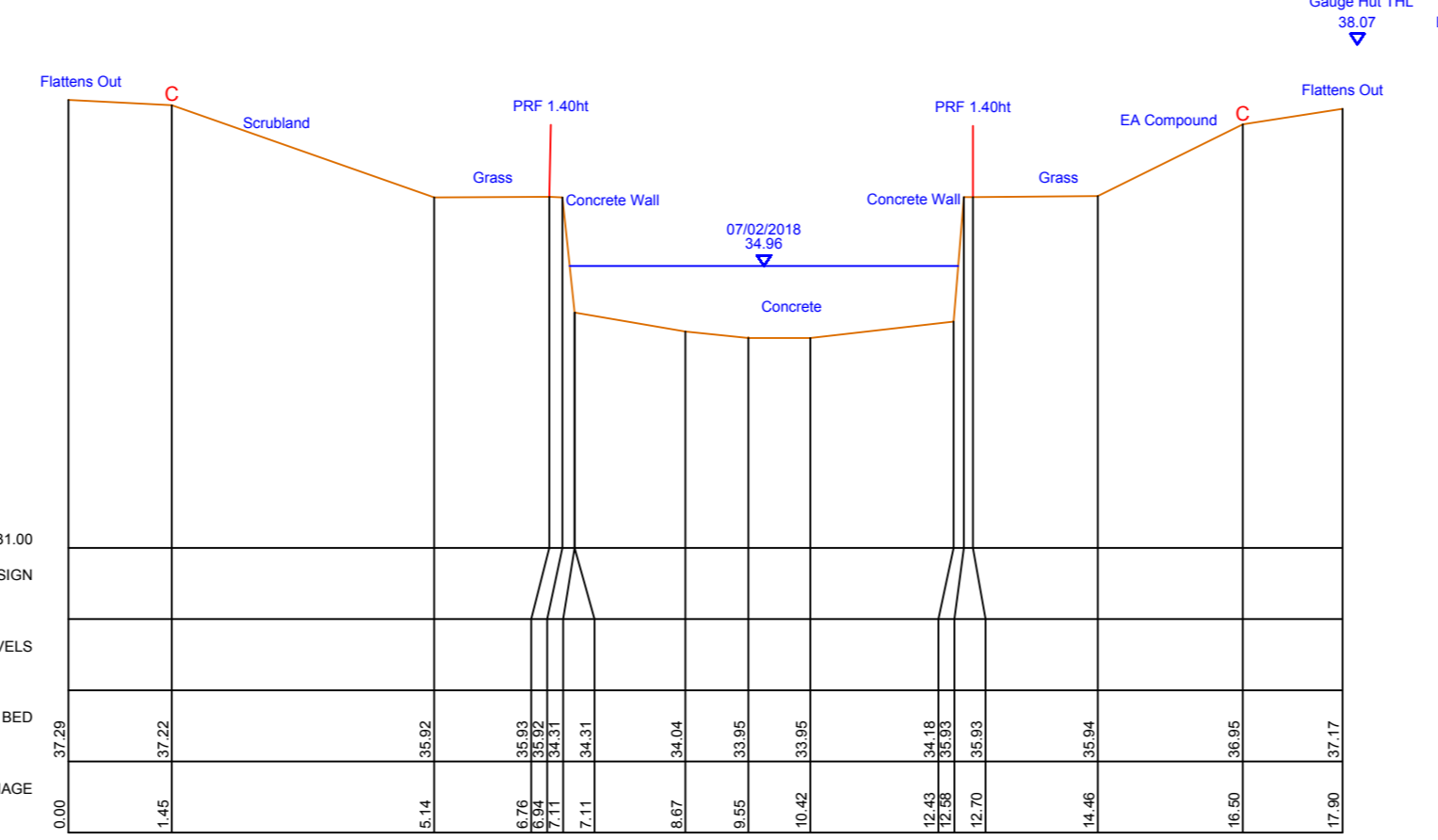
EST001_02577
601452.42mE 140750.06mN Brg 45
Footbridge
Tunnel Length = 4.22m



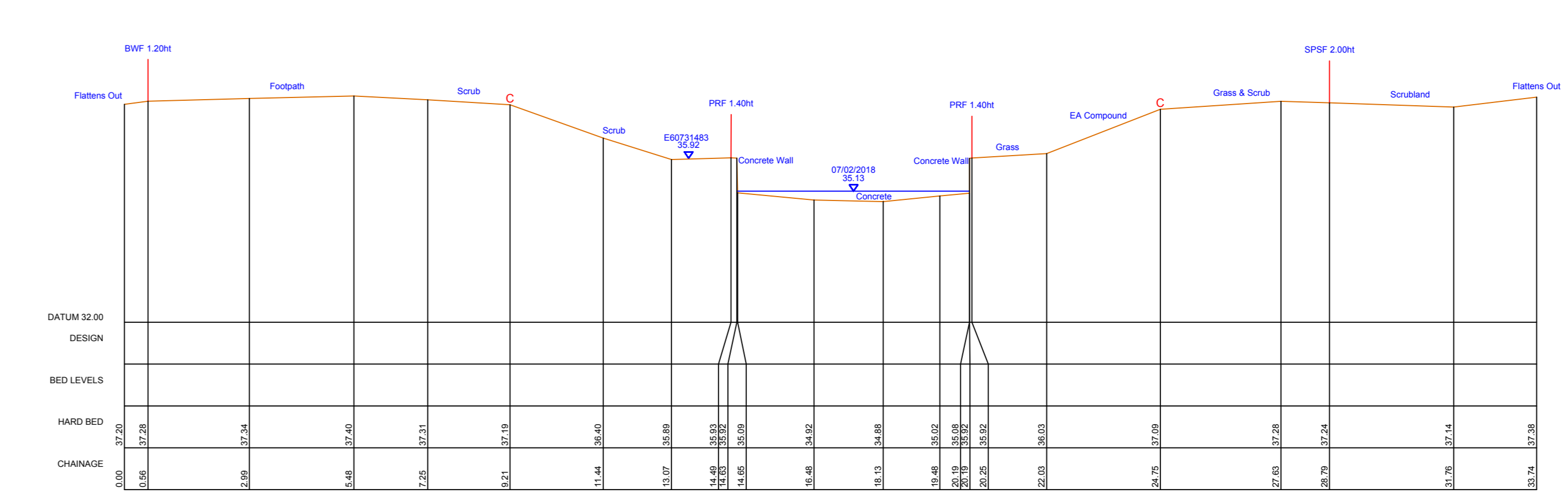
EST001_02605
601466.21mE 140732.41mN Brg 59
Open Channel



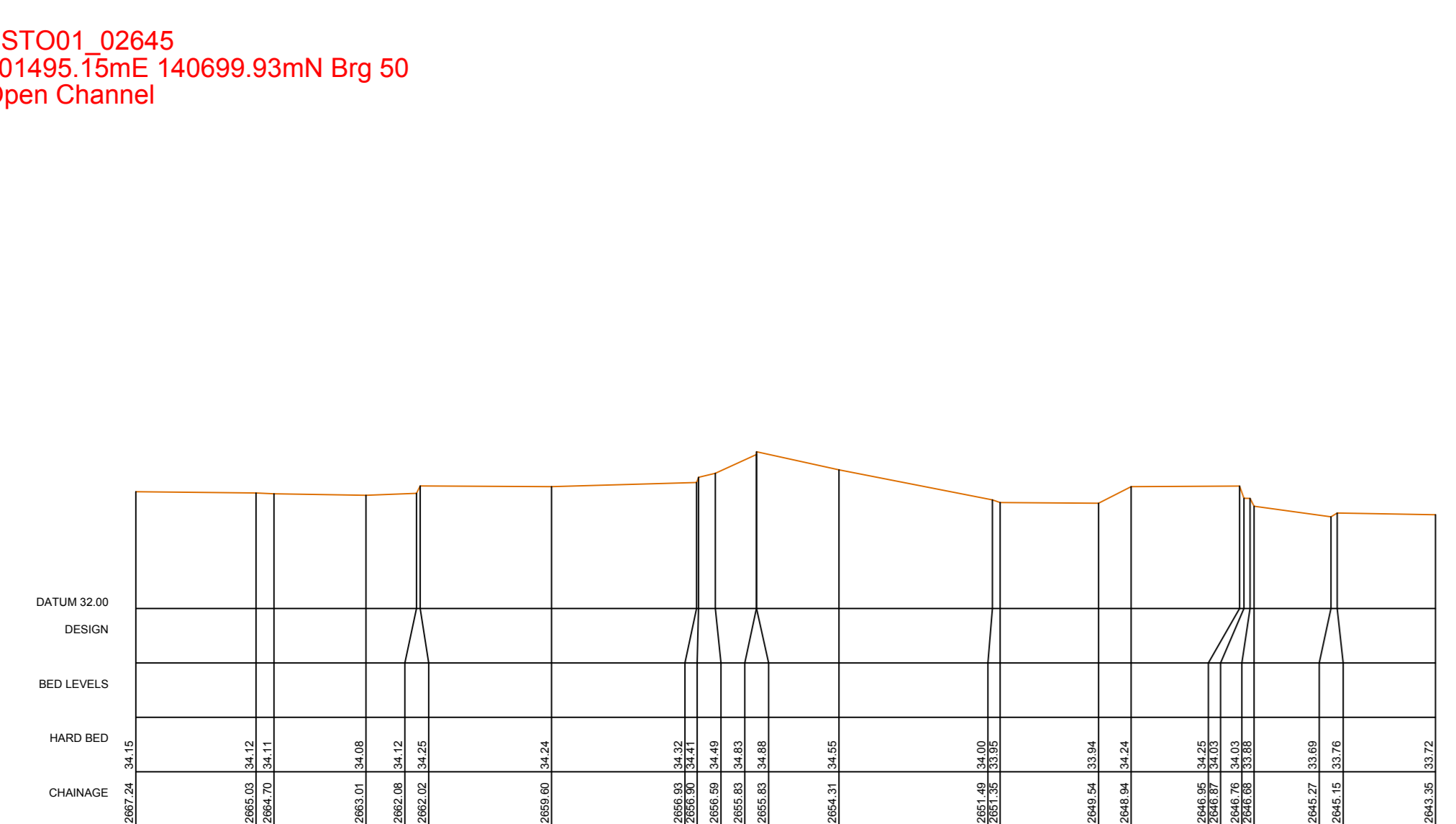
EST001_02645
601495.15mE 140699.93mN Brg 50
Open Channel



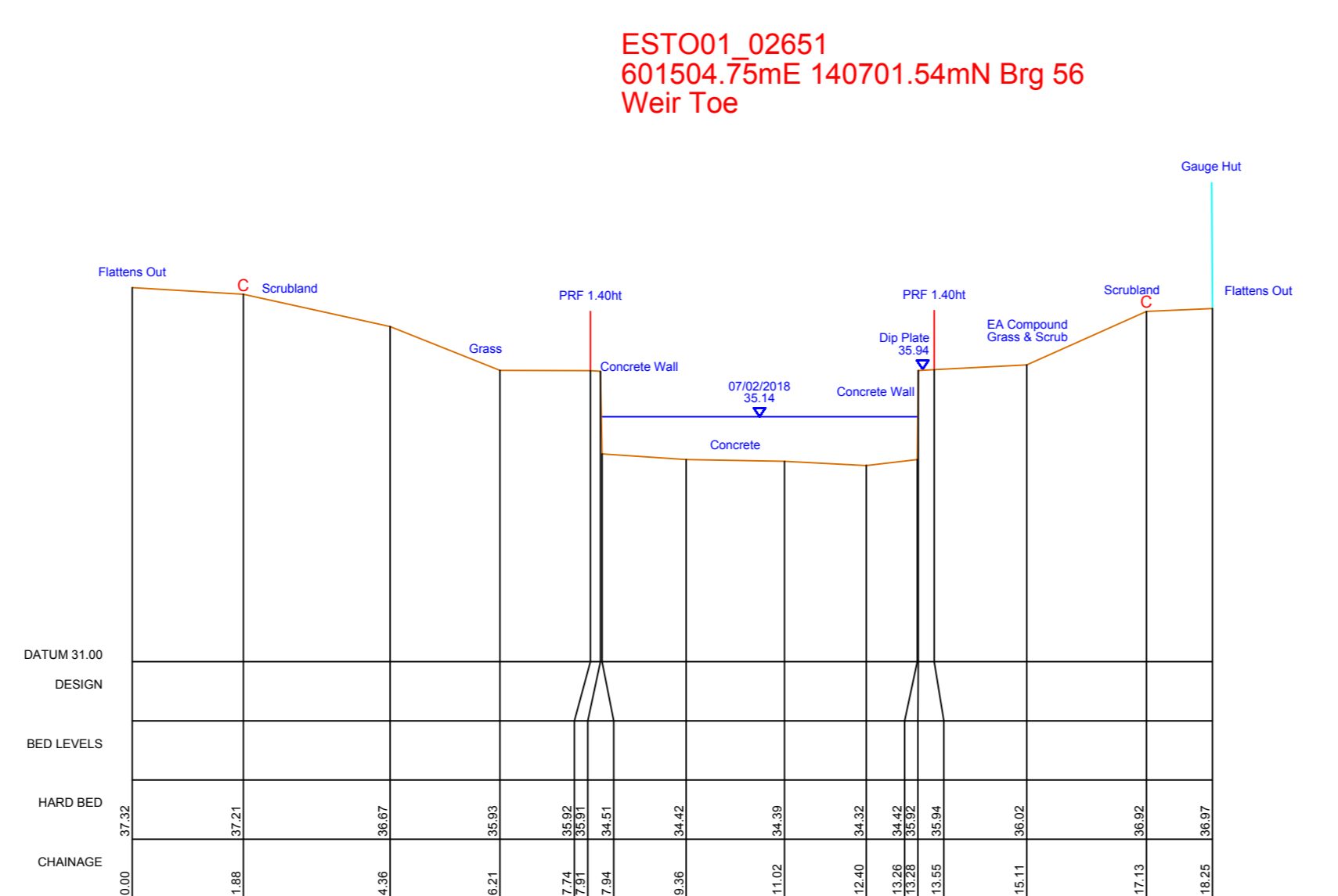
EST001_02651
601504.75mE 140701.54mN Brg 56
Weir Toe



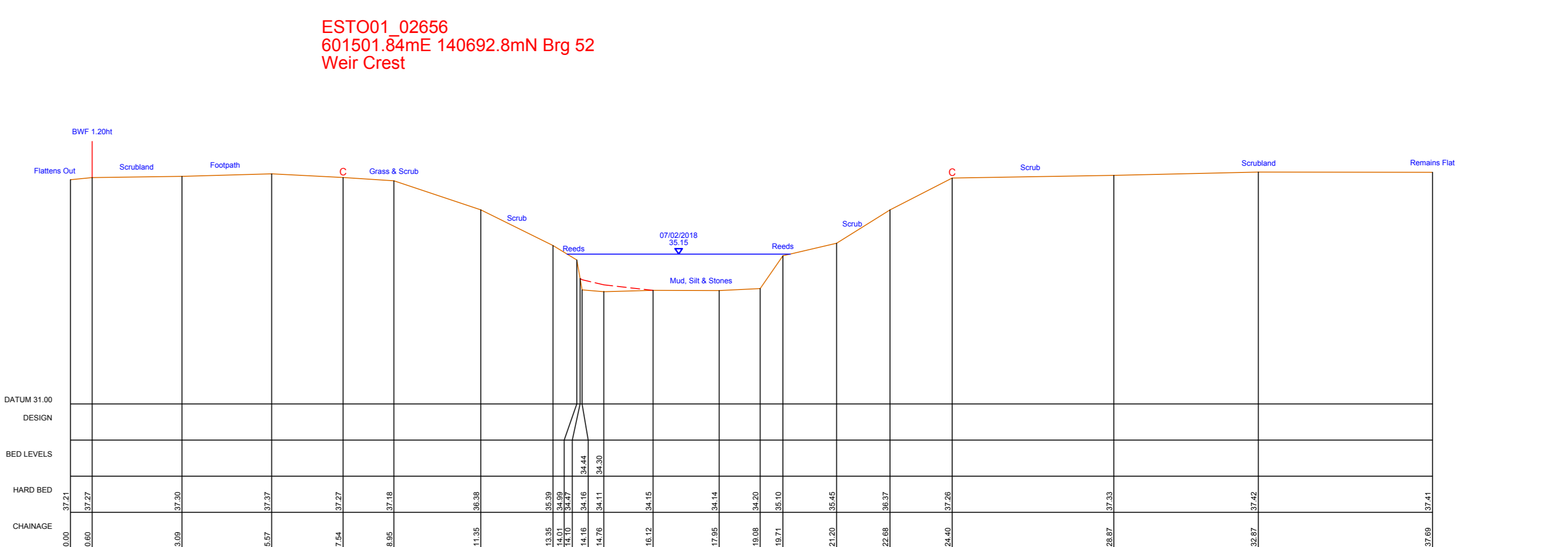
EST001_02656
601501.84mE 140692.8mN Brg 52
Weir Crest



Through Section EST001_2656
Weir Crest



EST001_02657
601507.68mE 140696.48mN Brg 55
Weir Heel



EST001_02665
601506.77mE 140686.88mN Brg 56
Open Channel

NOTES:
1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

AS	AS BECK	FW	THE SHORED	HW	HEAVY WALL
AB	ALBANY	FW	WATER GATE	HR	HIGH DIST
BA	BALANCE	FW	WATER GATE	HR	HIGH DIST
BB	BALANCE	FW	WATER GATE	HR	HIGH DIST
BC	BALANCE	FW	WATER GATE	HR	HIGH DIST
BD	BALANCE	FW	WATER GATE	HR	HIGH DIST
BE	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST
BF	BALANCE	FW	WATER GATE	HR	HIGH DIST

AMENDMENT

NO	DATE	BY	DESCRIPTION	DRN	CHD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
020730012	TR 0103 4107	36.925
020730013	TR 0103 4108	36.925
020730014	TR 0259 4297	36.925
020730015	TR 0109 4202	36.925
020730016	TR 0109 4203	36.925
020730017	TR 0109 4204	36.925
020730018	TR 0109 4205	36.925
020730019	TR 0109 4206	36.925
020730020	TR 0109 4207	36.925
020730021	TR 0109 4208	36.925
020730022	TR 0109 4209	36.925
020730023	TR 0109 4210	36.925
020730024	TR 0109 4211	36.925
020730025	TR 0109 4212	36.925
020730026	TR 0109 4213	36.925
020730027	TR 0109 4214	36.925
020730028	TR 0109 4215	36.925
020730029	TR 0109 4216	36.925
020730030	TR 0109 4217	36.925
020730031	TR 0109 4218	36.925
020730032	TR 0109 4219	36.925
020730033	TR 0109 4220	36.925
020730034	TR 0109 4221	36.925
020730035	TR 0109 4222	36.925
020730036	TR 0109 4223	36.925
020730037	TR 0109 4224	36.925
020730038	TR 0109 4225	36.925
020730039	TR 0109 4226	36.925

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Endonour Park, London Road, Addlestone, East Sussex, Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
EAST STOUR (EST001)
CROSS SECTIONS
EST001_02196 TO EST001_02665

SURVEYED BY: MALTBY LAND SURVEYS LTD
SURVEY DATE: DECEMBER 2017 – MARCH 2018
SCALE: 1:100
DATE: MAR 18

DRN: RC **CHKD:** ITS
DATE: MAR 18 **DATE:** MAR 18

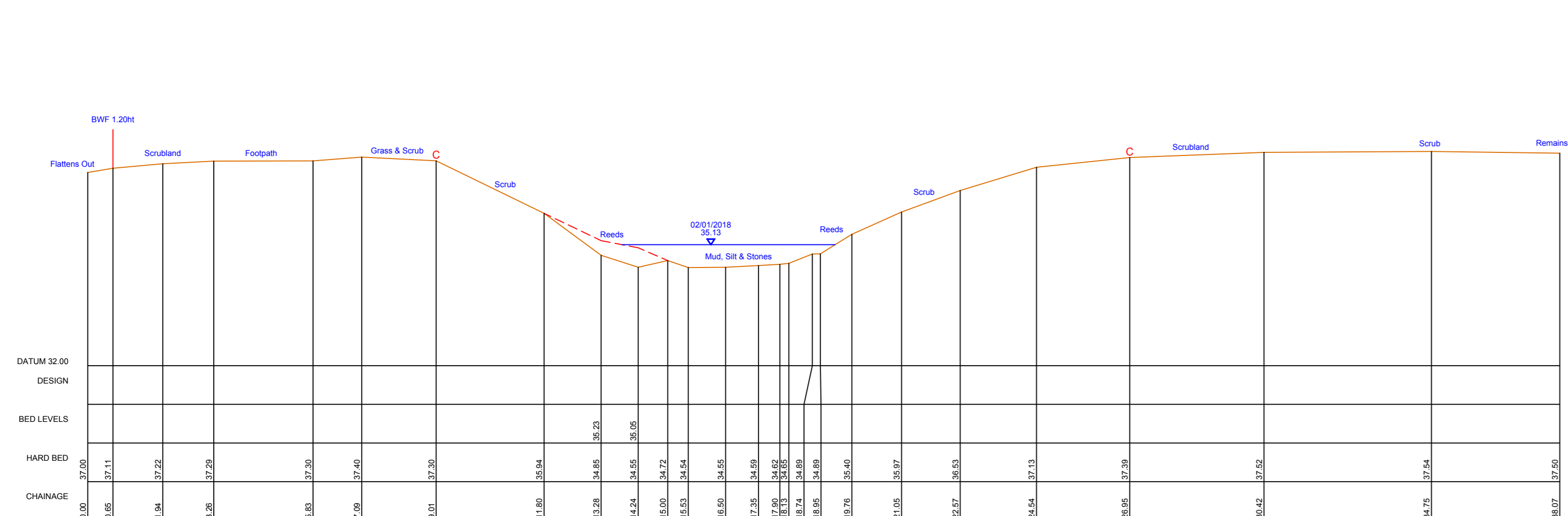
GRID: NATIONAL GRID **DRAWING NO.:** X-J01058-06
DWG FILENAME: E-2018-01-20.dwg

KEY TO SECTIONS:

- WATER LEVEL
- VISBLE BED (TOP OF SILT) AND GROUND
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

KEY TO LONGITUDINAL SECTION ONLY:

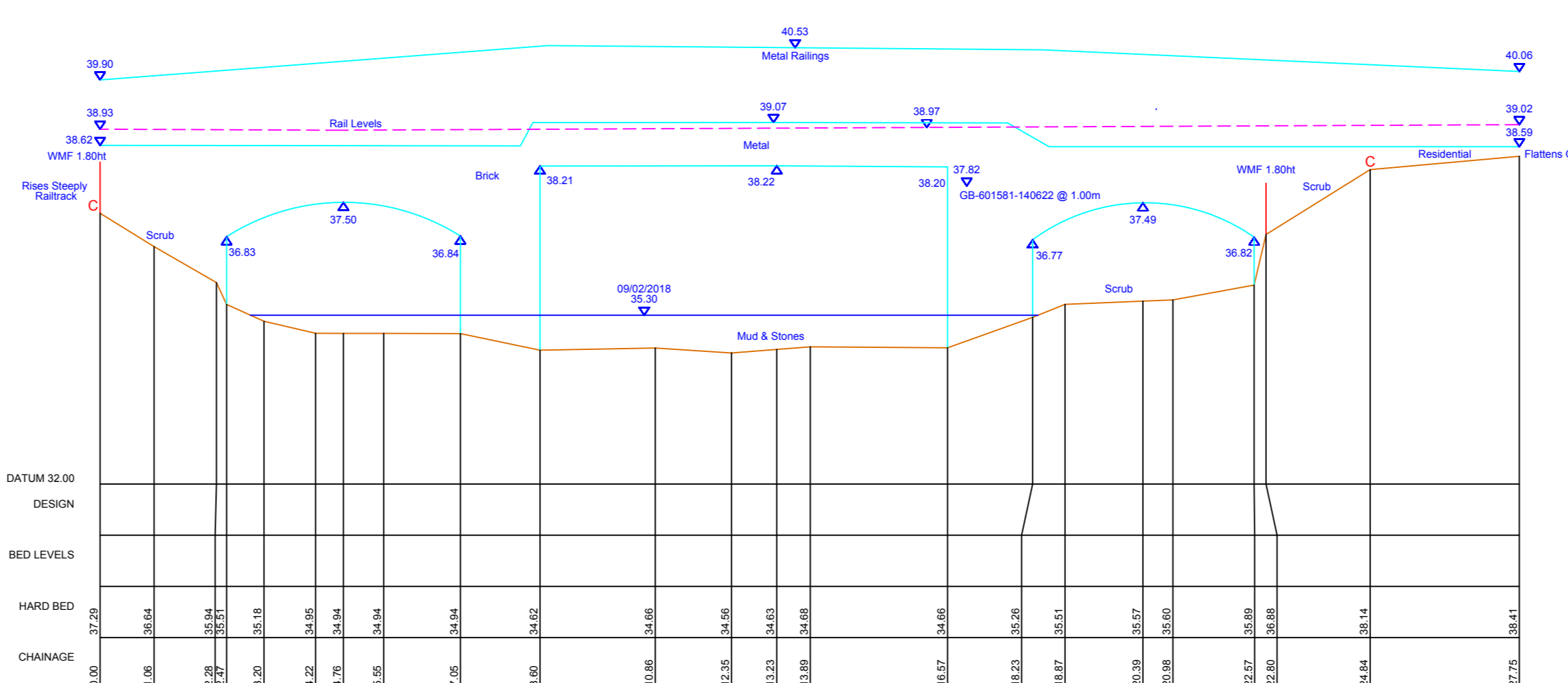
- LEFT BANK CREST
- RIGHT BANK CREST
- POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



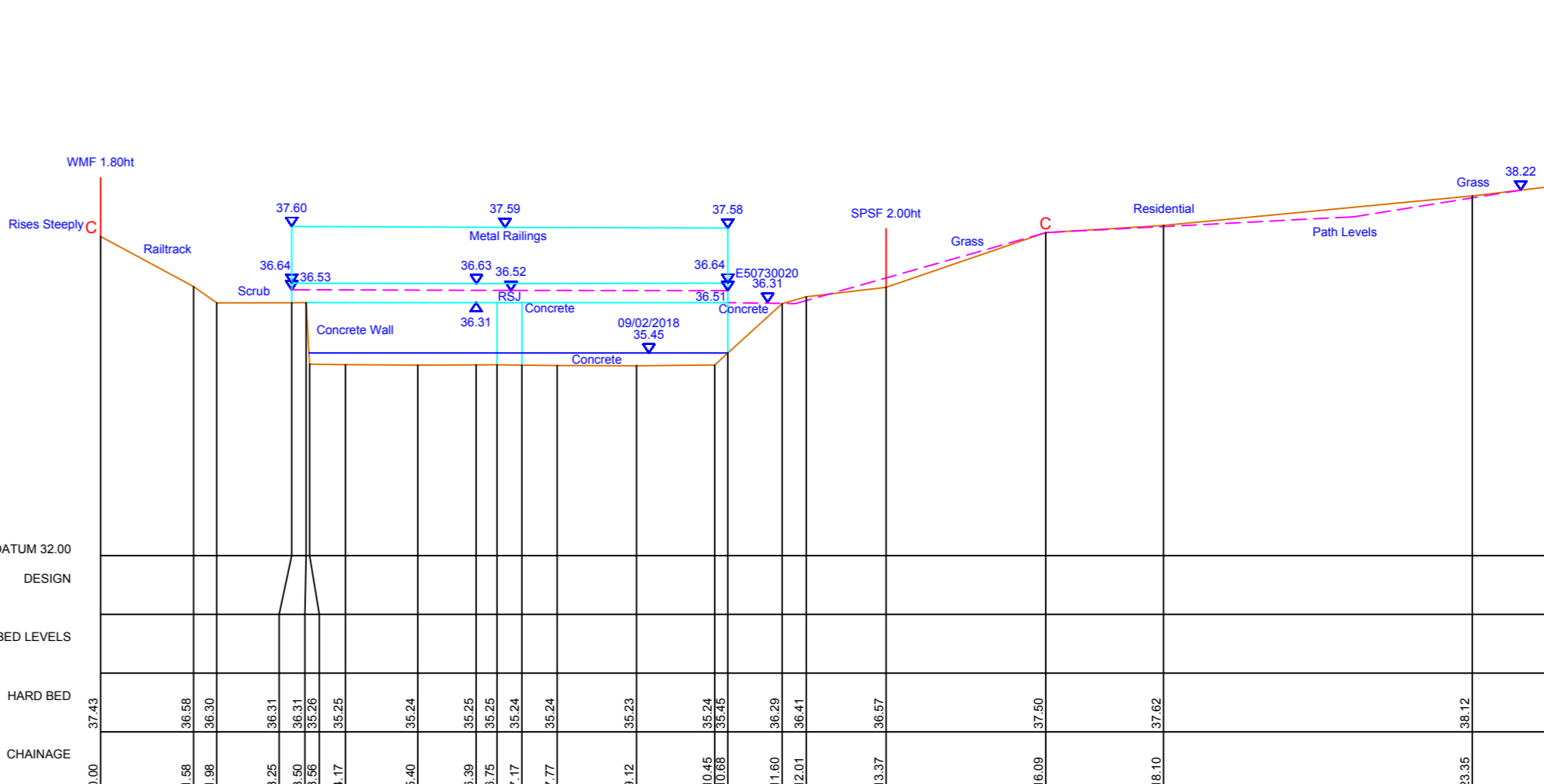
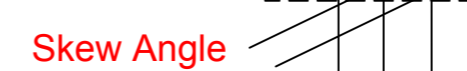
ESTO01_02705
601528.85mE 140653.07mN Brg 56
Open Channel



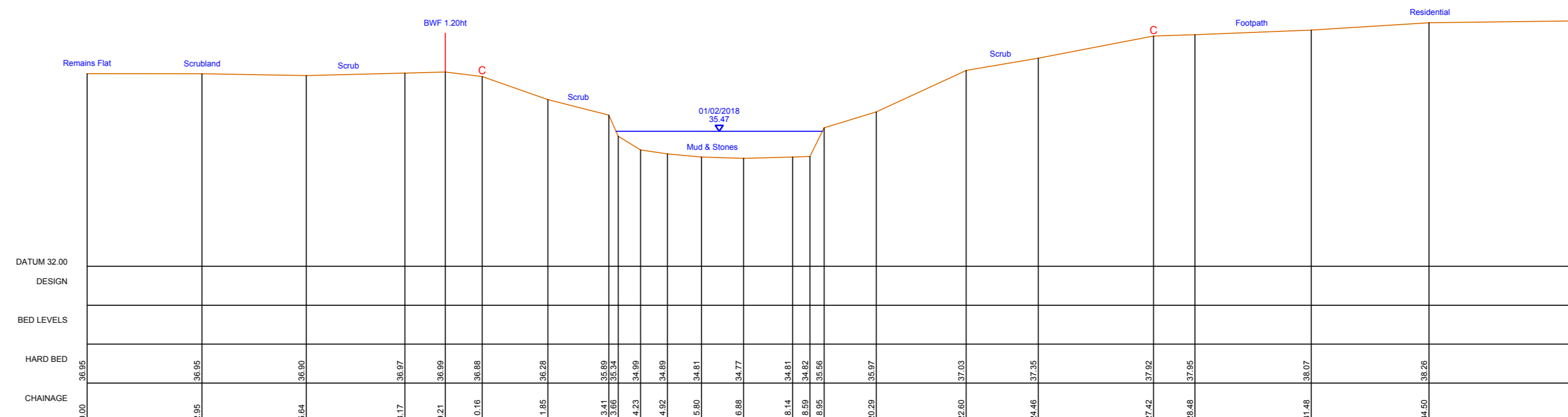
Through Section ESTO01_2775
Footbridge



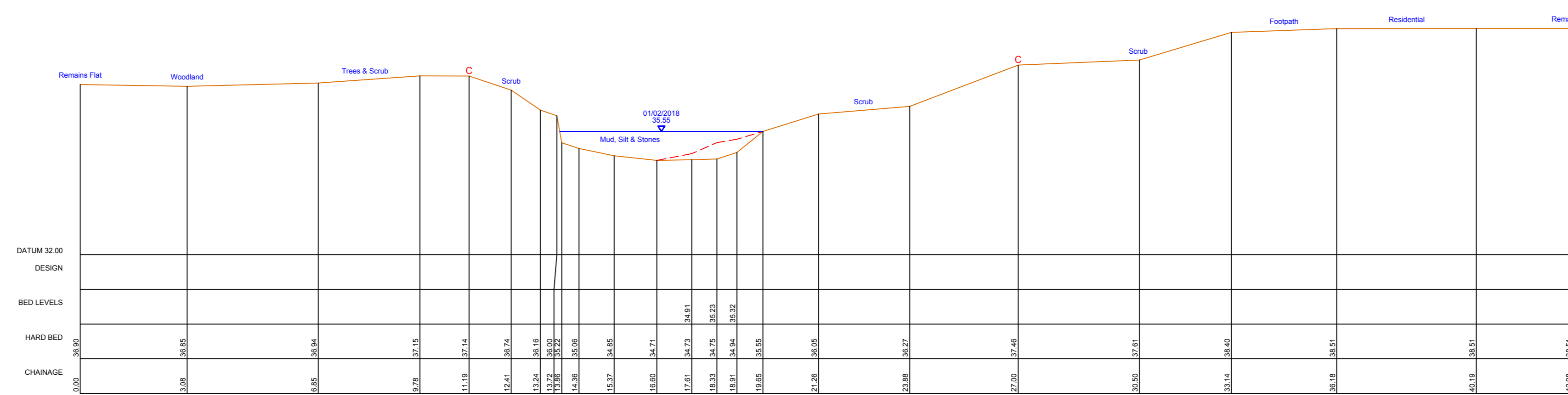
ESTO01_02763
601582.32mE 140606.02mN Brg 356
Rail Bridge
Tunnel Length = 10.55m
Skew Angle = 40°



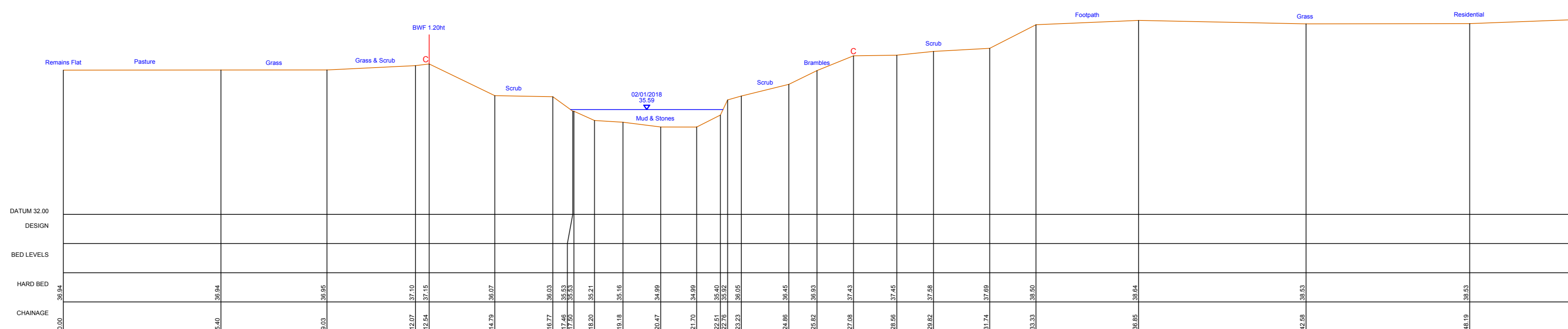
ESTO01_02775
601584.87mE 140605.71mN Brg 43
Footbridge
Tunnel Length = 0.68m



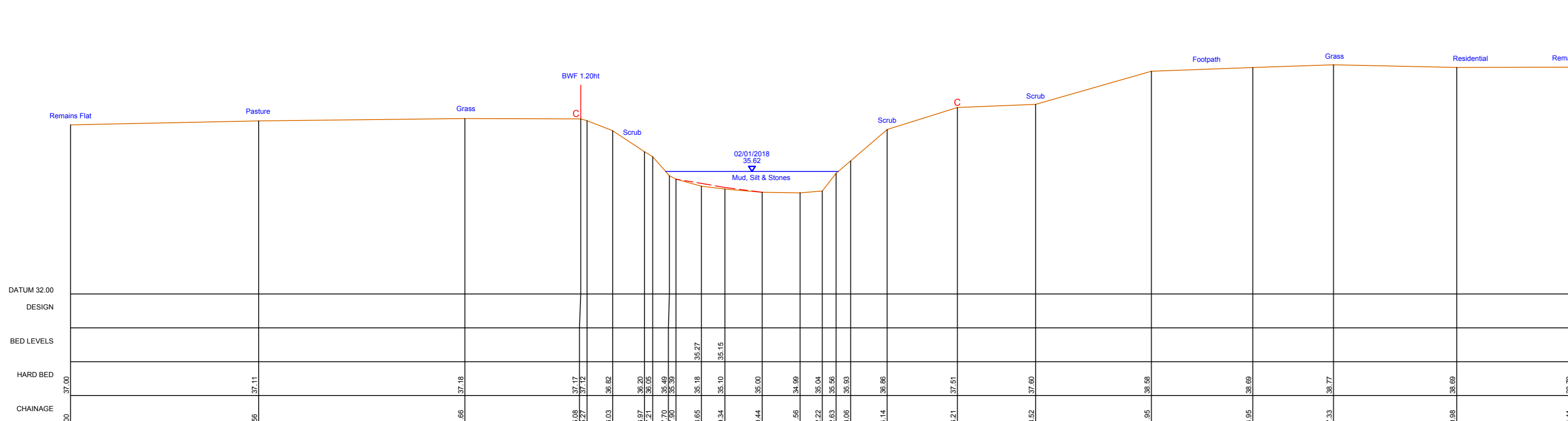
ESTO01_02821
601619.02mE 140564.32mN Brg 16
Open Channel



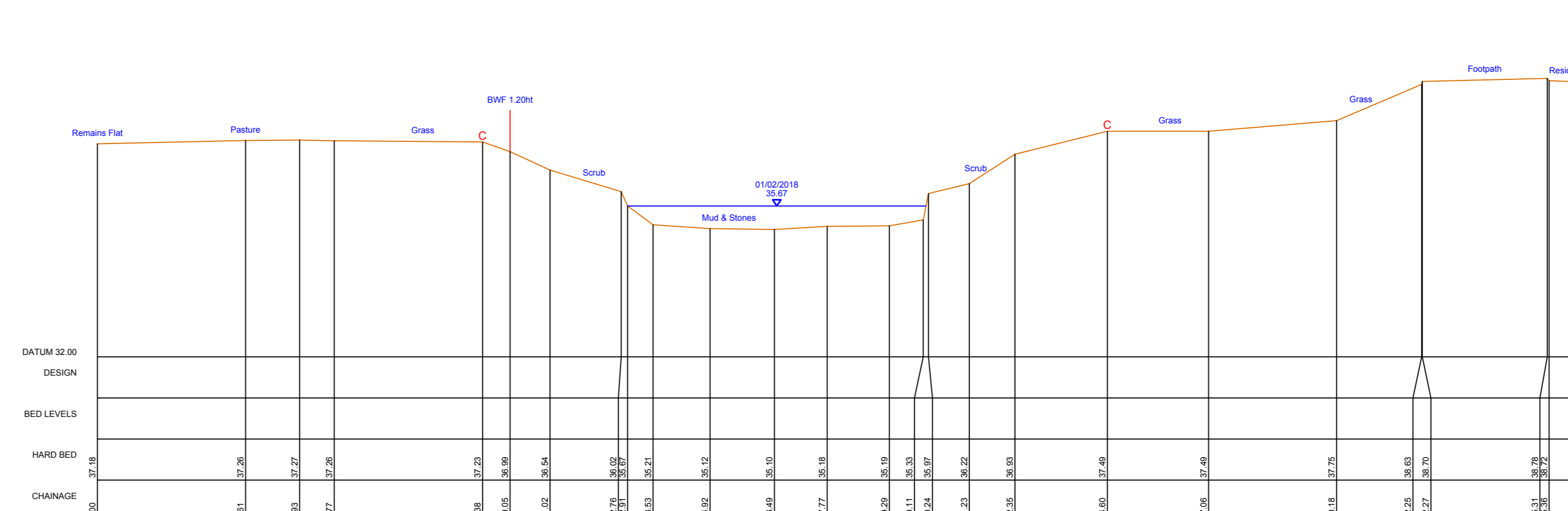
ESTO01_02904
601699.17mE 140542.41mN Brg 13
Open Channel



ESTO01_03003
601794.83mE 140522.01mN Brg 14
Open Channel



ESTO01_03096
601886.50mE 140503.89mN Brg 10
Open Channel



ESTO01_03202
601977.90mE 140476.34mN Brg 43
Open Channel

NOTES:

1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
(Symbol)	Water Level	(Symbol)	Bank Crest	(Symbol)	Point
(Symbol)	Visible Bed	(Symbol)	Structure	(Symbol)	Spot Height
(Symbol)	Hard Bed	(Symbol)	Vegetation	(Symbol)	Contour
(Symbol)	Left Bank Crest	(Symbol)	Right Bank Crest	(Symbol)	Spot Height

AMENDMENT	NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:	DESCRIPTION	LEVEL
E0730012	TR 0103 4107	35.975
E0730405	TR 0229 4297	38.480
E0730019	TR 0195 4202	38.480
E0730007	TR 0199 4203	38.480
E0730001	TR 0199 4201	38.480
E0730008	TR 0199 4204	38.480
E0730004	TR 0199 4205	38.480
E0730005	TR 0199 4206	38.480
E0730006	TR 0199 4207	38.480
E0730003	TR 0199 4208	38.480
E0730002	TR 0199 4209	38.480
E0730001	TR 0199 4210	38.480
E0730000	TR 0199 4211	38.480
E0730009	TR 0199 4212	38.480
E0730010	TR 0199 4213	38.480
E0730011	TR 0199 4214	38.480
E0730012	TR 0199 4215	38.480
E0730013	TR 0199 4216	38.480
E0730014	TR 0199 4217	38.480
E0730015	TR 0199 4218	38.480
E0730016	TR 0199 4219	38.480
E0730017	TR 0199 4220	38.480
E0730018	TR 0199 4221	38.480
E0730019	TR 0199 4222	38.480
E0730020	TR 0199 4223	38.480

Environment Agency
KENT & SOUTH LONDON REGION
Orford House, Orfordour Park, London Road, Addlestone, Medway, Kent, ME19 5PH

PROJECT/WATERCOURSE: EAST STOUR, ASHFORD TO STANFORD

SITE/LIMITS: EAST STOUR (ESTO01) CROSS SECTIONS ESTO01_02705 TO ESTO01_03202

SURVEYED BY: MALTBY LAND SURVEYS LTD Rev 12_15/17

SURVEY DATE: DECEMBER 2017 - MARCH 2018

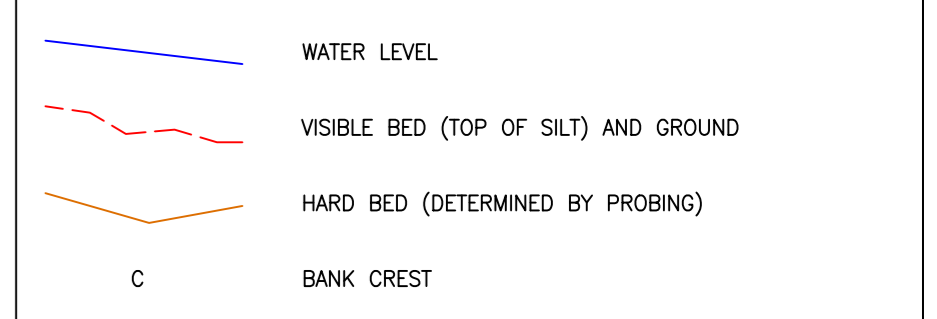
SCALE: 1:100 DRN: RC CHKD: ITS

DATUM: OS GPS ACTIVE DATE: MAR 18 DATE: MAR 18

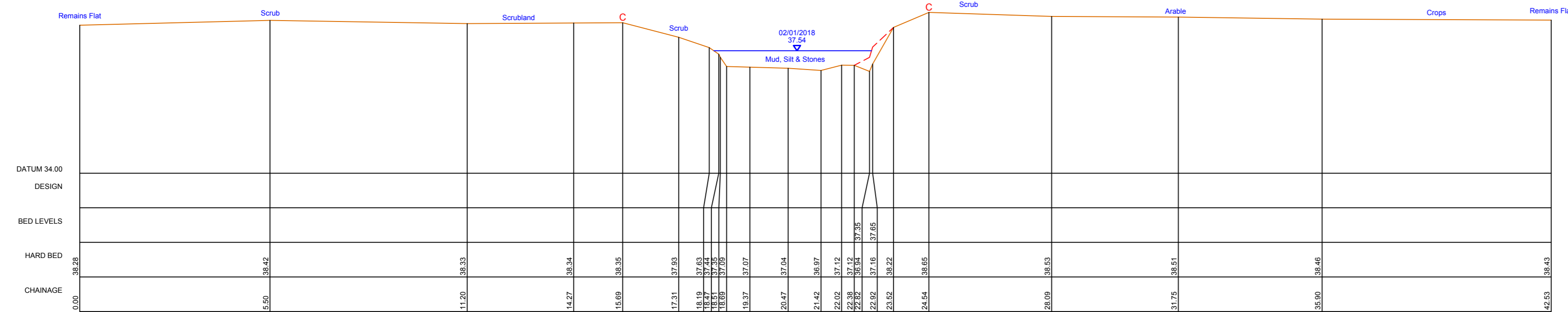
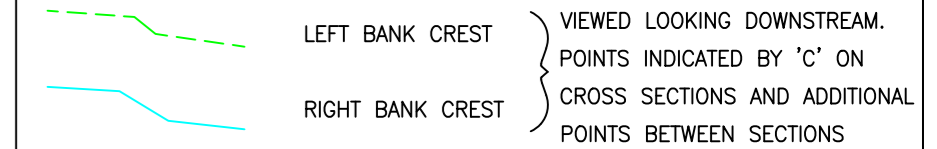
GRID: NATIONAL GRID DRAWING NO. X-101058-07 REV.

CAD FILENAME: E-21058-01-30.dwg

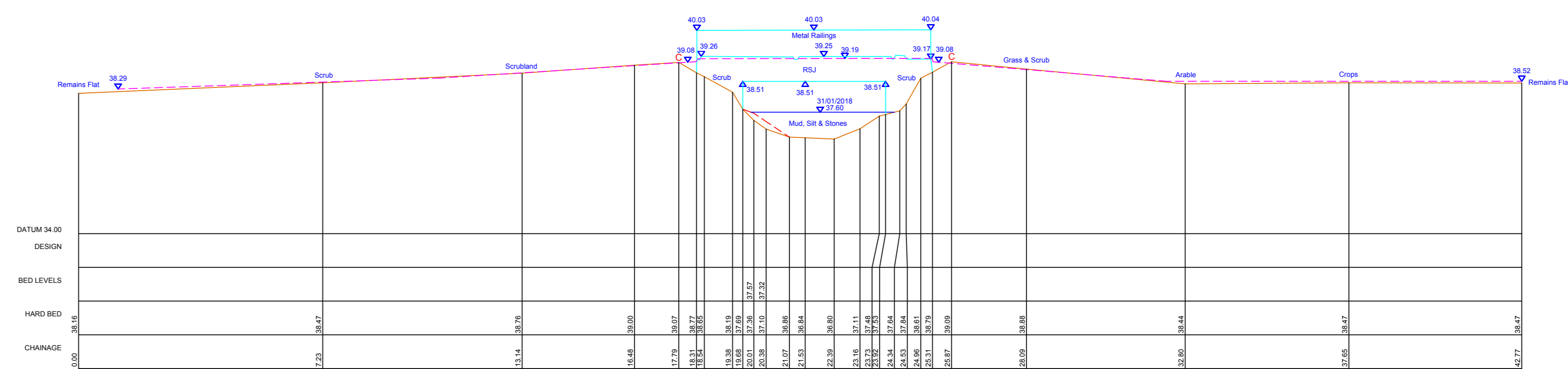
KEY TO SECTIONS:



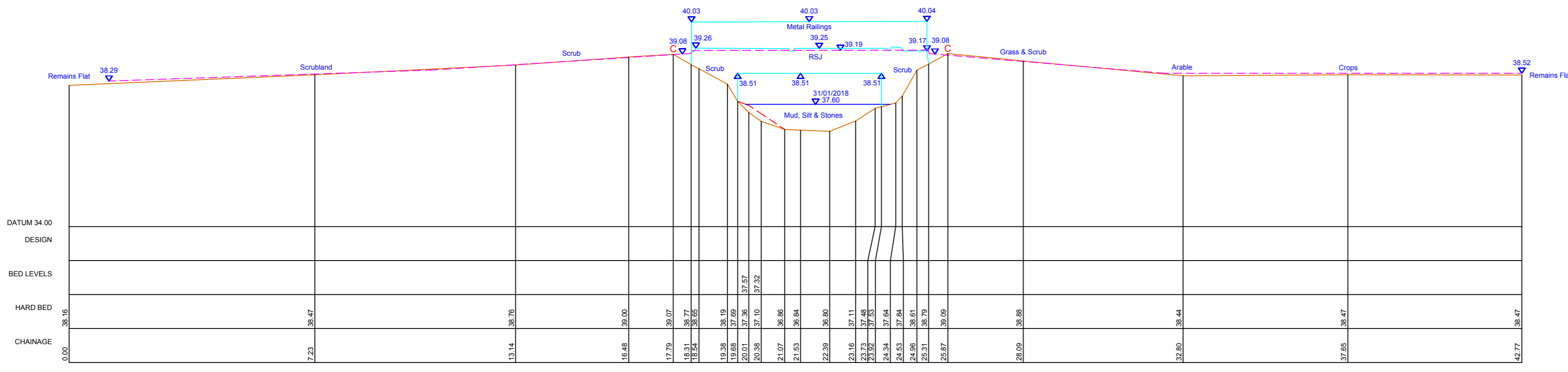
KEY TO LONGITUDINAL SECTION ONLY:



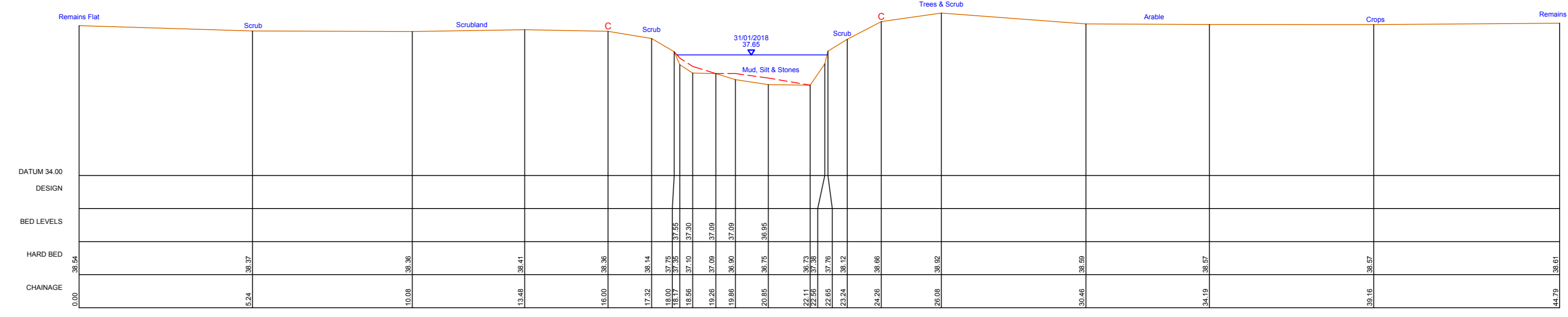
ESTO01_04717 602826.53mE 139713.92mN Brg 31 Open Channel



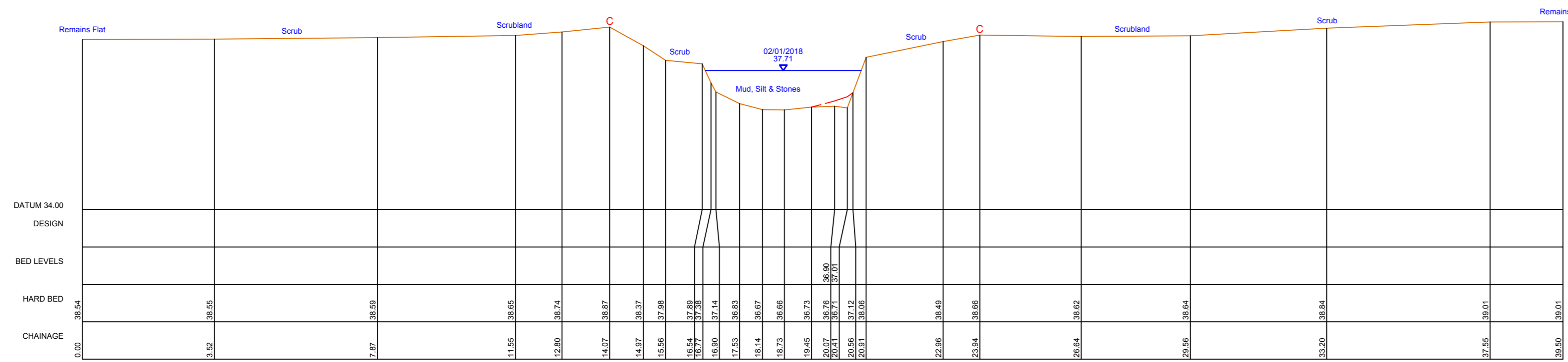
ESTO01_04735 602843.81mE 139702.98mN Brg 24 Footbridge Tunnel Length = 4.02m



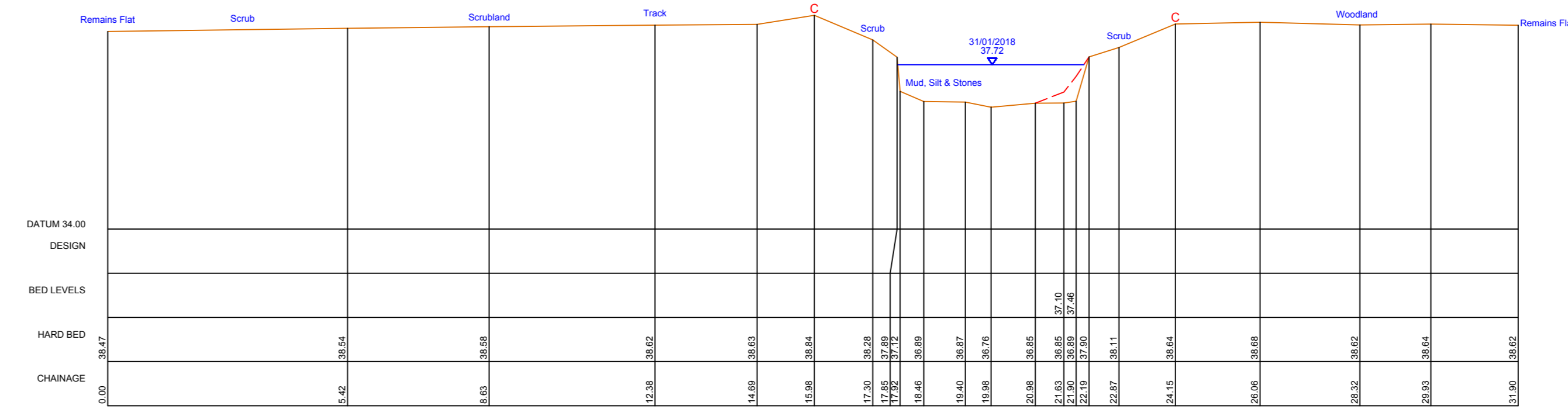
ESTO01_04738 602847.64mE 139703mN Brg 23 Footbridge Tunnel Length = 0.88m



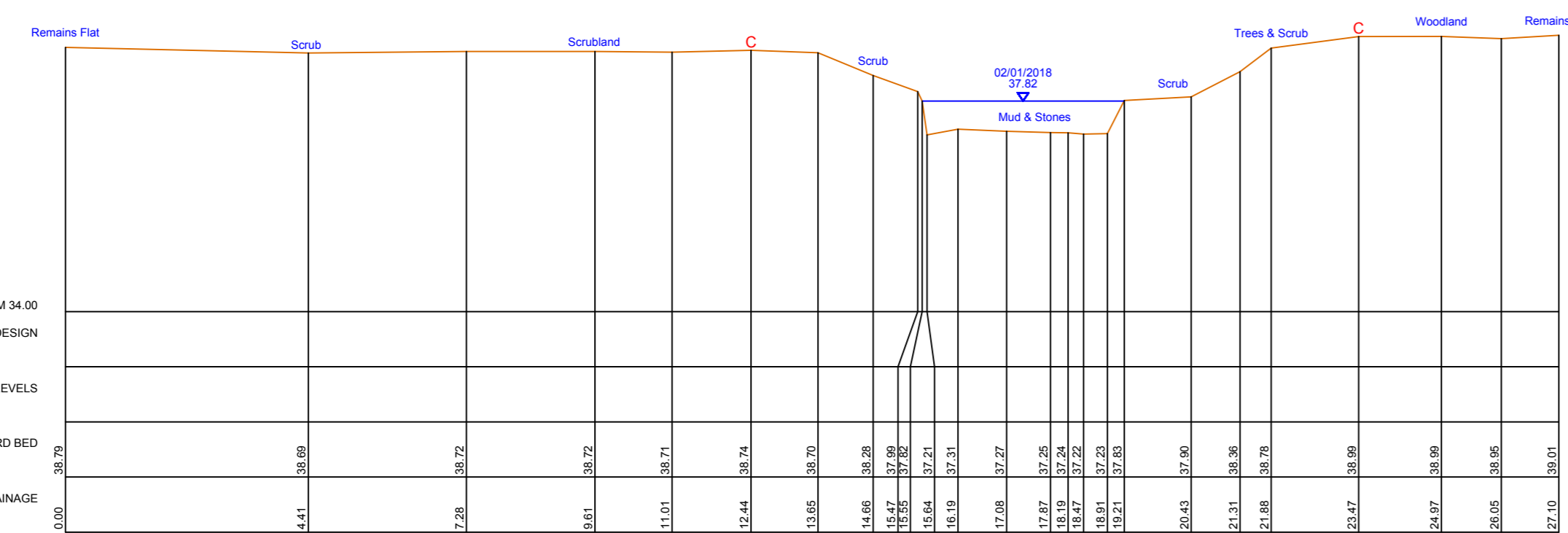
ESTO01_04804 602916.24mE 139682.68mN Brg 360 Open Channel



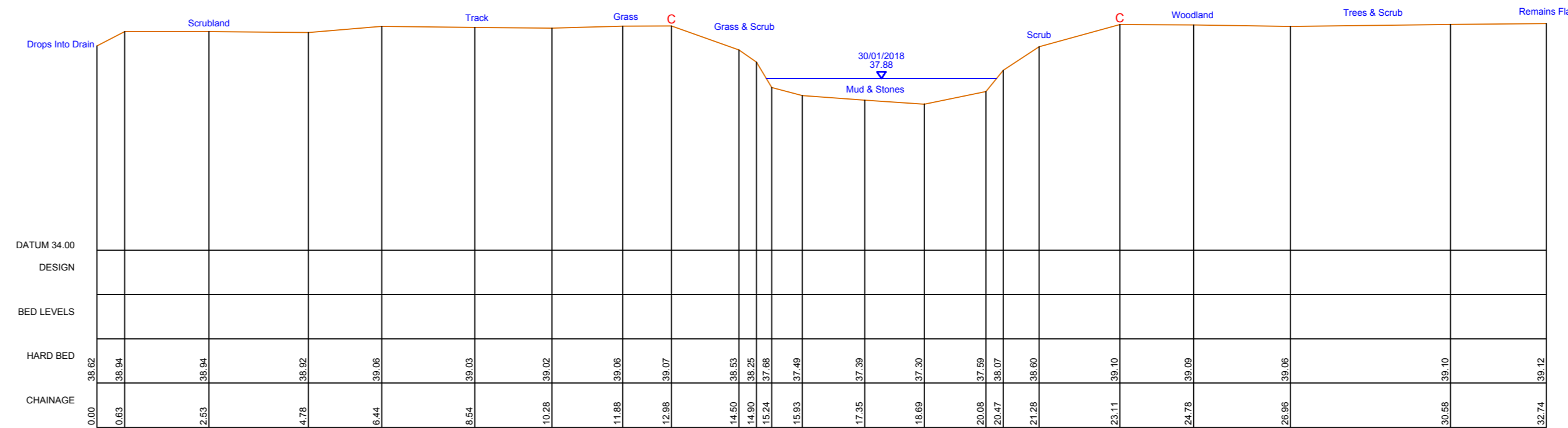
ESTO01_04907 602994.42mE 139648.03mN Brg 31 Open Channel



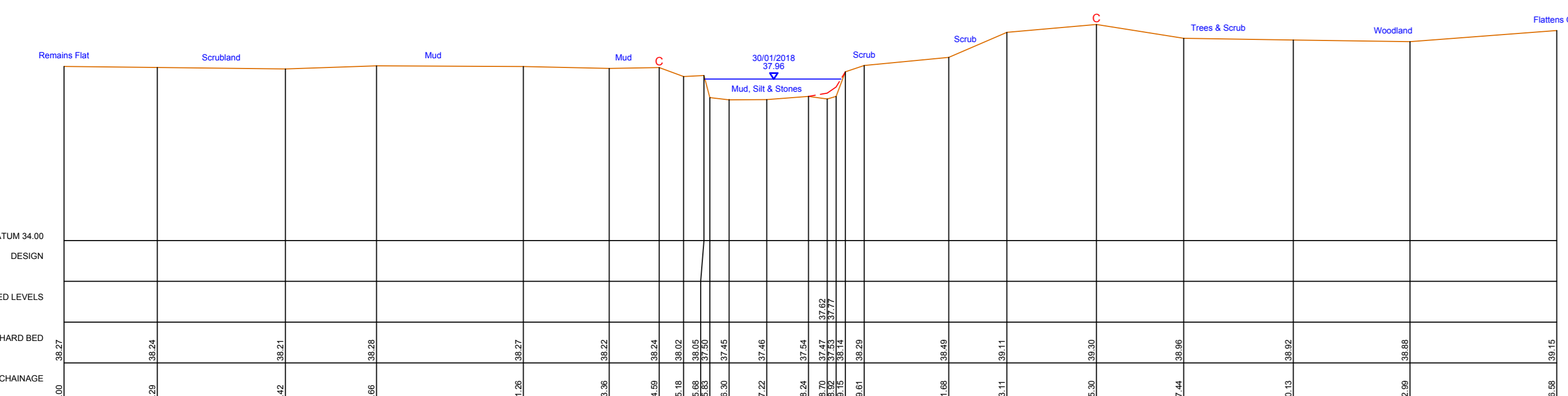
ESTO01_05008 603077.88mE 139597.7mN Brg 39 Open Channel



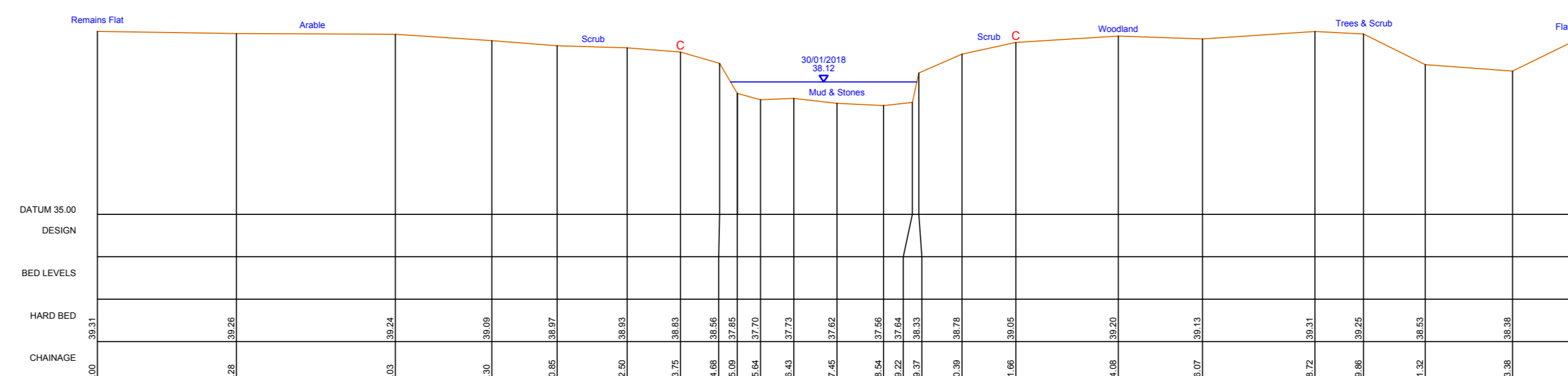
ESTO01_05114 603167.19mE 139579.68mN Brg 50 Open Channel



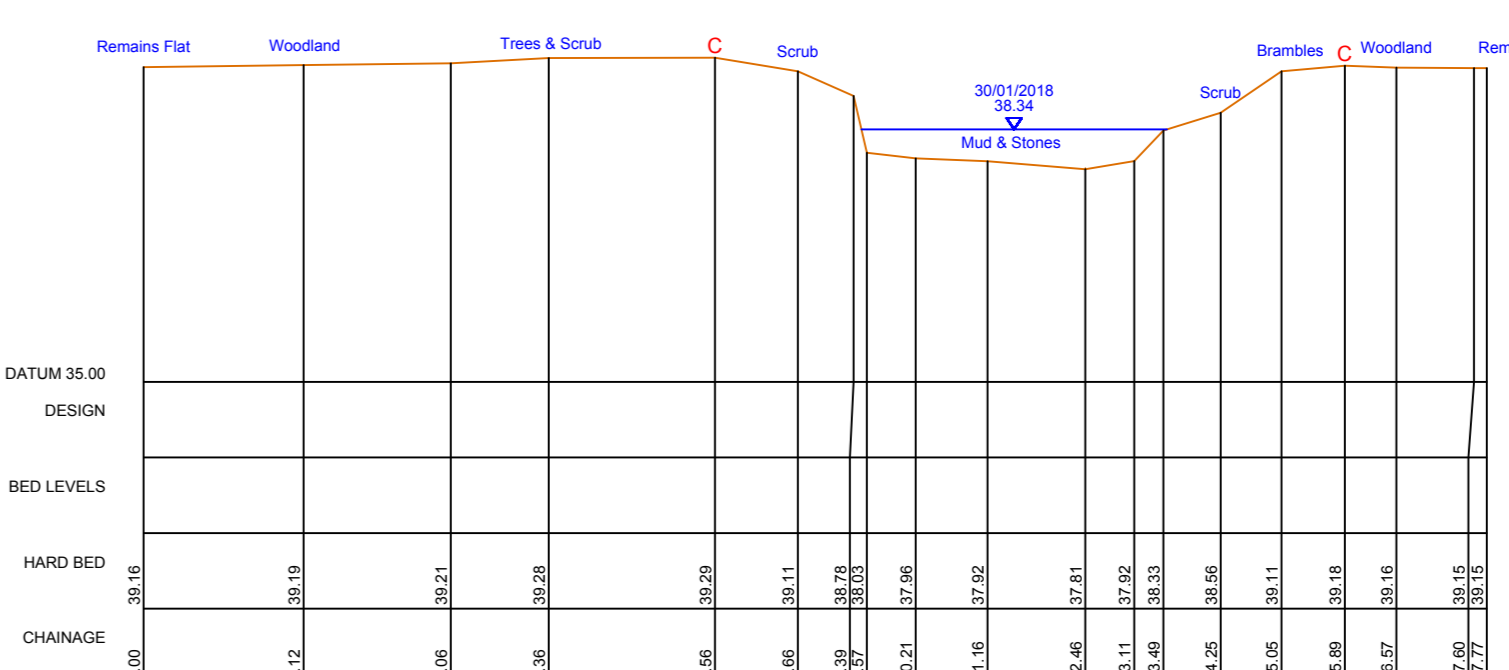
ESTO01_05214 603198.86mE 139504.82mN Brg 102 Open Channel



ESTO01_05304 603233.45mE 139421.06mN Brg 55 Open Channel



ESTO01_05397 603341.71mE 139397.4mN Brg 339 Open Channel



ESTO01_05496 603408.01mE 139371.32mN Brg 41 Open Channel

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Table with 4 columns: CODE, SYMBOL, DESCRIPTION, and DATE. It lists various symbols used in the drawing such as water level, bed levels, and bank crests.

Table with 4 columns: AMENDMENT, DESCRIPTION, DRAWN, and DATE. It lists amendments made to the drawing.

Table with 3 columns: TYPE, DESCRIPTION, and LEVEL. It lists control points used in the survey.



KENT & SOUTH LONDON REGION

Project address: Orchard House, Deodar Park, London Road, Addington, Kent, ME9 5SH

PROJECT/WATERCOURSE: EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS: EAST STOUR (ESTO01) CROSS SECTIONS ESTO01_04717 TO ESTO01_05496

SURVEYED BY: MALTBY LAND SURVEYS LTD

SURVEY DATE: DECEMBER 2017 - MARCH 2018

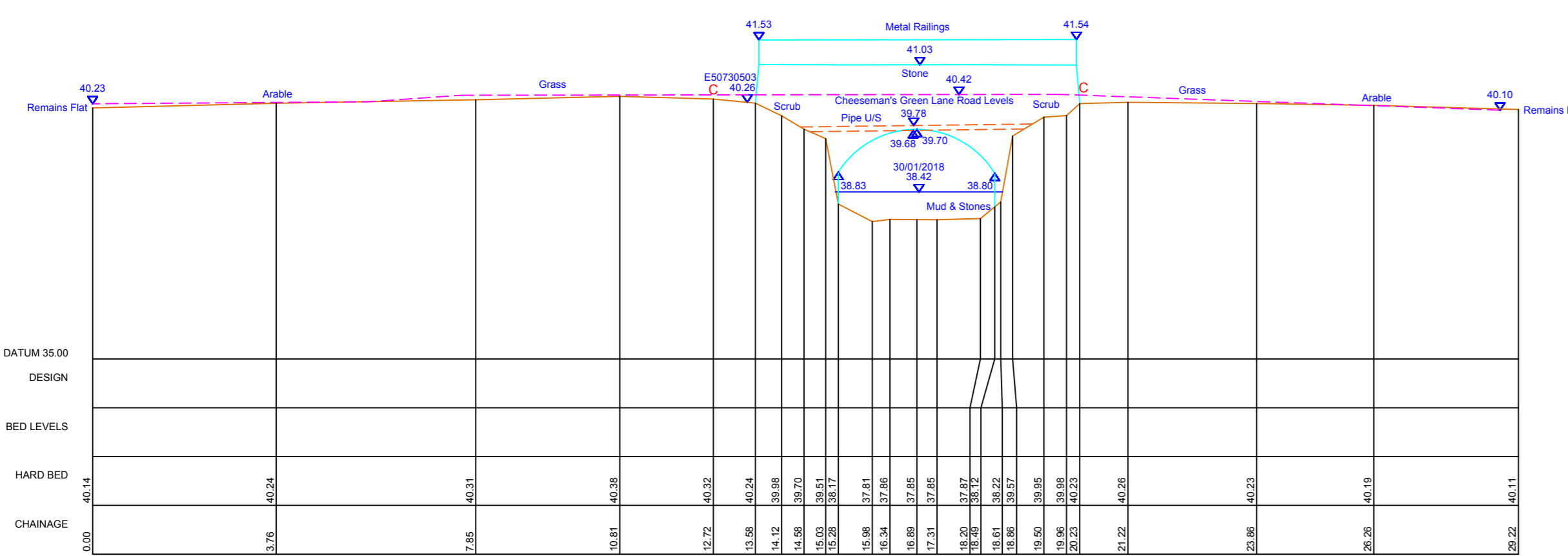
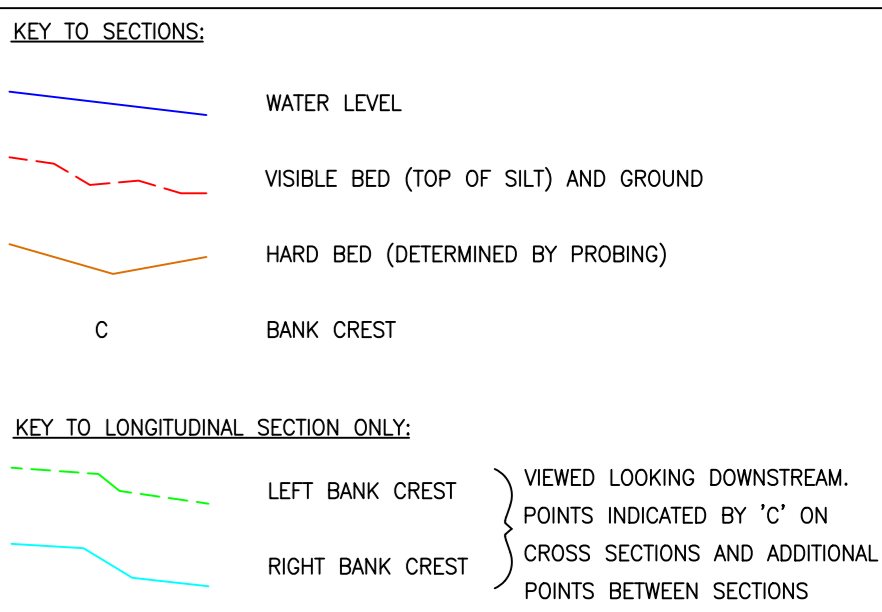
SCALE: 1:100

DATUM: OS GPS ACTIVE

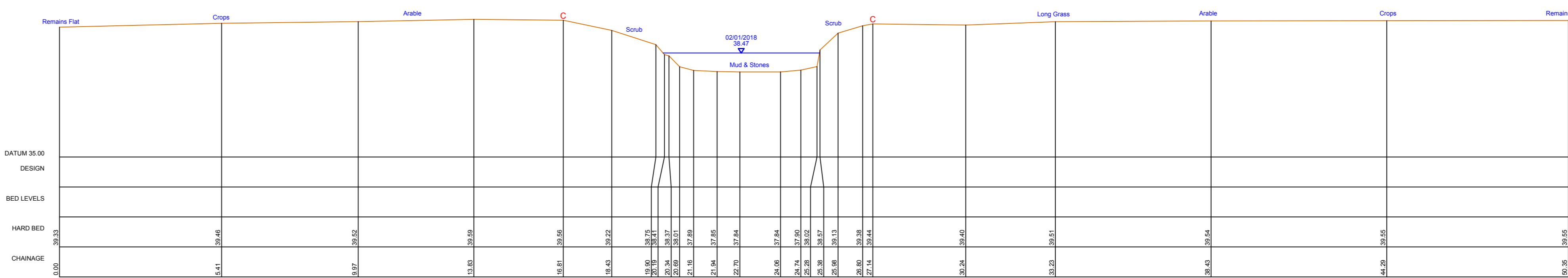
GRID: NATIONAL GRID

DWG FILENAME: X-011058-01-31.dwg

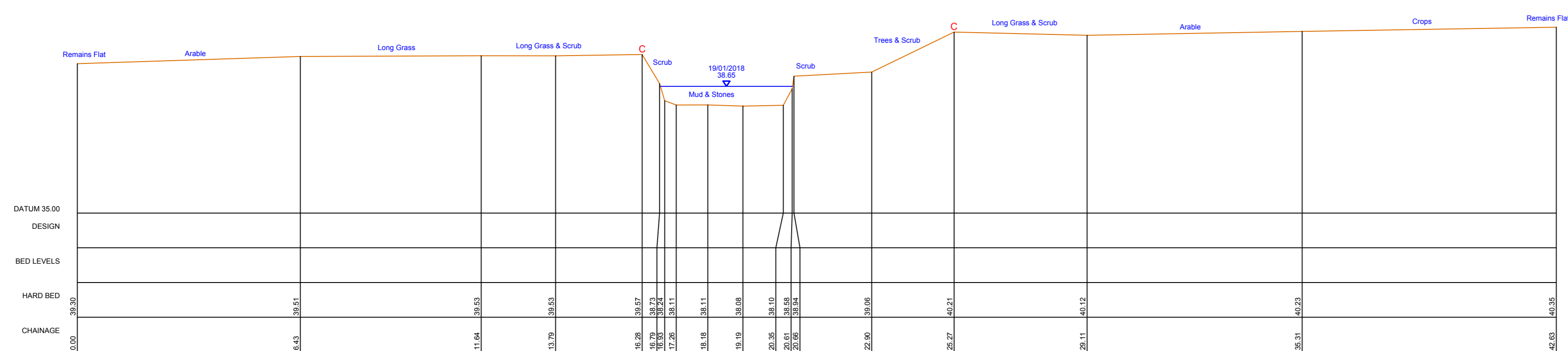
X-011058-10



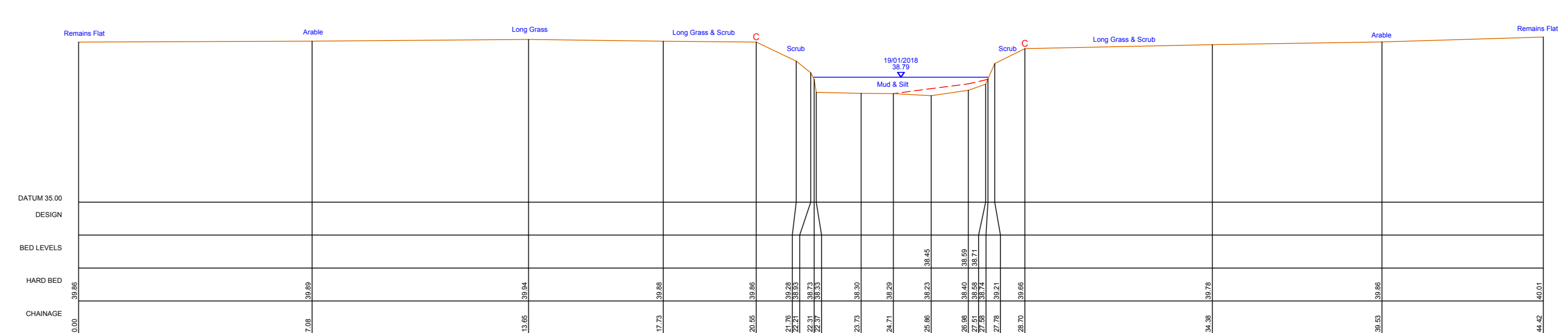
EST001_05532
603433.55mE 139344.64mN Brg 37
Cheeseman's Green Lane Road Bridge
Tunnel Length = 6.66m



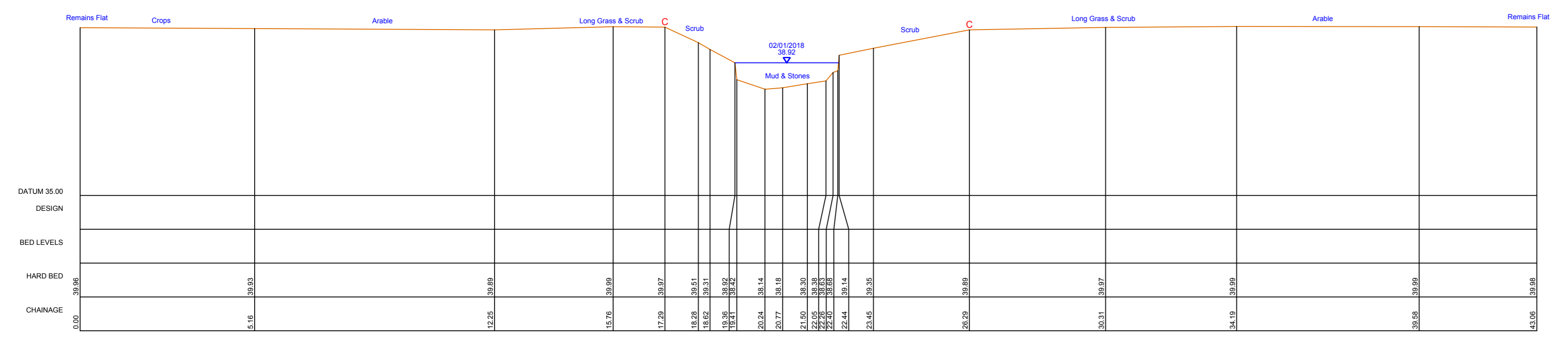
EST001_05545
603447.16mE 139337.31mN Brg 21
Open Channel



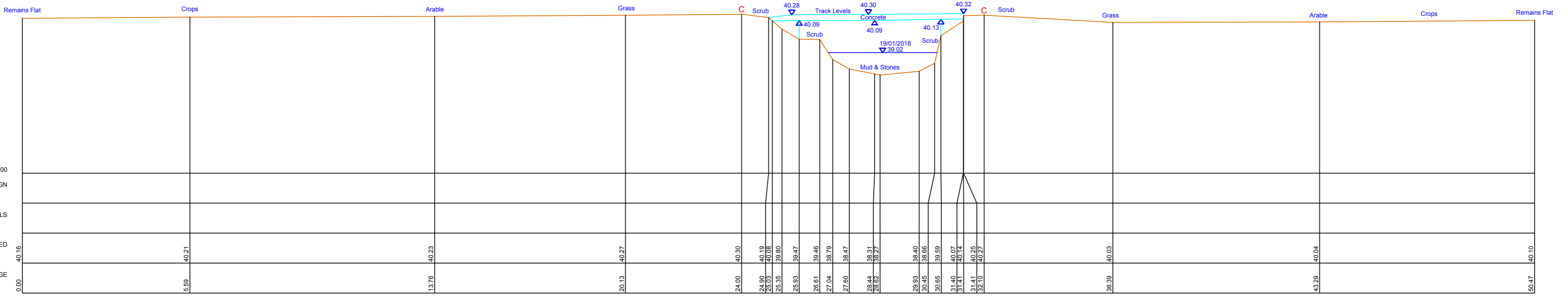
EST001_05701
603565.78mE 139302.6mN Brg 29
Open Channel



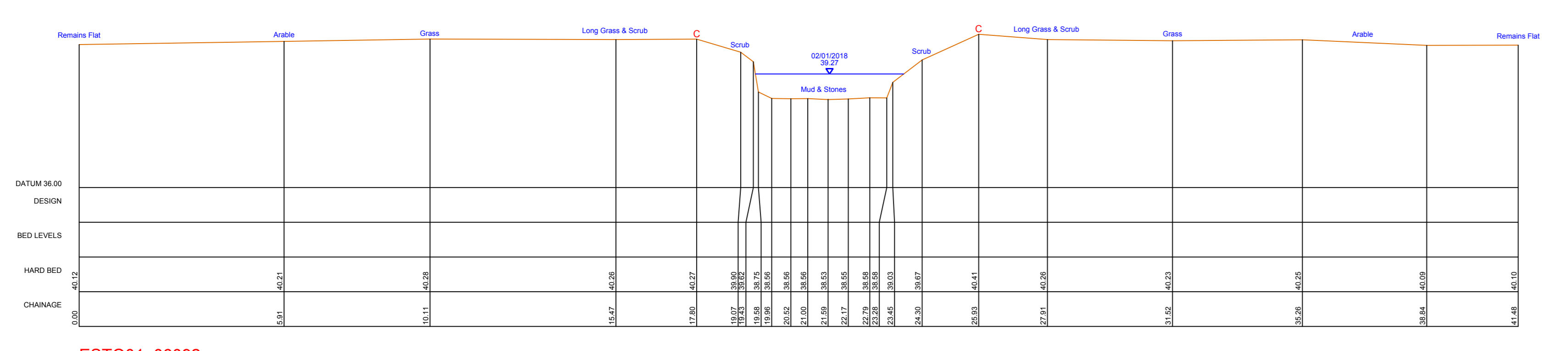
EST001_05810
603650.6mE 139248.34mN Brg 10
Open Channel



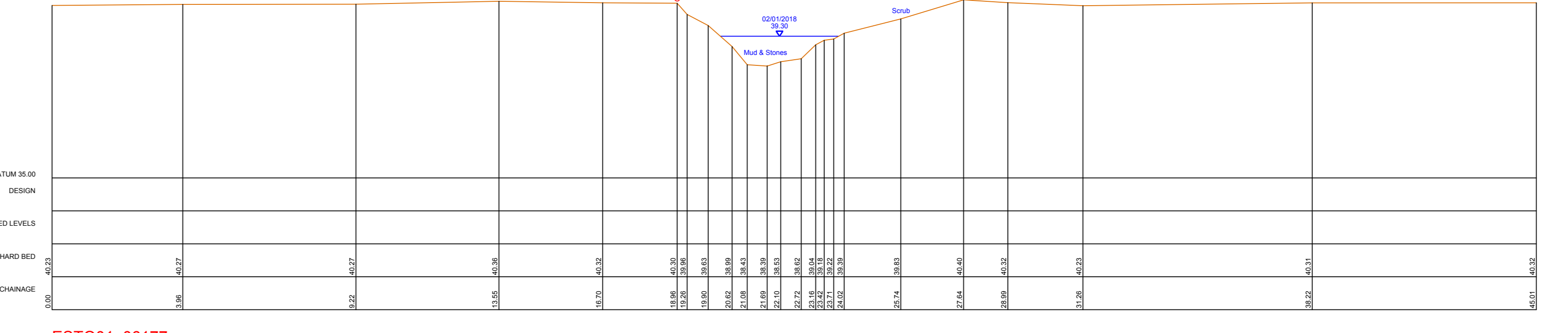
EST001_05900
603693.52mE 139266.37mN Brg 100
Open Channel



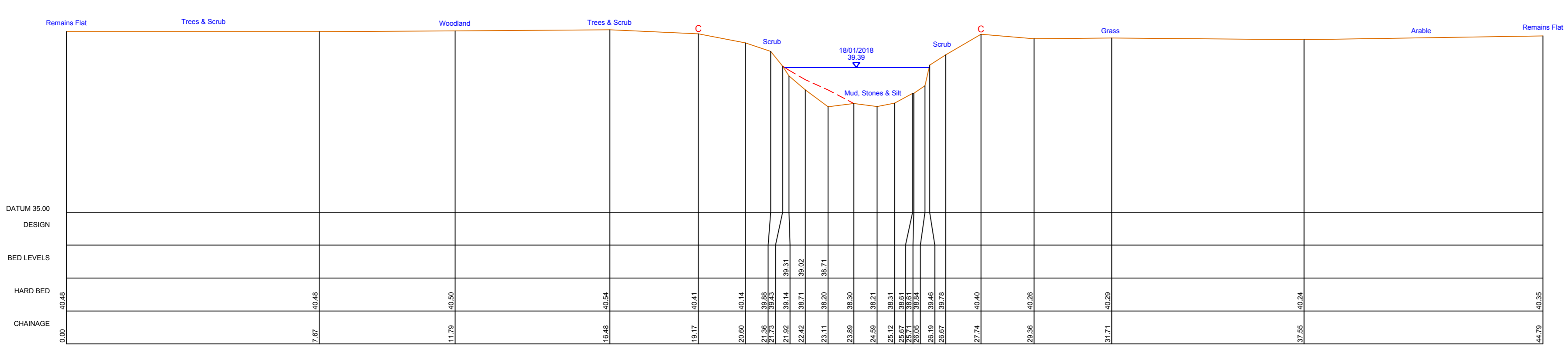
EST001_06009
603774.16mE 139210.72mN Brg 32
Access Bridge
Tunnel Length = 3.84m



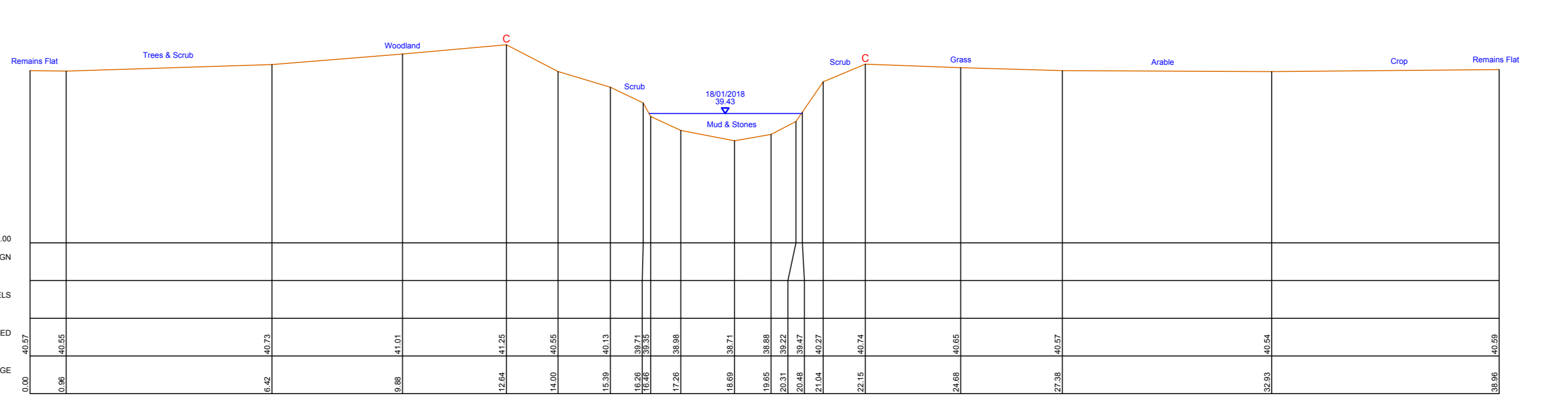
EST001_06092
603752.67mE 139168.23mN Brg 100
Open Channel



EST001_06177
603732.16mE 139080.8mN Brg 77
Open Channel



EST001_06303
603793.56mE 138981.13mN Brg 27
Open Channel



EST001_06393
603867.62mE 138960.82mN Brg 50
Open Channel

NOTES:

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SURVEY LEGEND

AD	AD BENCH	FW	THE CHANNEL	FW	FRONT WALL
AK	AK BENCH	FW	FRONT WALL	FW	FRONT WALL
AK	AK BENCH	FW	FRONT WALL	FW	FRONT WALL
AK	AK BENCH	FW	FRONT WALL	FW	FRONT WALL
AK	AK BENCH	FW	FRONT WALL	FW	FRONT WALL

AMENDMENT

NO	DESCRIPTION	DRN	CHG	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
TR	10103 4107	56.975
TR	10103 4108	56.975
TR	10103 4109	56.975
TR	10103 4110	56.975
TR	10103 4111	56.975
TR	10103 4112	56.975
TR	10103 4113	56.975
TR	10103 4114	56.975
TR	10103 4115	56.975
TR	10103 4116	56.975
TR	10103 4117	56.975
TR	10103 4118	56.975
TR	10103 4119	56.975
TR	10103 4120	56.975
TR	10103 4121	56.975
TR	10103 4122	56.975
TR	10103 4123	56.975
TR	10103 4124	56.975
TR	10103 4125	56.975
TR	10103 4126	56.975
TR	10103 4127	56.975
TR	10103 4128	56.975
TR	10103 4129	56.975
TR	10103 4130	56.975
TR	10103 4131	56.975
TR	10103 4132	56.975
TR	10103 4133	56.975
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TR	10103 4135	56.975
TR	10103 4136	56.975
TR	10103 4137	56.975
TR	10103 4138	56.975
TR	10103 4139	56.975
TR	10103 4140	56.975
TR	10103 4141	56.975
TR	10103 4142	56.975
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TR	10103 4147	56.975
TR	10103 4148	56.975
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TR	10103 4152	56.975
TR	10103 4153	56.975
TR	10103 4154	56.975
TR	10103 4155	56.975
TR	10103 4156	56.975
TR	10103 4157	56.975
TR	10103 4158	56.975
TR	10103 4159	56.975
TR	10103 4160	56.975

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Ordnance Park, London Road, Addington, West Malling, Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
EAST STOUR (EST001)
CROSS SECTIONS
EST001_05532 TO EST001_06393

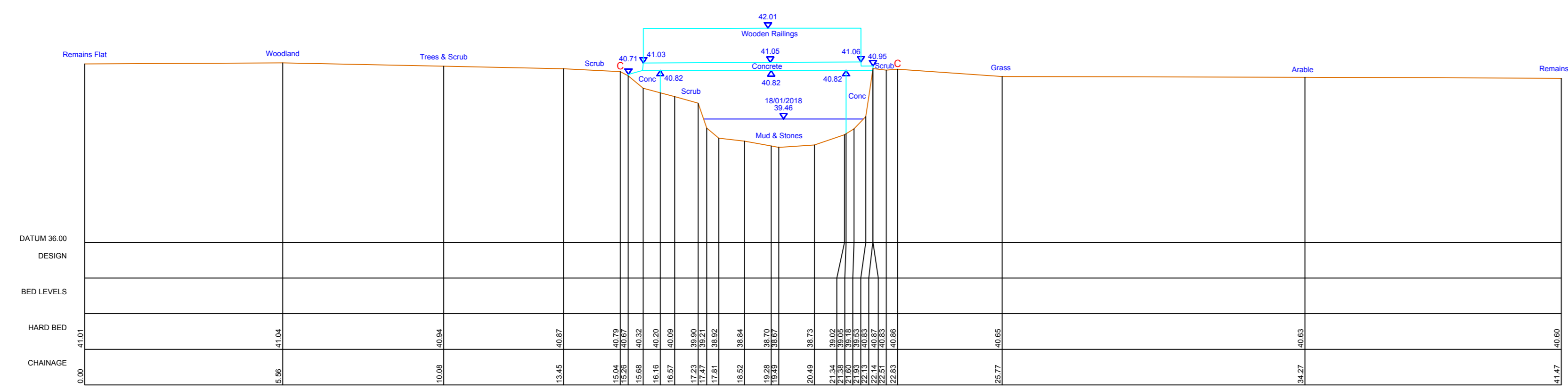
SURVEYED BY: MALTBY LAND SURVEYS LTD
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100
DATUM: OS GPS ACTIVE
GRID: NATIONAL GRID
DRAWING NO.: X-J01058-11

KEY TO SECTIONS:

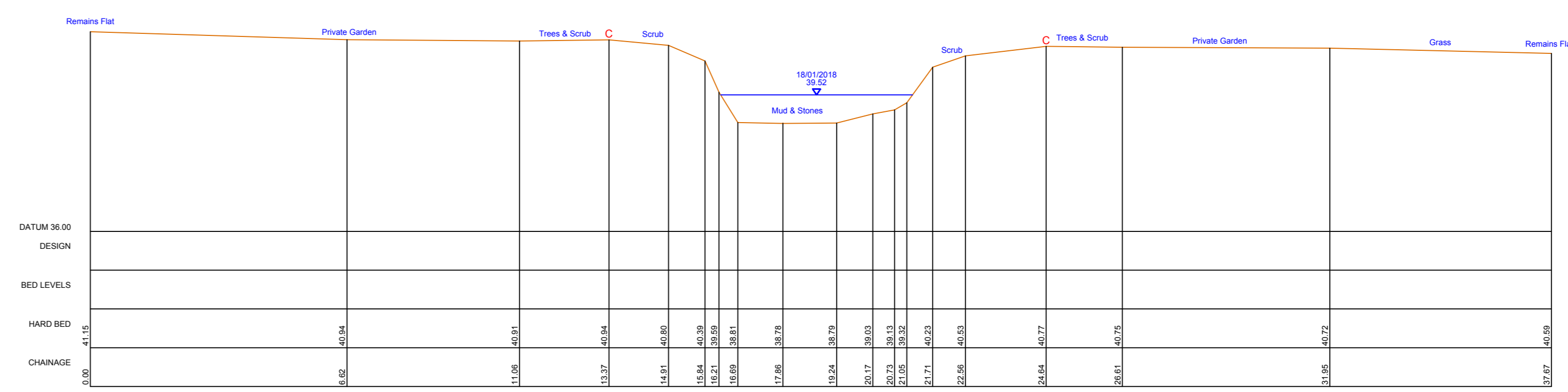
- WATER LEVEL
- VISIBLE BED (TOP OF SILT) AND GROUND
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

KEY TO LONGITUDINAL SECTION ONLY:

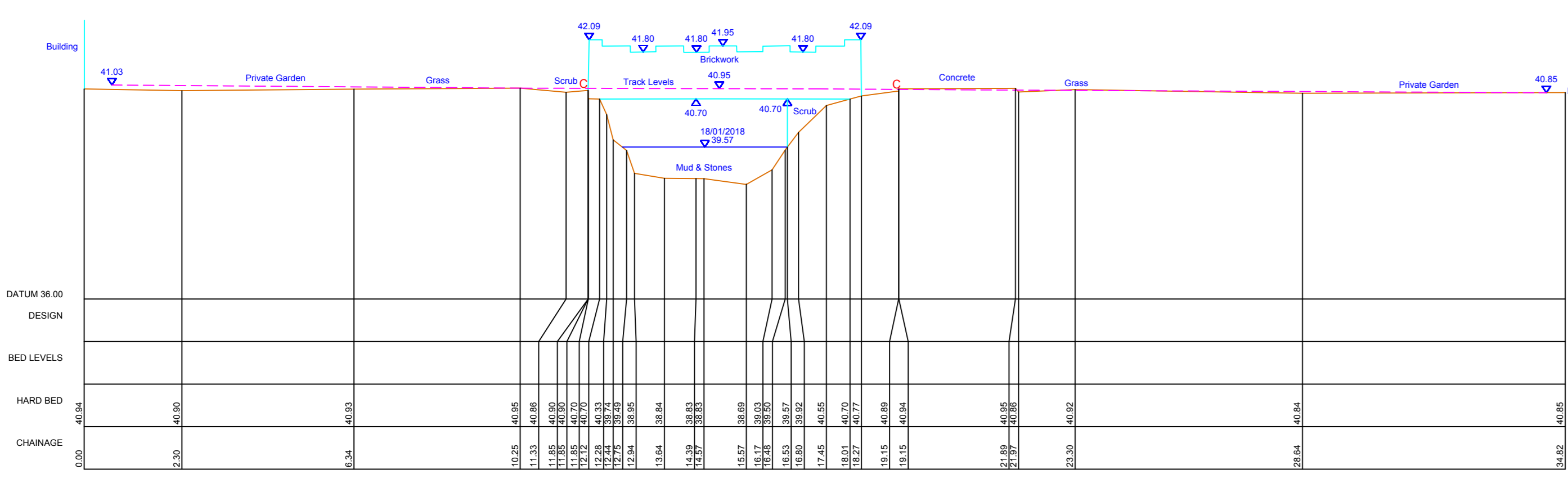
- VIEWED LOOKING DOWNSTREAM
- POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS
- LEFT BANK CREST
- RIGHT BANK CREST



ESTO01_06443
603896.67mE 138929.83mN Brg 54
Footbridge
Tunnel Length = 0.46m



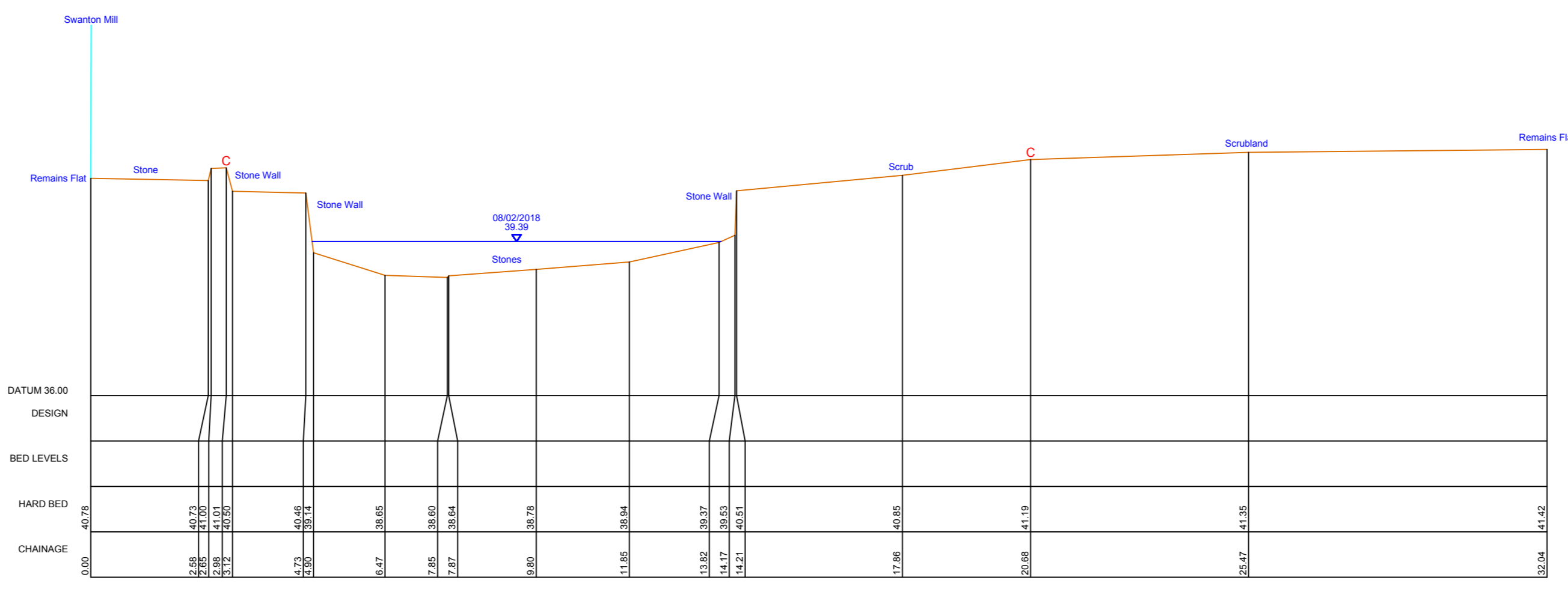
ESTO01_06489
603881.83mE 138903.75mN Brg 95
Open Channel



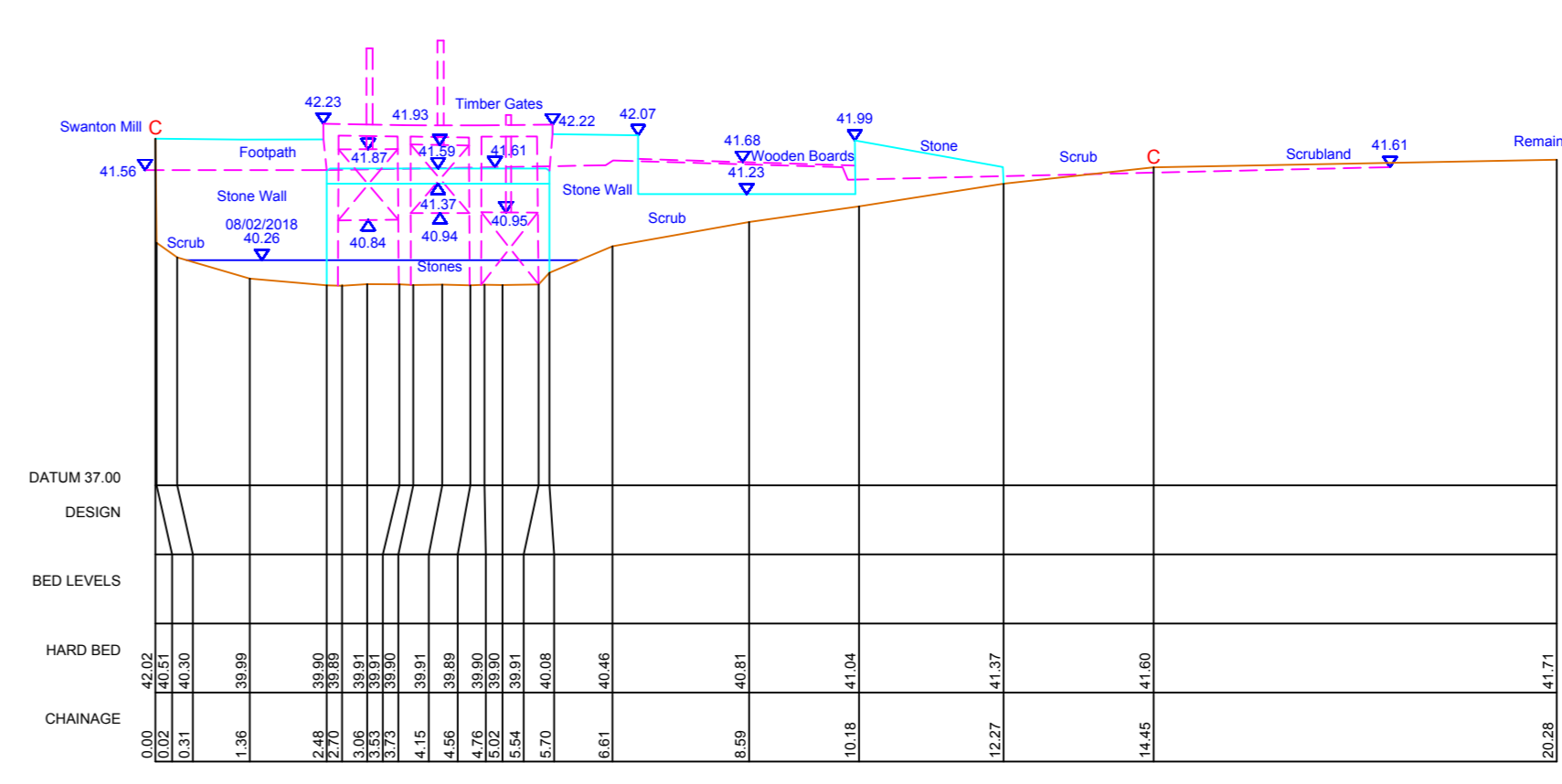
ESTO01_06503
603886.5mE 138893.88mN Brg 113
Footbridge
Tunnel Length = 2.83m
Skew Angle = 20°



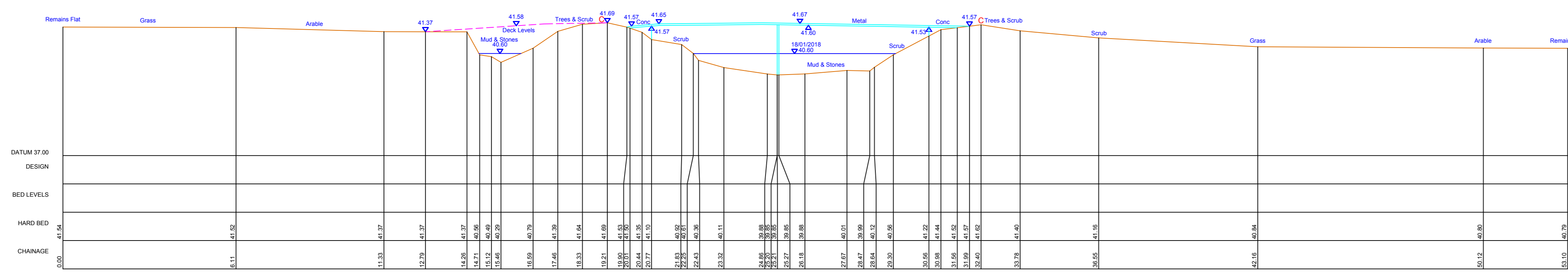
Through Section ESTO01_06558
Swanton Mill Sluice Gate



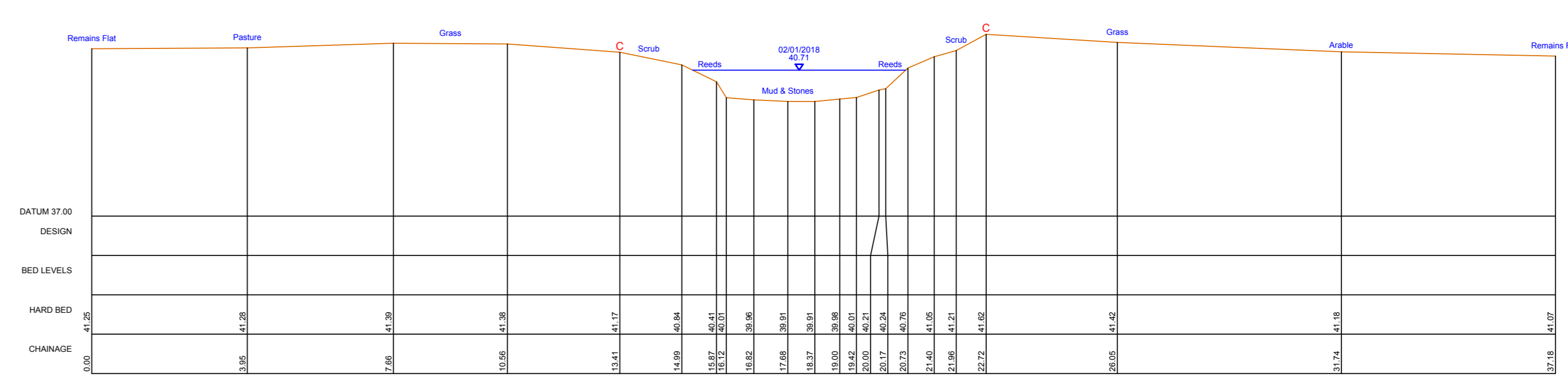
ESTO01_06550
603889.69mE 138850.16mN Brg 132
Open Channel



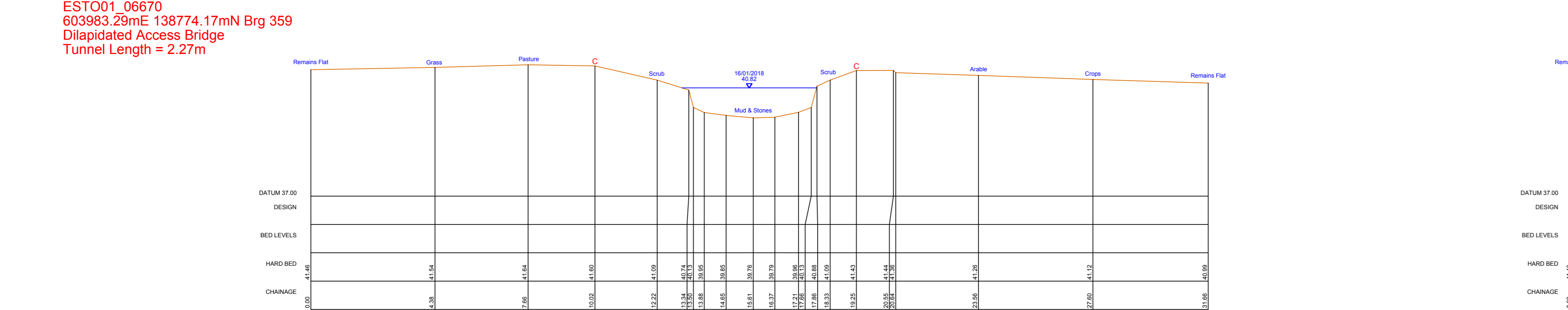
ESTO01_06558
603886.62mE 138841.11mN Brg 131
Swanton Mill Sluice Gate
Tunnel Length = 2.09m



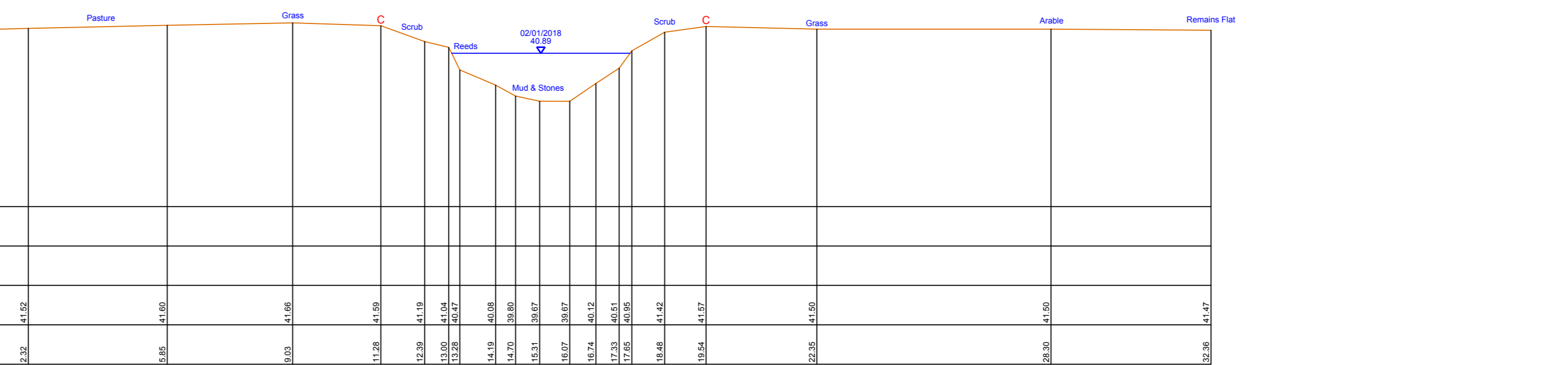
ESTO01_06591
603960.12mE 138796.5mN Brg 31
Open Channel



ESTO01_06810
604116.42mE 138833.12mN Brg 324
Open Channel

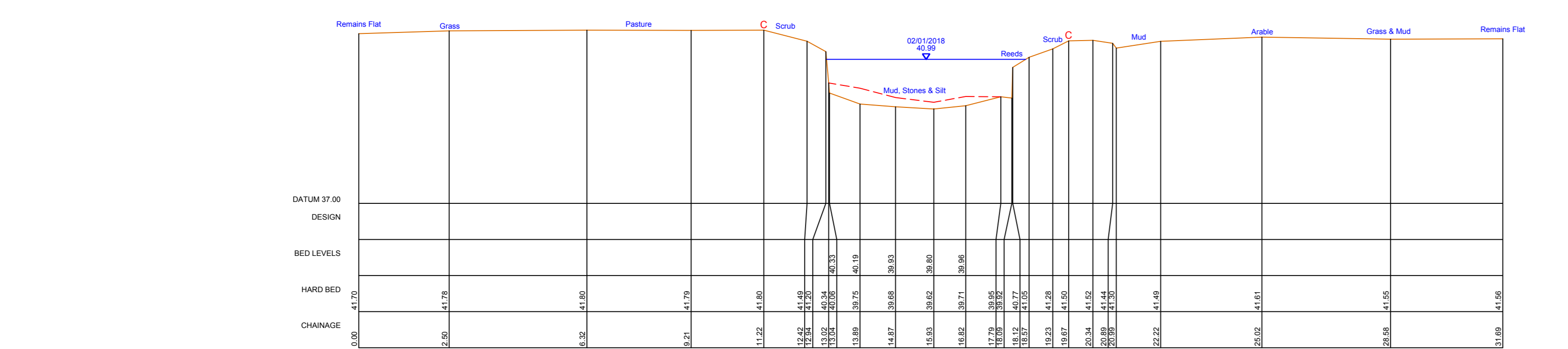


ESTO01_06670
603983.28mE 138774.17mN Brg 359
Dilapidated Access Bridge
Tunnel Length = 2.27m

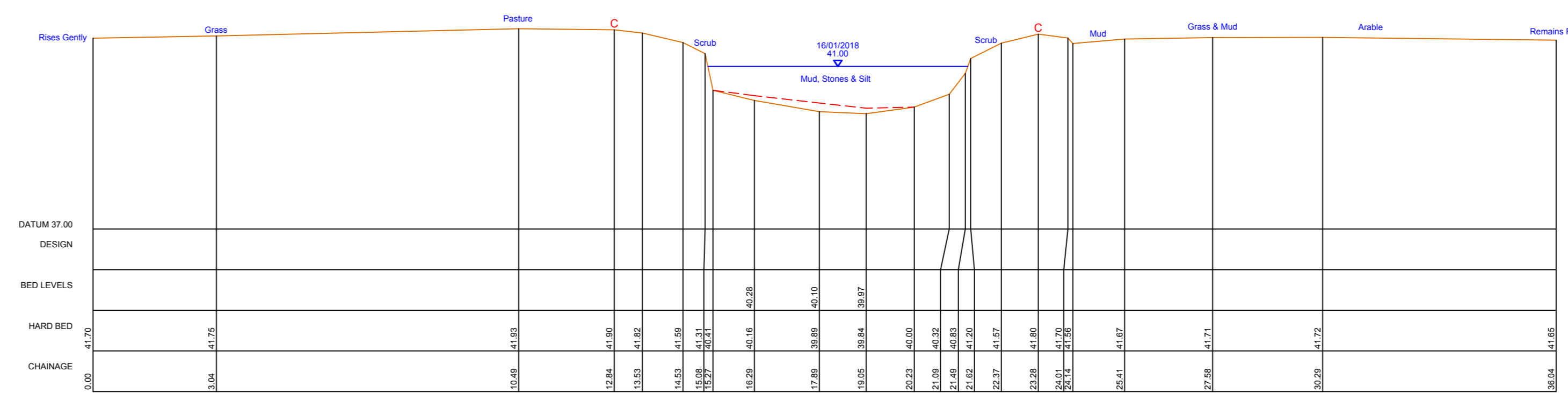


ESTO01_06916
604195.62mE 138872.97mN Brg 342
Open Channel

ESTO01_07016
604291.07mE 138910.06mN Brg 333
Open Channel



ESTO01_07119
604367.9mE 138950.66mN Brg 6
Open Channel



ESTO01_07212
604467.12mE 138969.56mN Brg 328
Open Channel

NOTES:

- A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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SURVEY LEGEND

AS BENCH	AS BENCH	AS BENCH	AS BENCH
...

AMENDMENT

NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
...

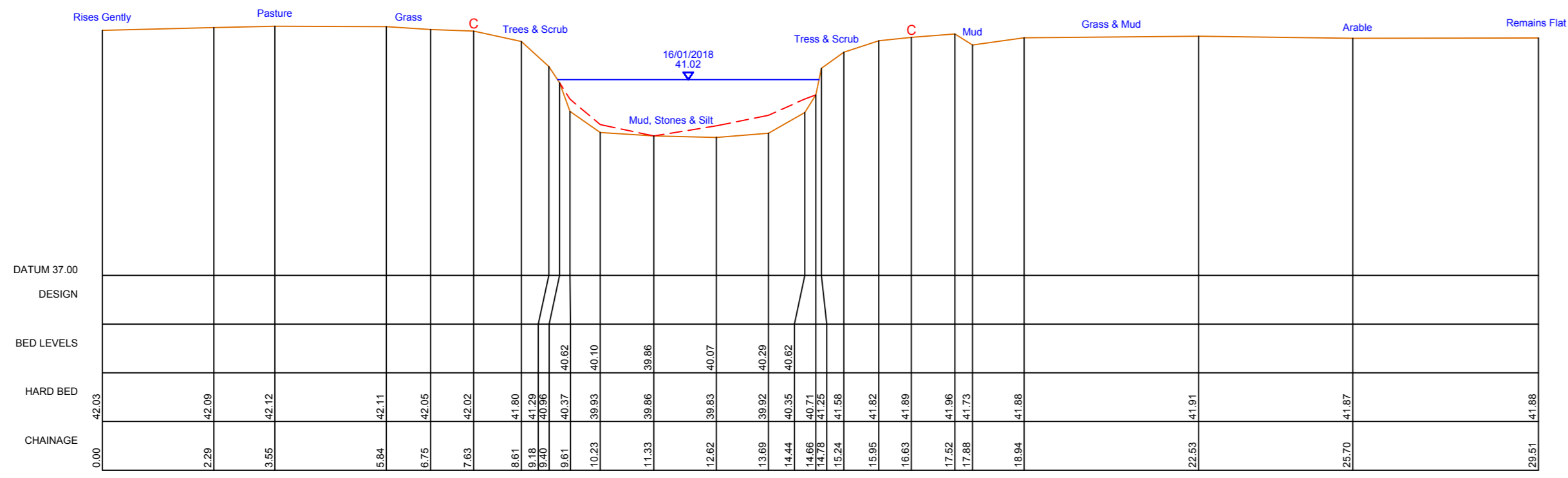
Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Ordnance Park, London Road, Addington, West Malling, Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

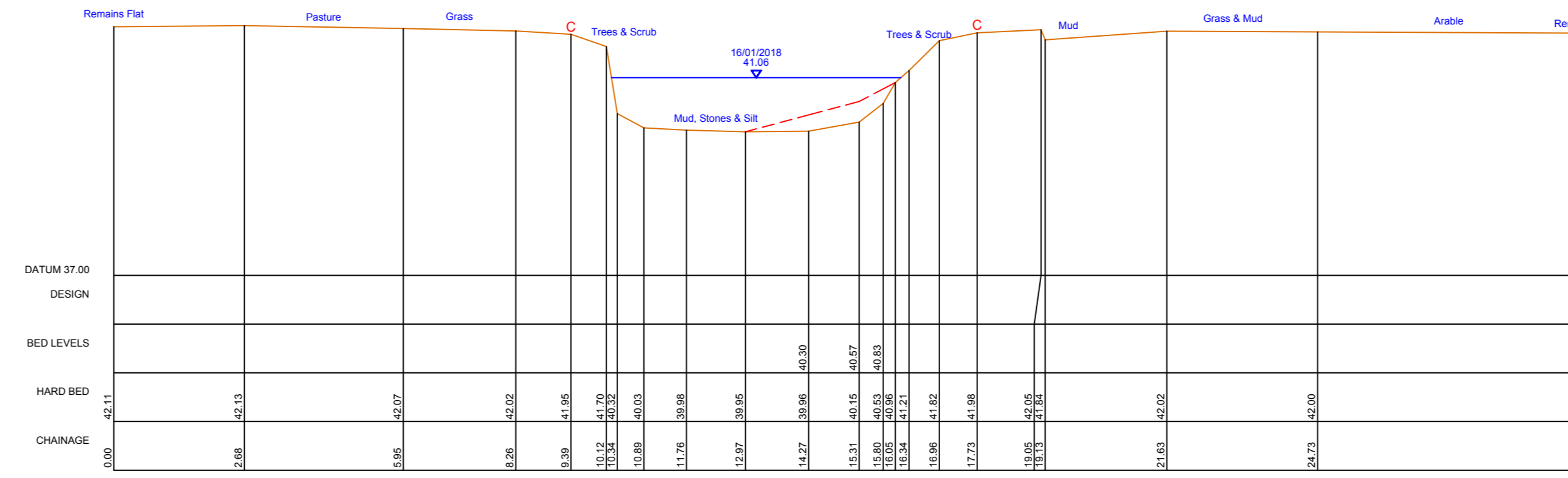
SITE/UMTS
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_06443 TO ESTO01_07212

SURVEYED BY: MALTBY LAND SURVEYS LTD Rev: 12_157
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100 DRN: RC CHKD: ITS
DATUM: OS GPS ACTIVE DATE: MAR 18 DATE: MAR 18
GRID: NATIONAL GRID DRAWING NO. REV.
DWG FILENAME: A-2018-01-30.dwg X-J01058-12

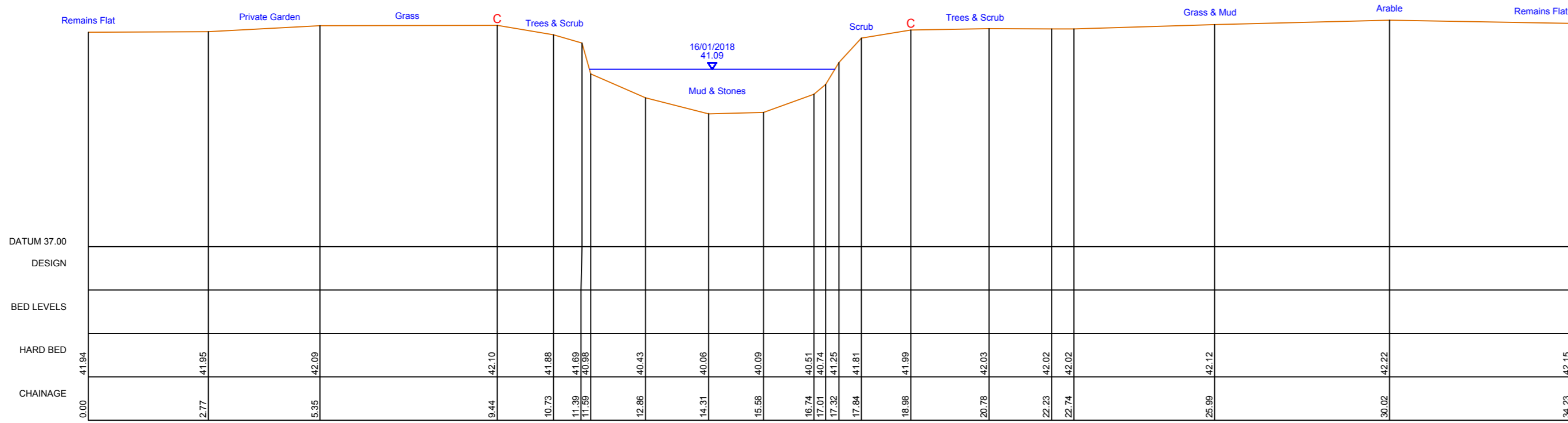
- KEY TO SECTIONS:**
- WATER LEVEL
 - VISIBLE BED (TOP OF SILT) AND GROUND
 - HARD BED (DETERMINED BY PROBING)
 - BANK CREST
- KEY TO LONGITUDINAL SECTION ONLY:**
- VIEWED LOOKING DOWNSTREAM
 - LEFT BANK CREST
 - RIGHT BANK CREST
- POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



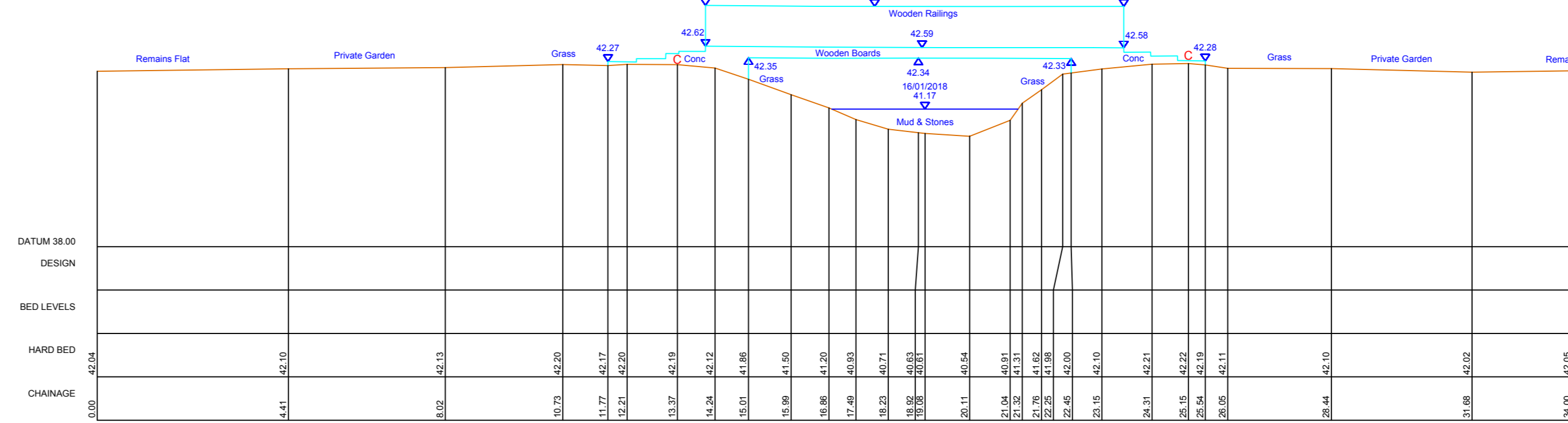
ESTO01_07310
604534.44mE 138979.21mN Brg 15
Open Channel



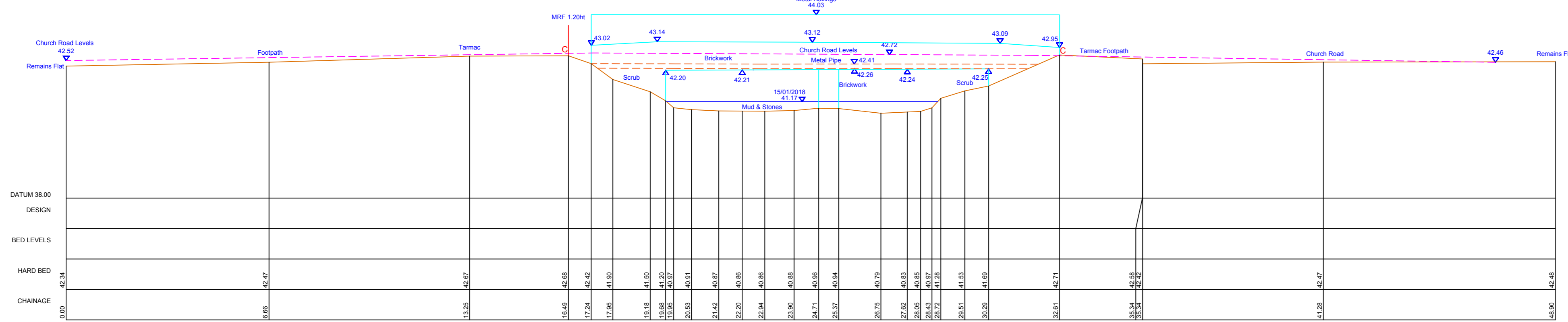
ESTO01_07416
604633.97mE 138983.07mN Brg 29
Open Channel



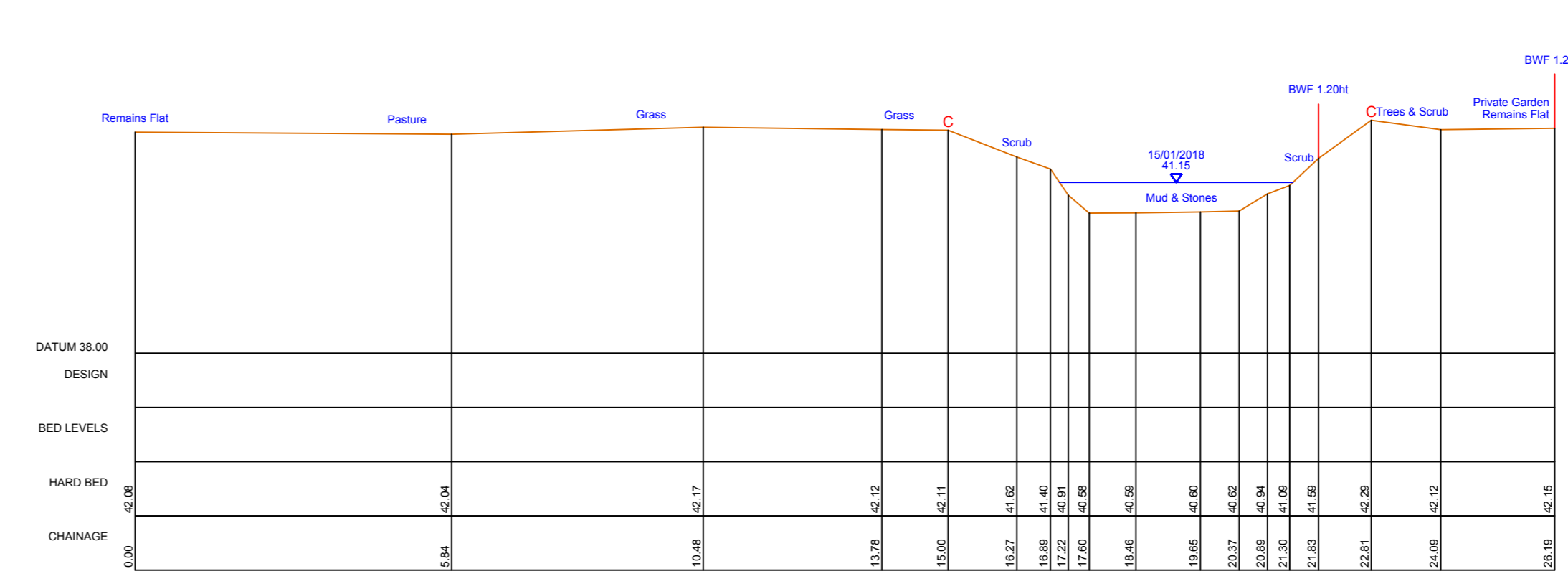
ESTO01_07513
604716.71mE 138972.35mN Brg 306
Open Channel



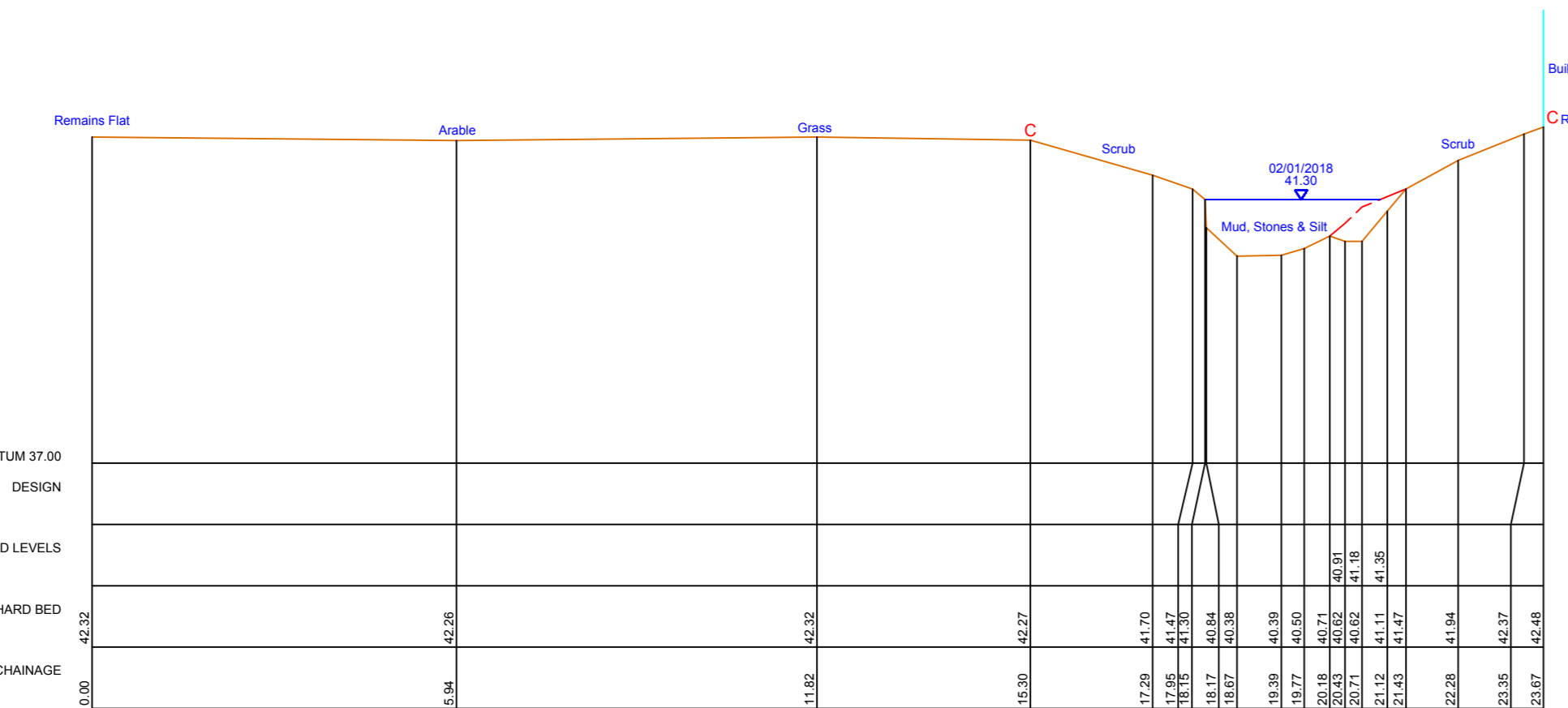
ESTO01_07572
604739.82mE 139016.46mN Brg 330
Footbridge
Tunnel Length = 0.86m



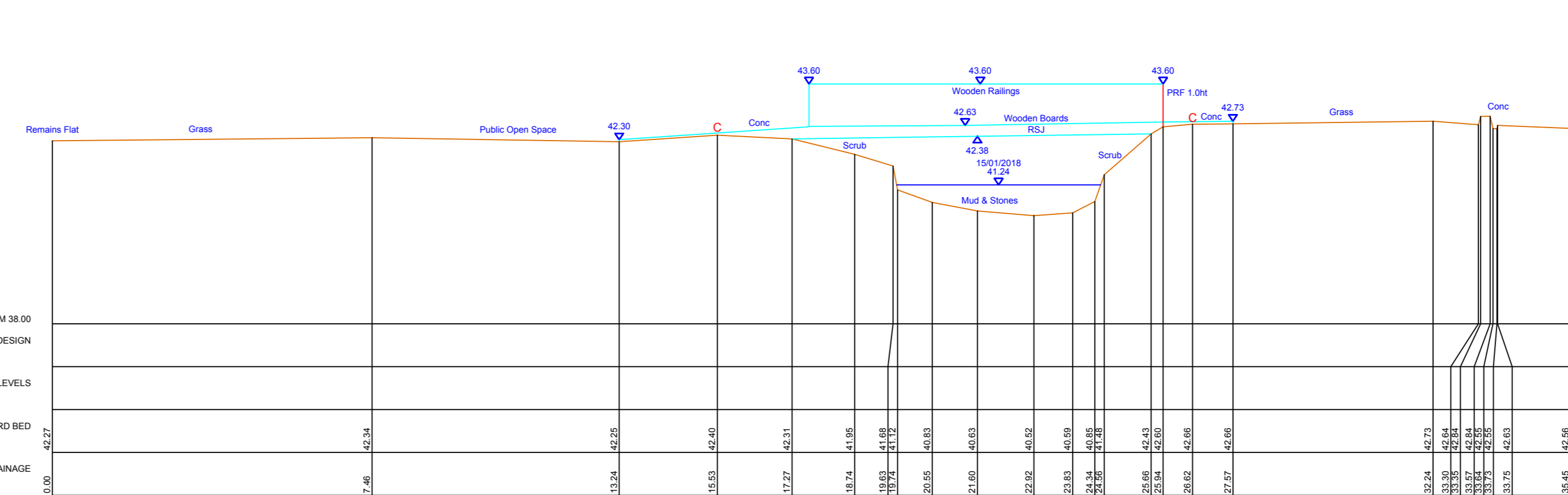
ESTO01_07600
604751.12mE 139016.01mN Brg 13
Church Road Bridge
Tunnel Length = 6.06m



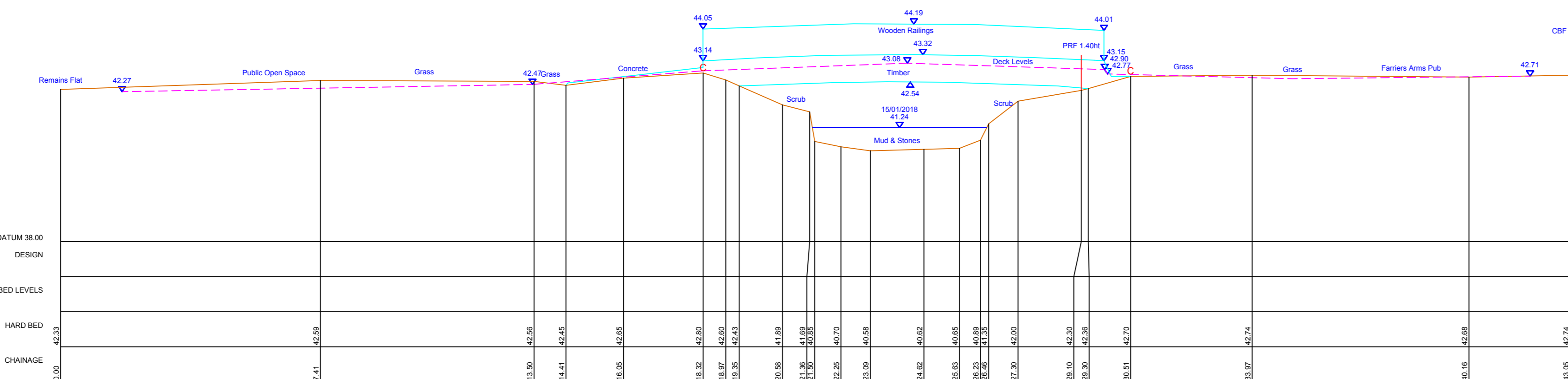
ESTO01_07611
604775.54mE 139029.42mN Brg 329
Open Channel



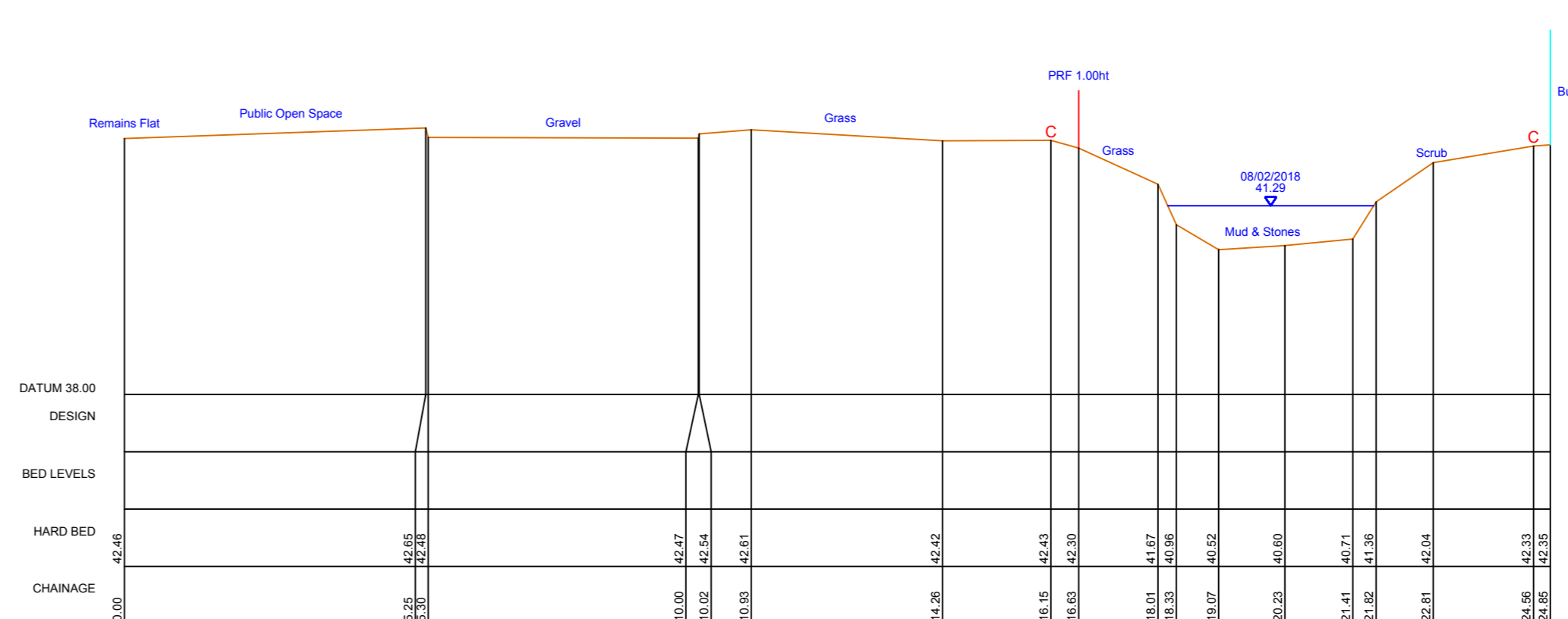
ESTO01_07716
604859.11mE 139059.32mN Brg 4
Open Channel



ESTO01_07765
604916.22mE 139070.08mN Brg 324
Footbridge
Tunnel Length = 1.09m



ESTO01_07779
604915.42mE 139067.22mN Brg 4
Footbridge
Tunnel Length = 2.10m



ESTO01_07792
604927.17mE 139070.26mN Brg 8
Open Channel

NOTES:

- A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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- UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

AS BED	AS BED	AS BED	AS BED
...

AMENDMENT

NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
...

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Leisure Park, London Road, Addington, West Malling, Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_07310 TO ESTO01_07792

SURVEYED BY: MALTBY LAND SURVEYS LTD
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100
DATUM: OS GPS ACTIVE
GRID: NATIONAL GRID

DRN: RC
DATE: MAR 18
CHKD: ITS
DATE: MAR 18
DRAWING NO. X-J01058-13

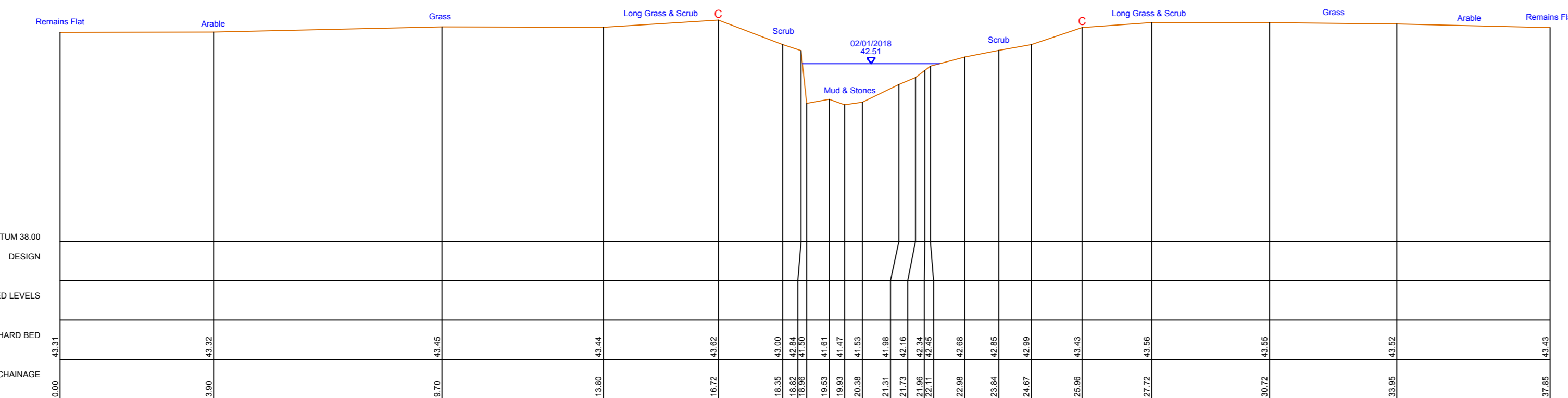
KEY TO SECTIONS:

— WATER LEVEL
 --- VISBLE BED (TOP OF SILT) AND GROUND
 --- HARD BED (DETERMINED BY PROBING)
 C BANK CREST

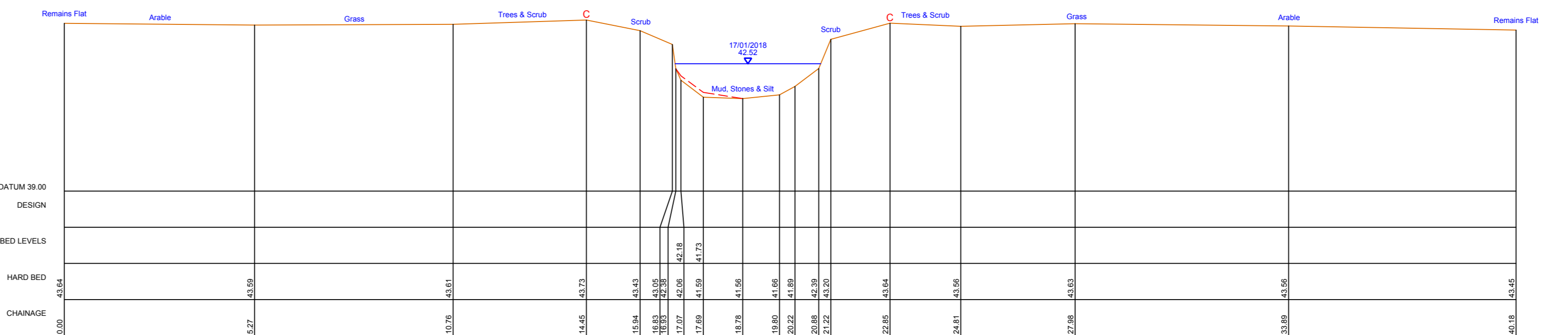
KEY TO LONGITUDINAL SECTION ONLY:

--- LEFT BANK CREST
 --- RIGHT BANK CREST

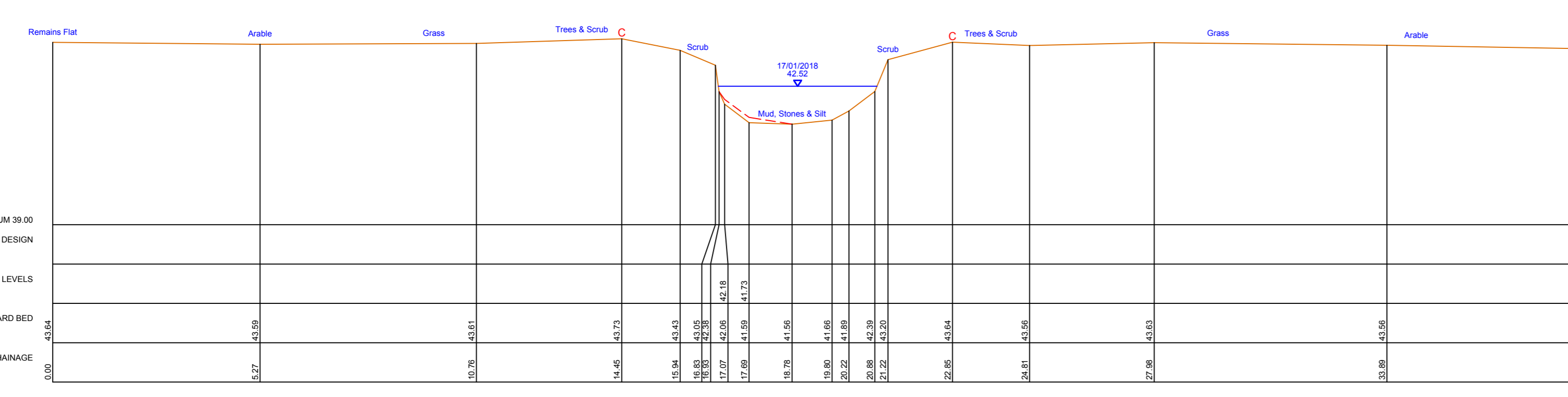
POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



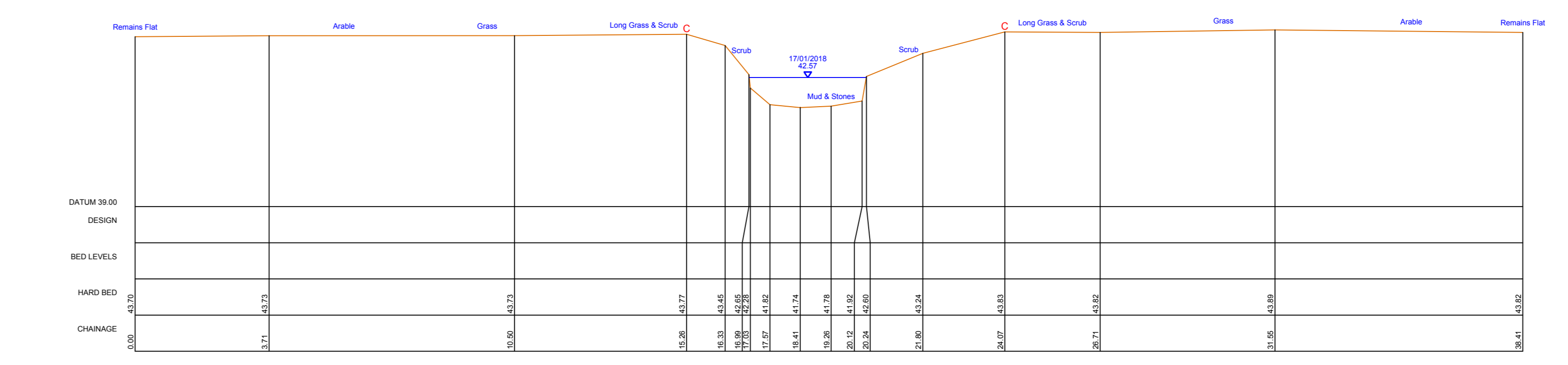
ESTO01_08523
 605268.65mE 138600.04mN Brg 90
 Open Channel



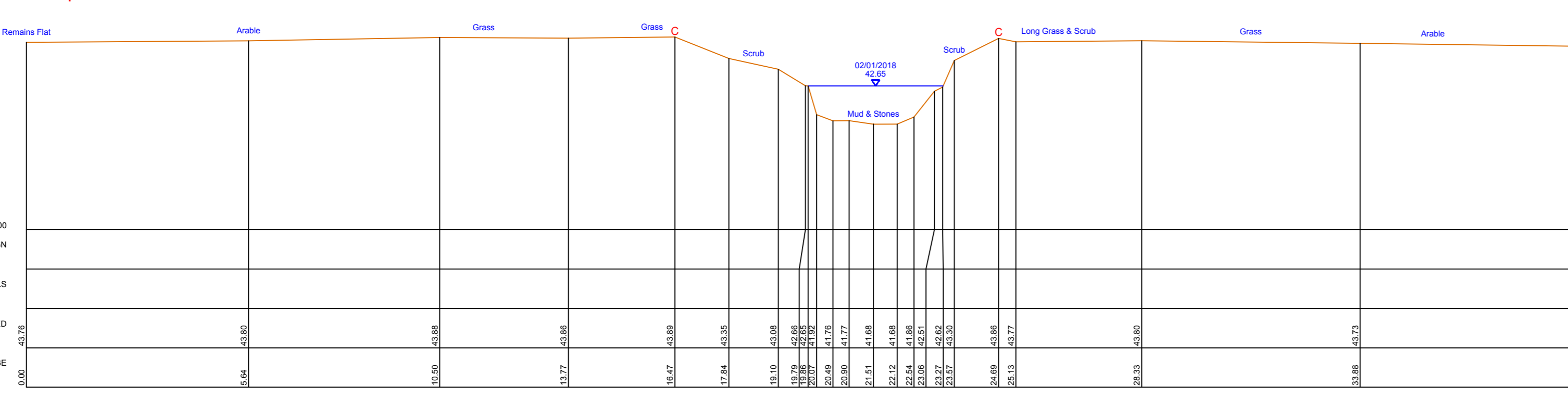
ESTO01_08616
 605247.32mE 138535.28mN Brg 118
 Open Channel



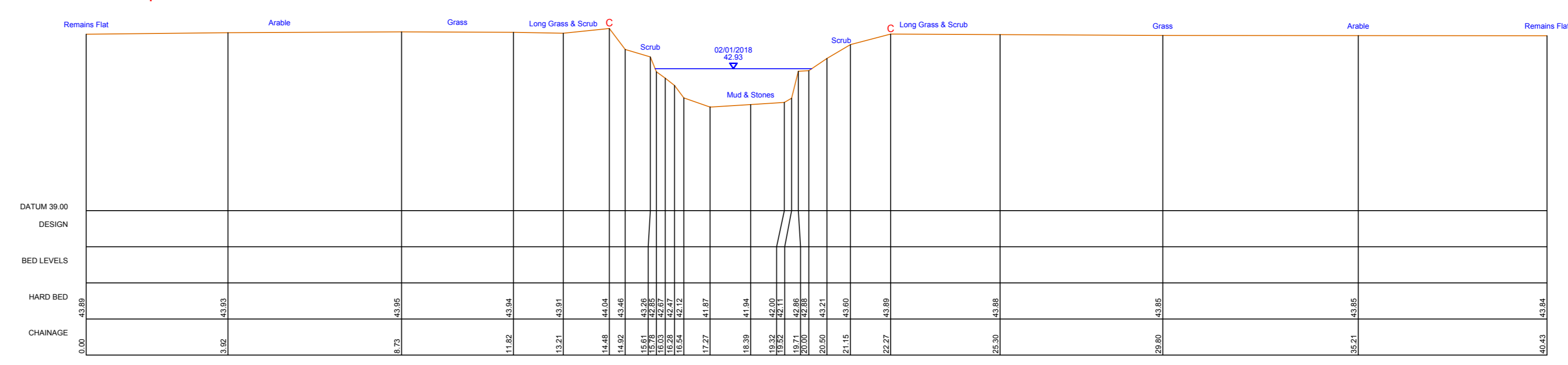
ESTO01_08616
 605247.32mE 138535.28mN Brg 118
 Open Channel



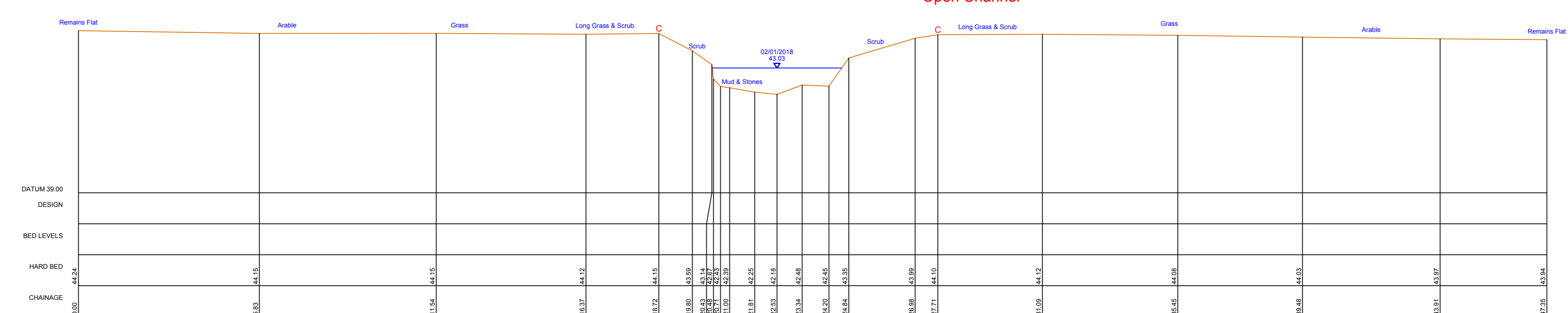
ESTO01_08720
 605245.76mE 138434.22mN Brg 100
 Open Channel



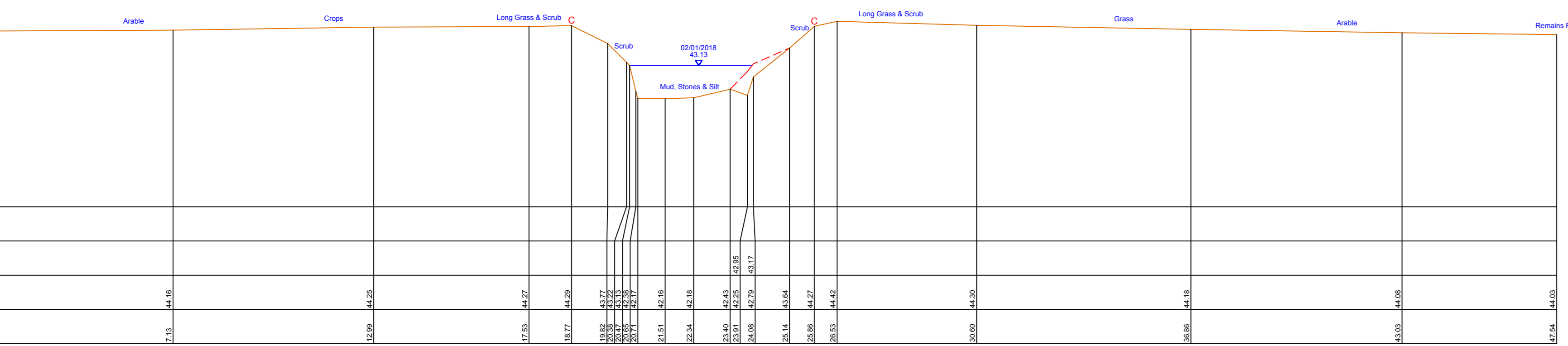
ESTO01_08836
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 Open Channel



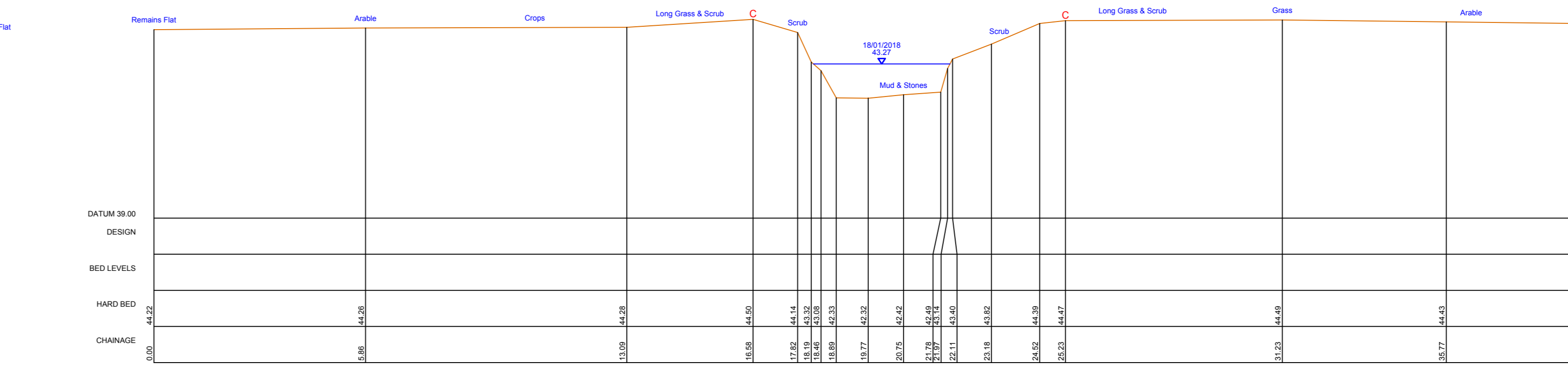
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 Open Channel



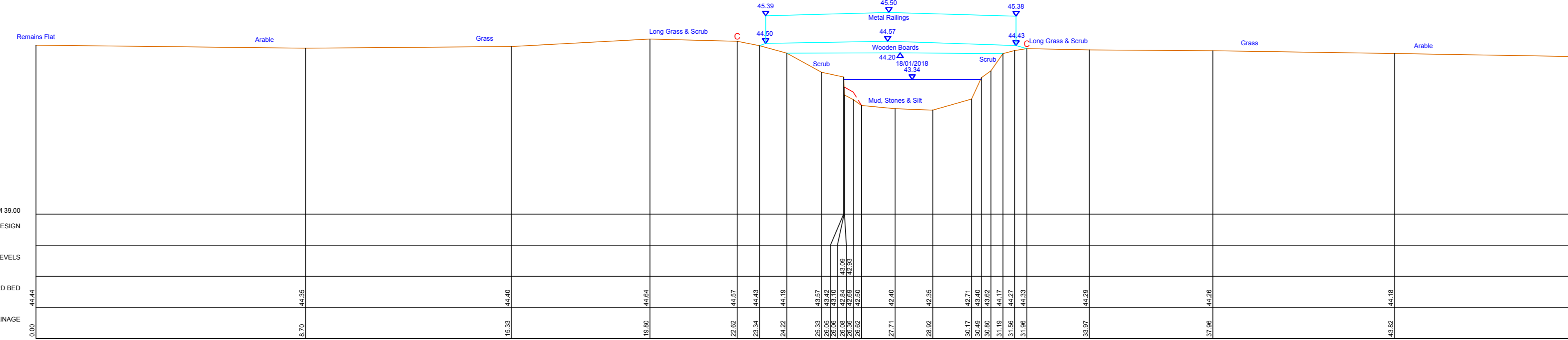
ESTO01_09022
 605443.8mE 138207.2mN Brg 14
 Open Channel



ESTO01_09119
 605553.15mE 138237.24mN Brg 312
 Open Channel



ESTO01_09226
 605592.15mE 138260.81mN Brg 68
 Open Channel



ESTO01_09305
 605666.94mE 138248.56mN Brg 7
 Footbridge
 Tunnel Length = 1.33m

NOTES:

1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
 2. THIS MAP IS REPRODUCED FROM THE OS MAP BY THE ENVIRONMENT AGENCY WITH PERMISSION OF ORDNANCE SURVEY ON BEHALF OF THE CONTROLLER OF HER MAJESTY'S STATIONERY OFFICE. © CROWN COPYRIGHT LICENCE. ALL RIGHTS RESERVED. UNAUTHORISED REPRODUCTION INFRINGES CROWN COPYRIGHT AND MAY LEAD TO PROSECUTION OR CIVIL PROCEEDINGS. LICENCE NO. 100026380.
 3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND	
AP	Apple
AS	Asphalt
B	Bank
BR	Brick
BU	Bush
CG	Concrete
CH	Channel
CP	Concrete Paving
CS	Concrete Slab
CR	Concrete Road
CT	Concrete Trench
CU	Concrete Underpass
DA	Dark Area
DC	Dark Concrete
DI	Ditch
DL	Dark Line
DM	Dark Mud
DN	Dark Noise
DP	Dark Point
DR	Dark Road
DS	Dark Stone
DT	Dark Trench
DU	Dark Underpass
DV	Dark Valley
DW	Dark Wall
DZ	Dark Zone
EA	Earth Area
EB	Earth Bank
EC	Earth Concrete
ED	Earth Ditch
EE	Earth Embankment
EF	Earth Fence
EG	Earth Gate
EH	Earth Hill
EI	Earth Inlet
EJ	Earth Junction
EK	Earth Kerb
EL	Earth Level
EM	Earth Mound
EN	Earth Noise
EO	Earth Offset
EP	Earth Point
EQ	Earth Quarter
ER	Earth Road
ES	Earth Slope
ET	Earth Trench
EU	Earth Underpass
EV	Earth Valley
EW	Earth Wall
EX	Earth Zone
FA	Fabric Area
FB	Fabric Bank
FC	Fabric Concrete
FD	Fabric Ditch
FE	Fabric Embankment
FF	Fabric Fence
FG	Fabric Gate
FH	Fabric Hill
FI	Fabric Inlet
FJ	Fabric Junction
FK	Fabric Kerb
FL	Fabric Level
FM	Fabric Mound
FN	Fabric Noise
FO	Fabric Offset
FP	Fabric Point
FQ	Fabric Quarter
FR	Fabric Road
FS	Fabric Slope
FT	Fabric Trench
FU	Fabric Underpass
FV	Fabric Valley
FW	Fabric Wall
FX	Fabric Zone
GA	Grass Area
GB	Grass Bank
GC	Grass Concrete
GD	Grass Ditch
GE	Grass Embankment
GF	Grass Fence
GG	Grass Gate
GH	Grass Hill
GI	Grass Inlet
GJ	Grass Junction
GK	Grass Kerb
GL	Grass Level
GM	Grass Mound
GN	Grass Noise
GO	Grass Offset
GP	Grass Point
GQ	Grass Quarter
GR	Grass Road
GS	Grass Slope
GT	Grass Trench
GU	Grass Underpass
GV	Grass Valley
GW	Grass Wall
GX	Grass Zone

AMENDMENT	DATE	BY

CONTROL USED:	DESCRIPTION	LEVEL
E00720012	TR 0103 4107	35.572
E00720013	TR 0103 4107	35.572
E00720014	TR 0103 4107	35.572
E00720015	TR 0103 4107	35.572
E00720016	TR 0103 4107	35.572
E00720017	TR 0103 4107	35.572
E00720018	TR 0103 4107	35.572
E00720019	TR 0103 4107	35.572
E00720020	TR 0103 4107	35.572
E00720021	TR 0103 4107	35.572
E00720022	TR 0103 4107	35.572
E00720023	TR 0103 4107	35.572
E00720024	TR 0103 4107	35.572
E00720025	TR 0103 4107	35.572
E00720026	TR 0103 4107	35.572
E00720027	TR 0103 4107	35.572
E00720028	TR 0103 4107	35.572
E00720029	TR 0103 4107	35.572
E00720030	TR 0103 4107	35.572
E00720031	TR 0103 4107	35.572
E00720032	TR 0103 4107	35.572
E00720033	TR 0103 4107	35.572
E00720034	TR 0103 4107	35.572
E00720035	TR 0103 4107	35.572
E00720036	TR 0103 4107	35.572
E00720037	TR 0103 4107	35.572
E00720038	TR 0103 4107	35.572
E00720039	TR 0103 4107	35.572
E00720040	TR 0103 4107	35.572

Environment Agency
 KENT & SOUTH LONDON REGION
 Orchard House, Entrance Park, London Road, Ashford, Kent, ME19 5QH

PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS:
**EAST STOUR (ESTO01)
 CROSS SECTIONS
 ESTO01_08532 TO ESTO01_09305**

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref: 12_157*

SURVEY DATE: DECEMBER 2017 - MARCH 2018

SCALE: 1:100
 DATUM: OS GPS ACTIVE
 GRID: NATIONAL GRID

DRN: RC
 DATE: MAR 18
 DRAWING NO: X-J01058-15

CHKD: ITS
 DATE: MAR 18
 REV:

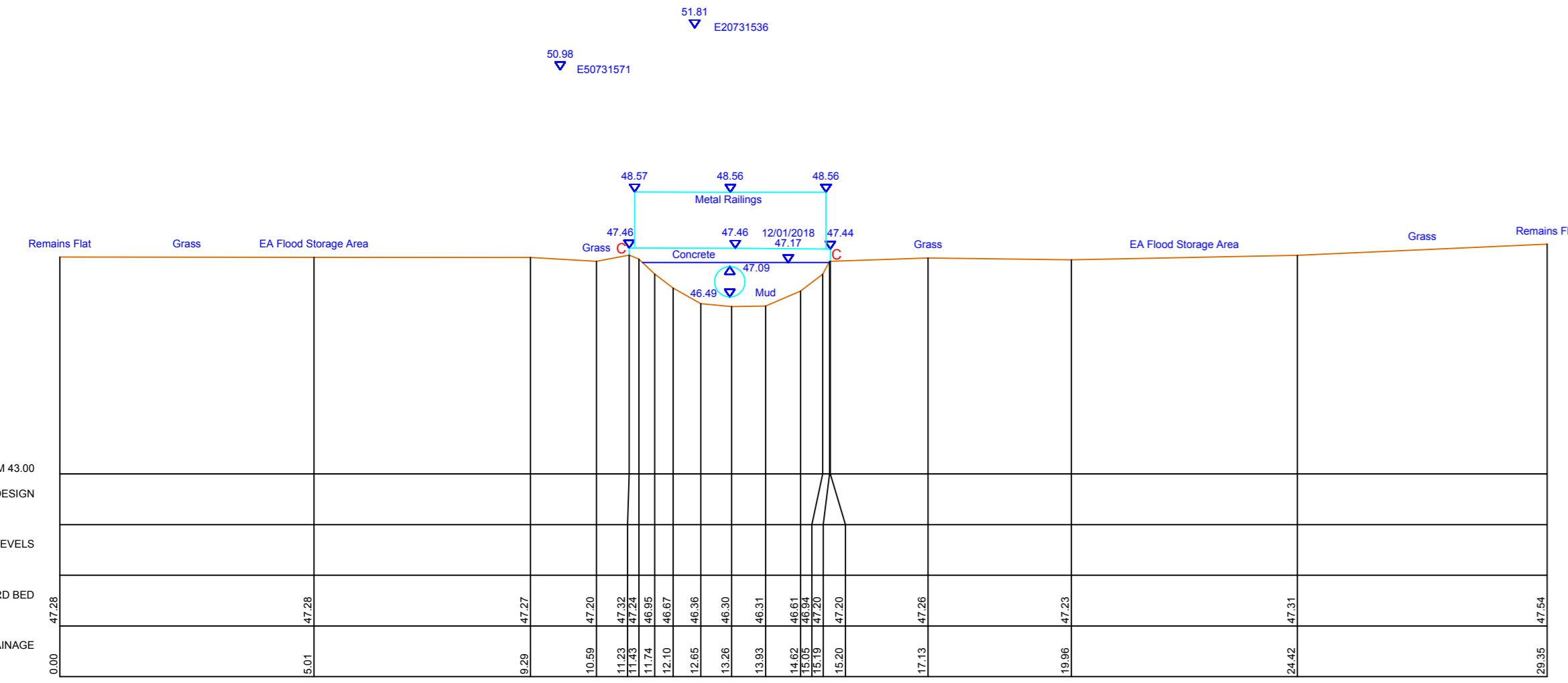
KEY TO SECTIONS:

- WATER LEVEL
- VISBLE BED (TOP OF SILT) AND GROUND
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

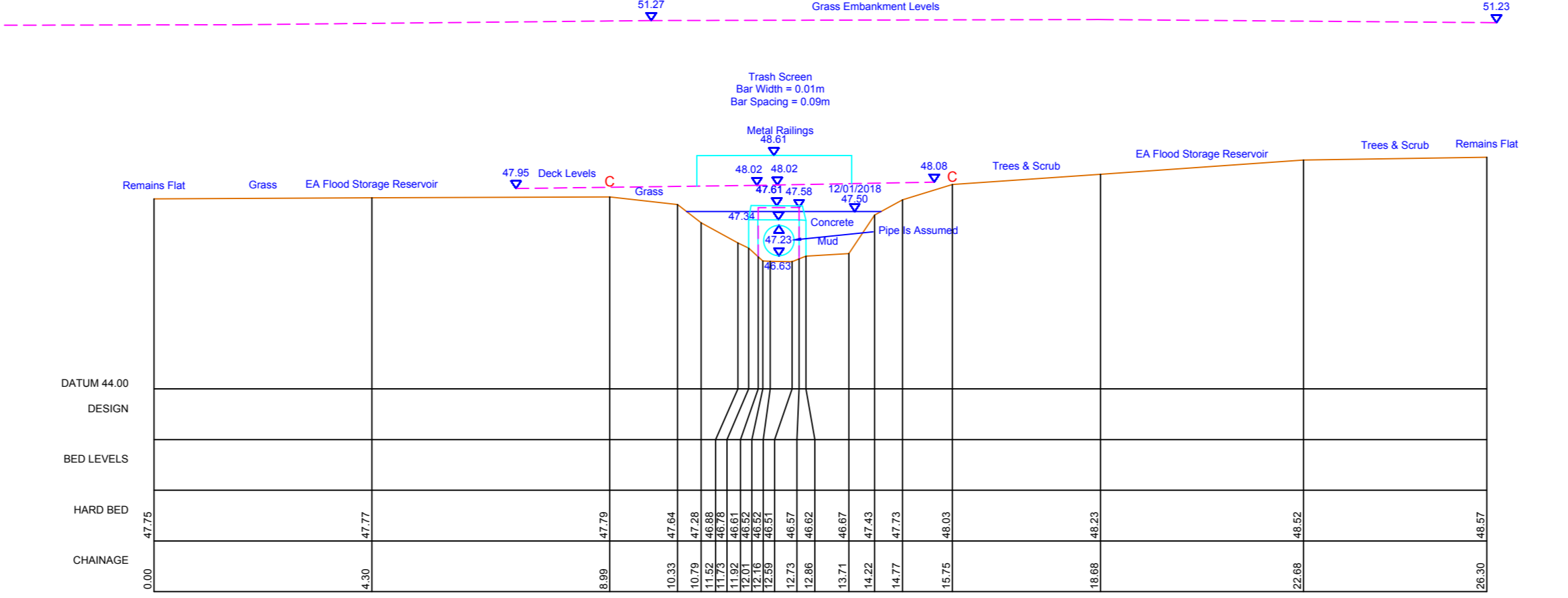
KEY TO LONGITUDINAL SECTION ONLY:

- VIEWED LOOKING DOWNSTREAM
- LEFT BANK CREST
- RIGHT BANK CREST

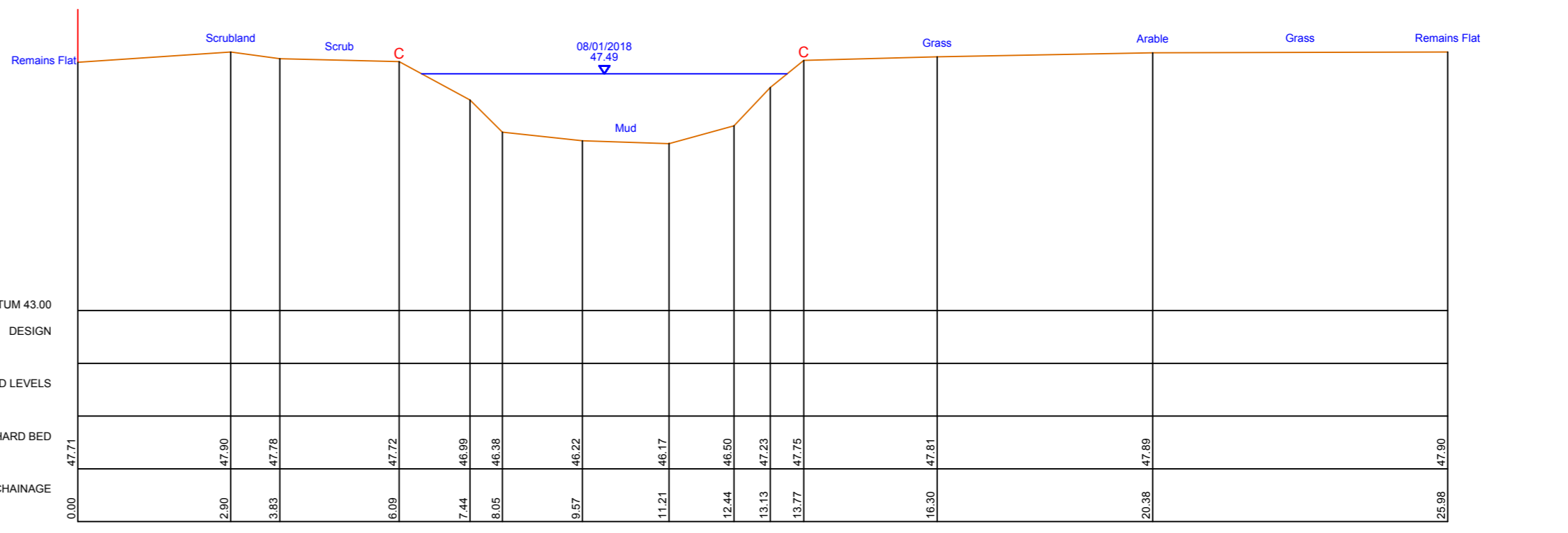
POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



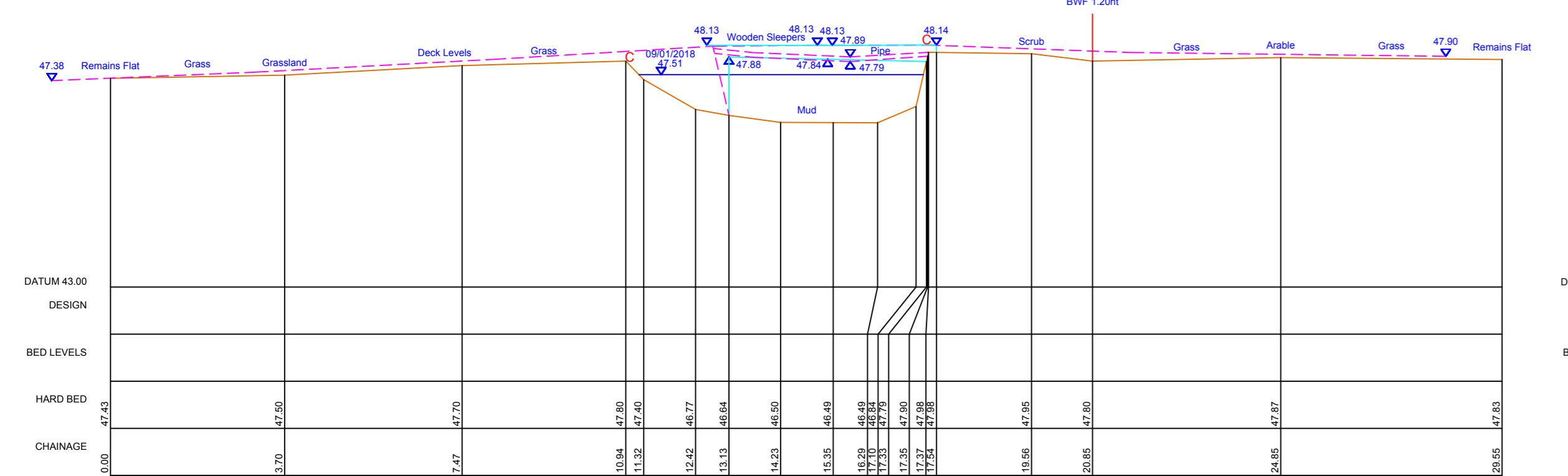
ESTO01_10528
606641.23mE 138134.91mN Brg 19
Culvert Exit
Tunnel Length = 46.12m



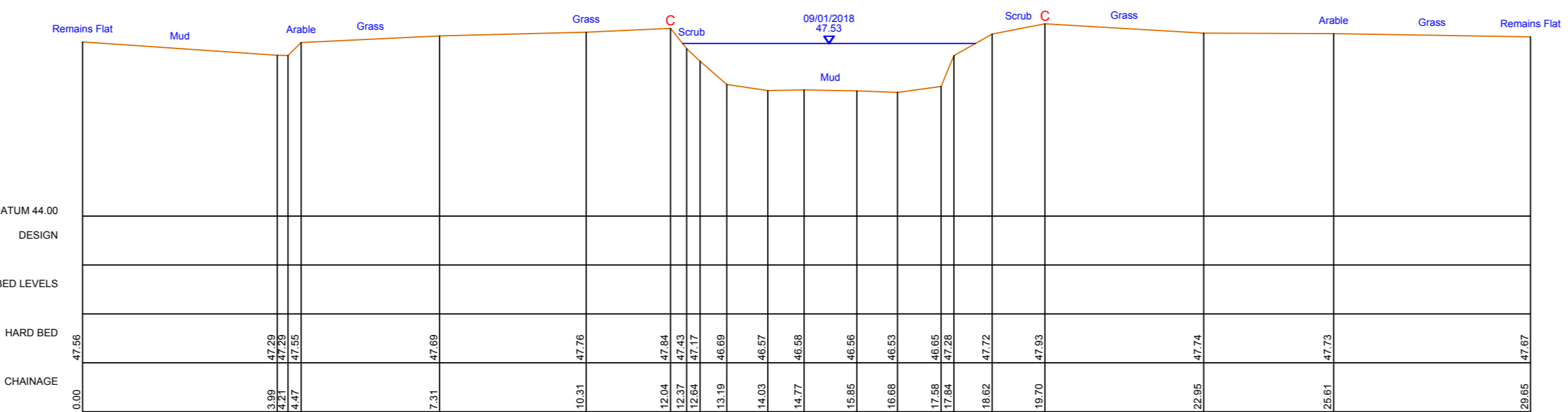
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606682.98mE 138121.79mN Brg 22
Culvert Entrance
Tunnel Length = 46.12m



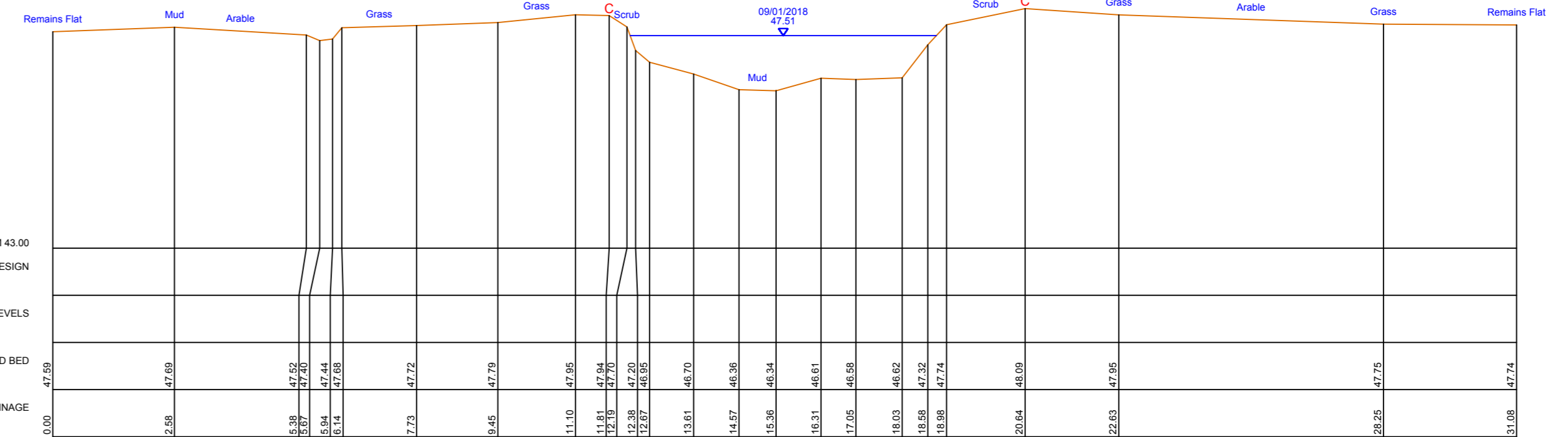
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Open Channel



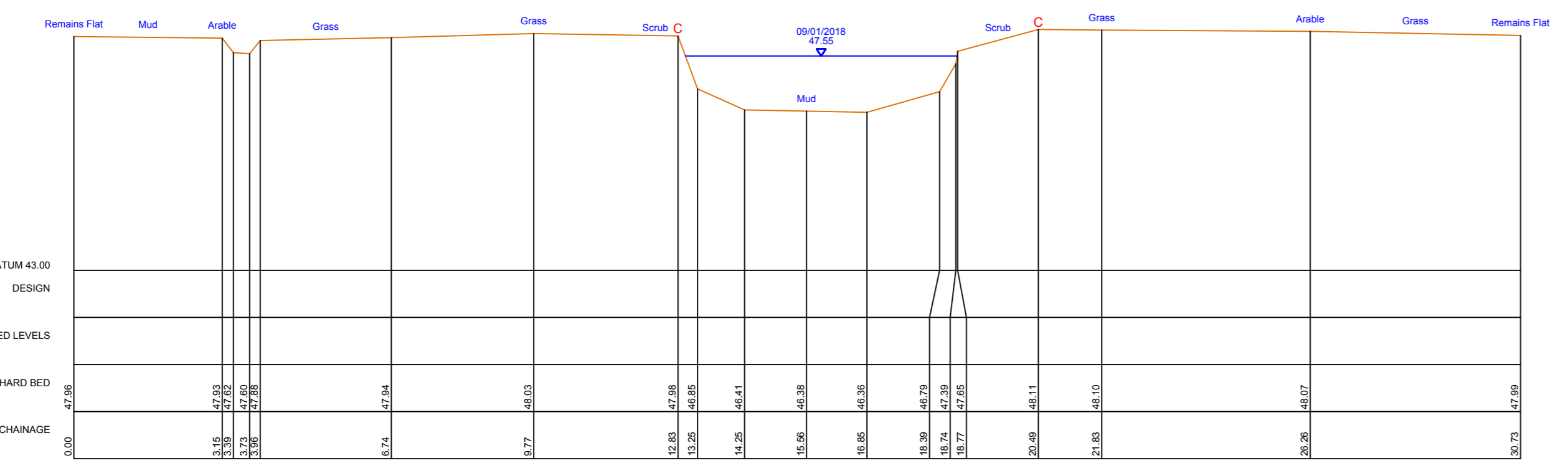
ESTO01_10723
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Access Bridge
Tunnel Length = 3.82m



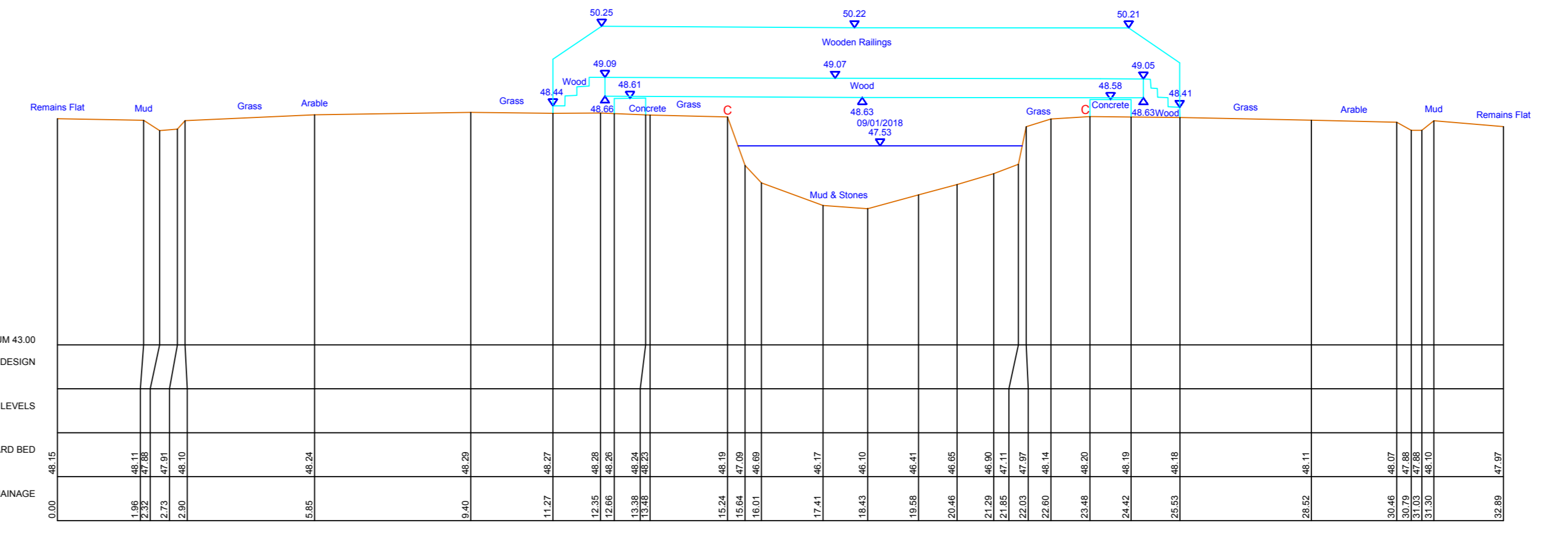
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Open Channel



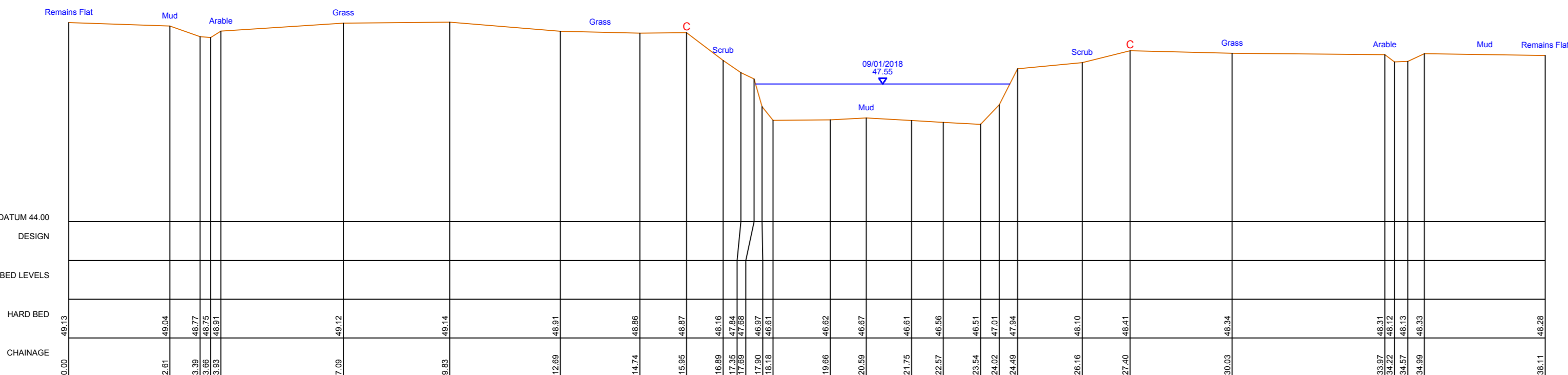
ESTO01_10924
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Open Channel



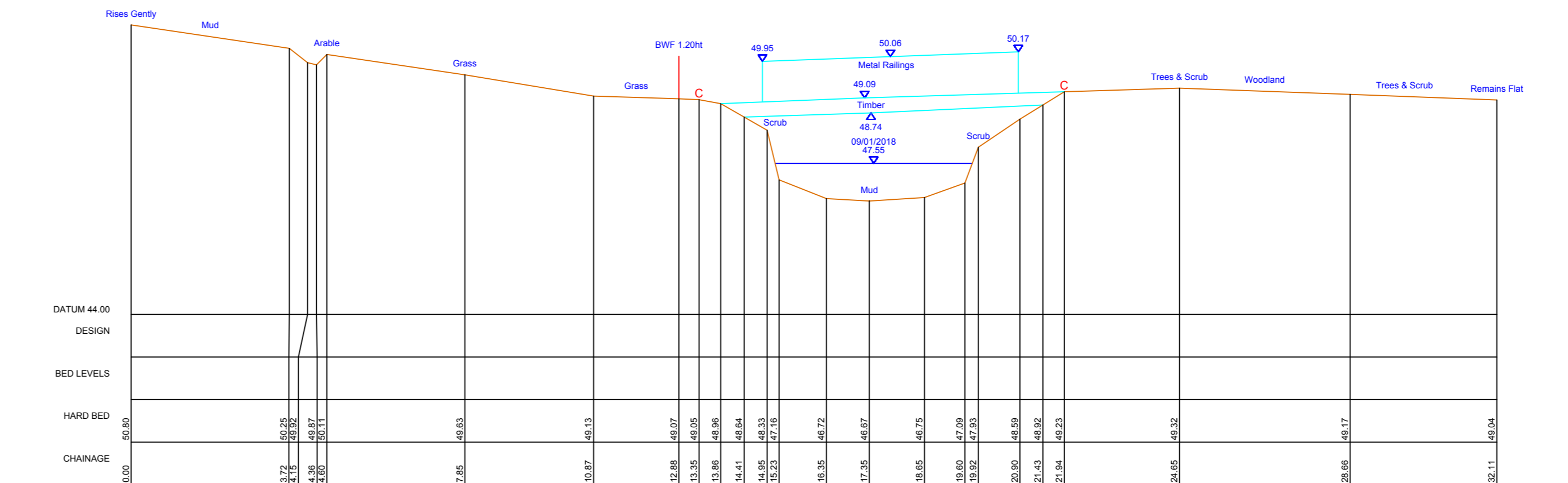
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Open Channel



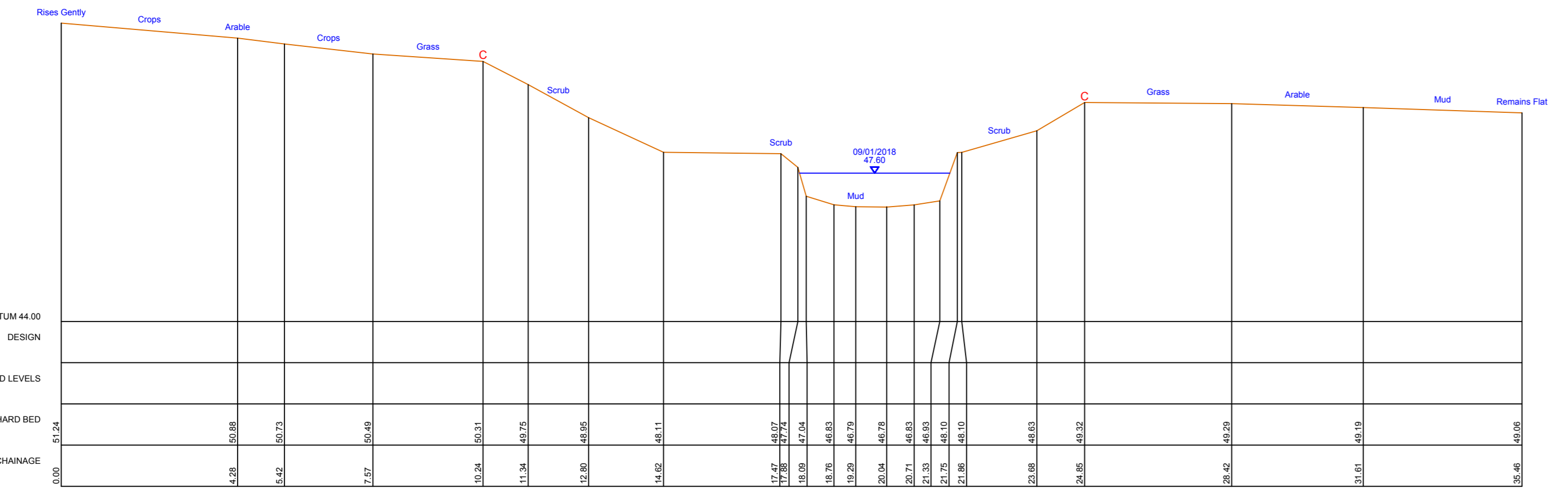
ESTO01_11157
607200.26mE 138153.29mN Brg 3
Footbridge
Tunnel Length = 1.14m



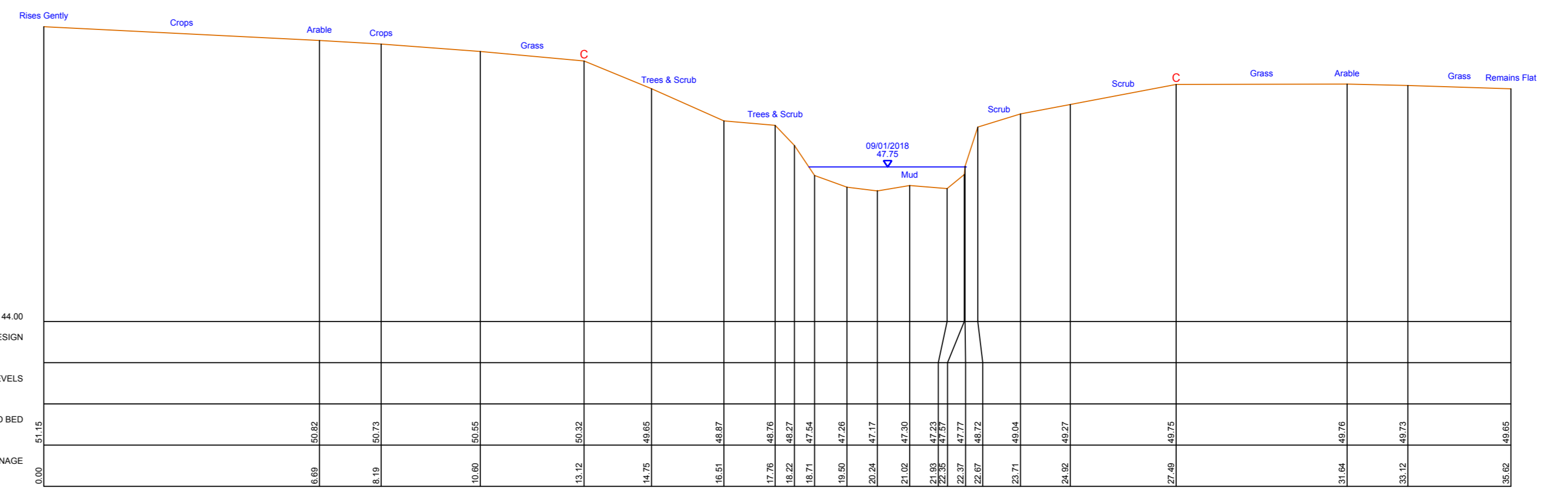
ESTO01_11228
607271.21mE 138151.68mN Brg 349
Open Channel



ESTO01_11320
607358.95mE 138158.66mN Brg 1
Footbridge
Tunnel Length = 0.69m



ESTO01_11425
607467.31mE 138161.04mN Brg 347
Open Channel



ESTO01_11532
607573.38mE 138186.09mN Brg 335
Open Channel

NOTES:

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SURVEY LEGEND

AS	AS BENCH	FW	THE CHANNEL	FW	THE CHANNEL	FW	THE CHANNEL
AW	AW BENCH	FW	THE CHANNEL	FW	THE CHANNEL	FW	THE CHANNEL
B	BENCH	FW	THE CHANNEL	FW	THE CHANNEL	FW	THE CHANNEL
BS	BS BENCH	FW	THE CHANNEL	FW	THE CHANNEL	FW	THE CHANNEL
C	C BENCH	FW	THE CHANNEL	FW	THE CHANNEL	FW	THE CHANNEL
...

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E0730012	TR 0103 4107	35.975
E0730013	TR 0103 4108	35.975
E0730014	TR 0103 4109	35.975
E0730015	TR 0103 4110	35.975
E0730016	TR 0103 4111	35.975
E0730017	TR 0103 4112	35.975
E0730018	TR 0103 4113	35.975
E0730019	TR 0103 4114	35.975
E0730020	TR 0103 4115	35.975
E0730021	TR 0103 4116	35.975
E0730022	TR 0103 4117	35.975
E0730023	TR 0103 4118	35.975
E0730024	TR 0103 4119	35.975
E0730025	TR 0103 4120	35.975
E0730026	TR 0103 4121	35.975
E0730027	TR 0103 4122	35.975
E0730028	TR 0103 4123	35.975
E0730029	TR 0103 4124	35.975
E0730030	TR 0103 4125	35.975
E0730031	TR 0103 4126	35.975
E0730032	TR 0103 4127	35.975
E0730033	TR 0103 4128	35.975
E0730034	TR 0103 4129	35.975
E0730035	TR 0103 4130	35.975
E0730036	TR 0103 4131	35.975
E0730037	TR 0103 4132	35.975
E0730038	TR 0103 4133	35.975
E0730039	TR 0103 4134	35.975
E0730040	TR 0103 4135	35.975
E0730041	TR 0103 4136	35.975
E0730042	TR 0103 4137	35.975
E0730043	TR 0103 4138	35.975
E0730044	TR 0103 4139	35.975
E0730045	TR 0103 4140	35.975
E0730046	TR 0103 4141	35.975
E0730047	TR 0103 4142	35.975
E0730048	TR 0103 4143	35.975
E0730049	TR 0103 4144	35.975
E0730050	TR 0103 4145	35.975
E0730051	TR 0103 4146	35.975
E0730052	TR 0103 4147	35.975
E0730053	TR 0103 4148	35.975
E0730054	TR 0103 4149	35.975
E0730055	TR 0103 4150	35.975
E0730056	TR 0103 4151	35.975
E0730057	TR 0103 4152	35.975
E0730058	TR 0103 4153	35.975
E0730059	TR 0103 4154	35.975
E0730060	TR 0103 4155	35.975

Environment Agency
KENT & SOUTH LONDON REGION
Orford House, Edenour Park, London Road, Ashford, Kent, ME19 5QH

PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS:
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_10528 TO ESTO01_11532

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref: 12_157*

SURVEY DATE: DECEMBER 2017 – MARCH 2018

SCALE: 1:100 **DRN:** RC **CHKD:** ITS

DATUM: OS GPS ACTIVE **DATE:** MAR 18 **DATE:** MAR 18

GRID: NATIONAL GRID **DRAWING NO.:** **REV.:** **REV.:** **REV.:**

CDW FILENAME: A-2018-01-30.dwg **DRAWING NO.:** X-J01058-18

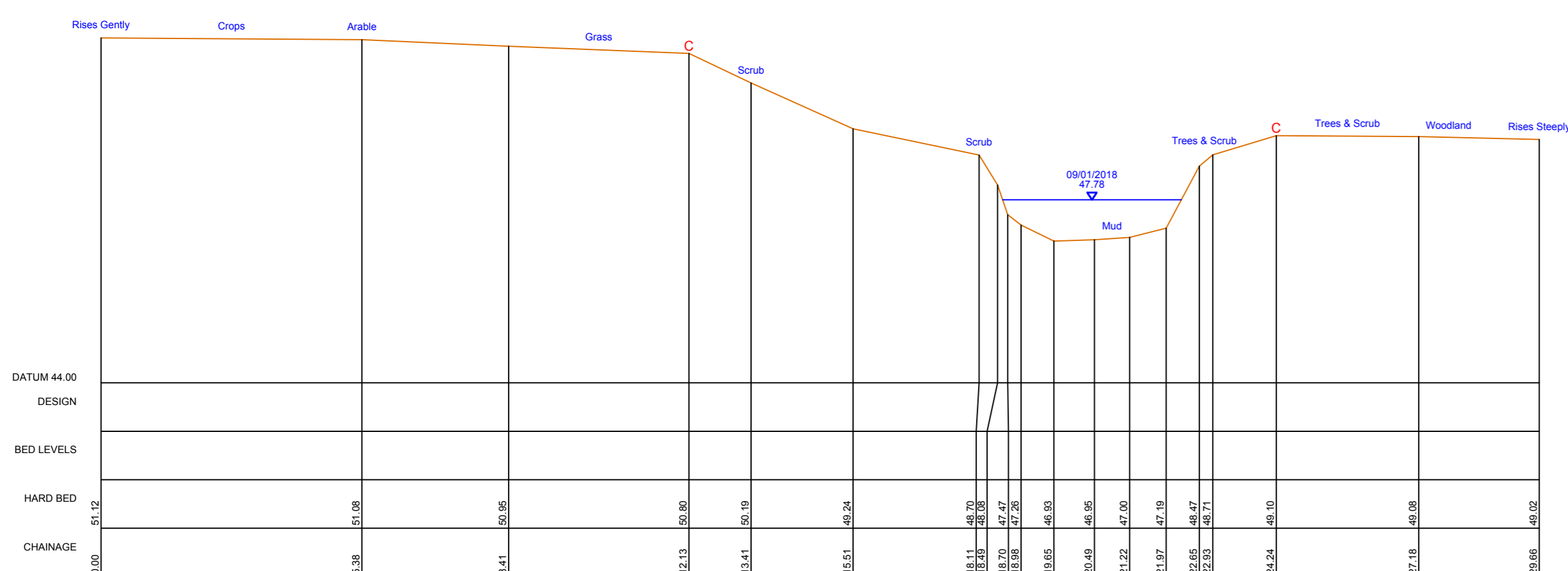
KEY TO SECTIONS:

- Water Level
- Visible Bed (Top of Silt) and Ground
- Hard Bed (Determined by Probing)
- C Bank Crest

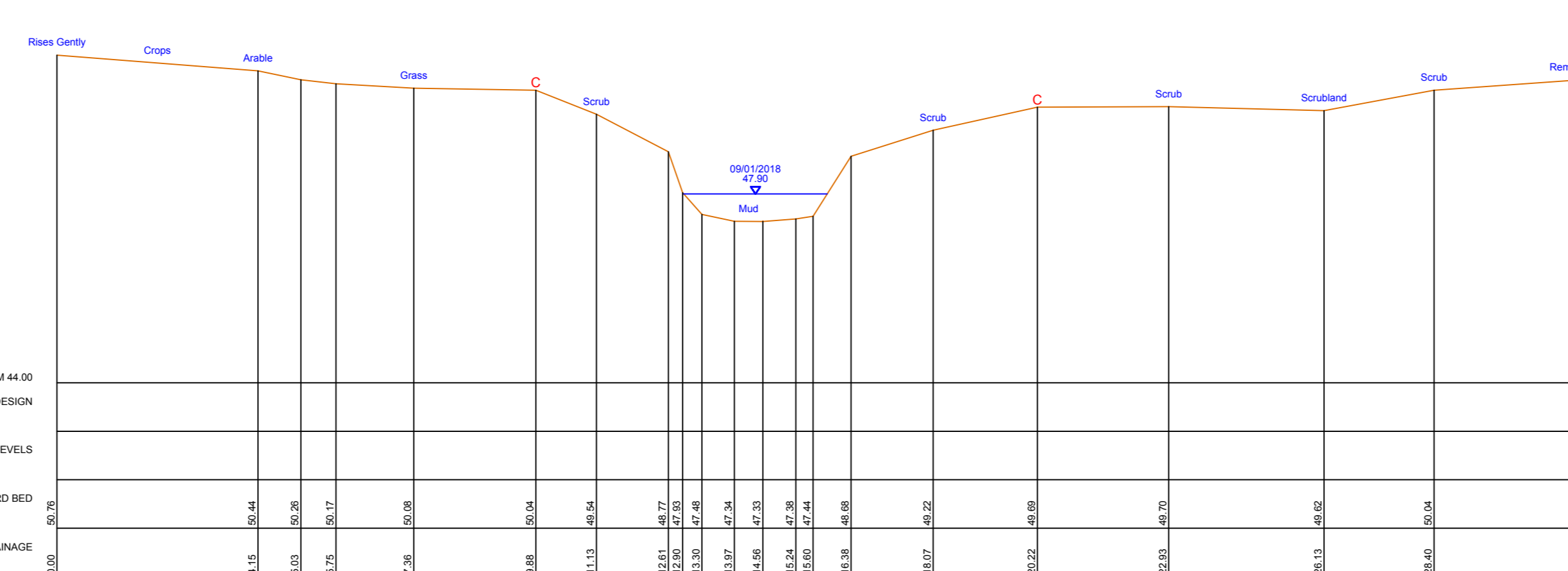
KEY TO LONGITUDINAL SECTION ONLY:

- Viewed Looking Downstream
- Left Bank Crest
- Right Bank Crest

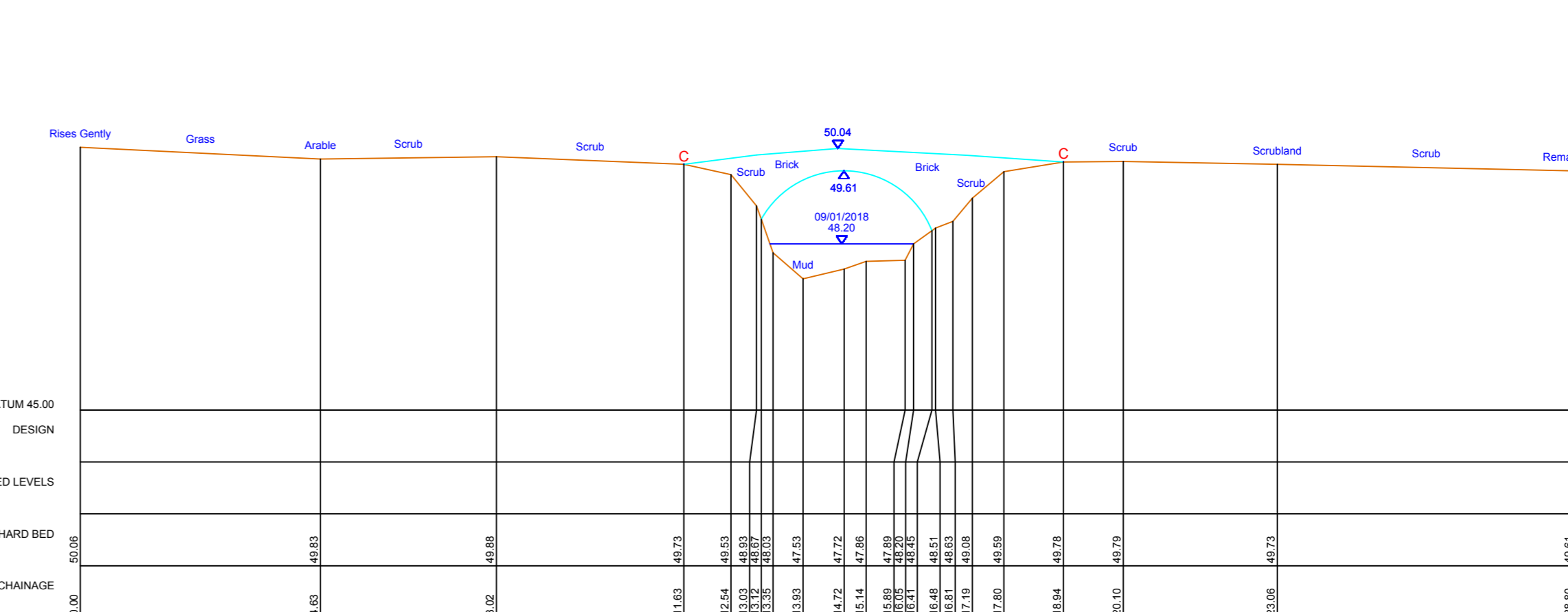
POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



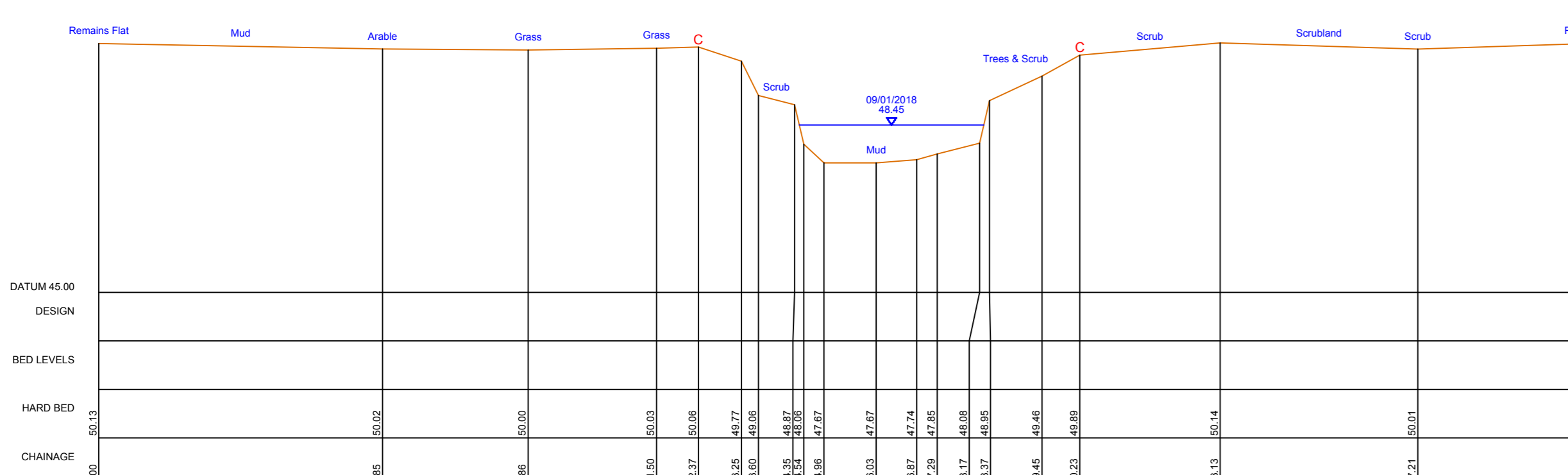
ESTO01_11624
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Open Channel



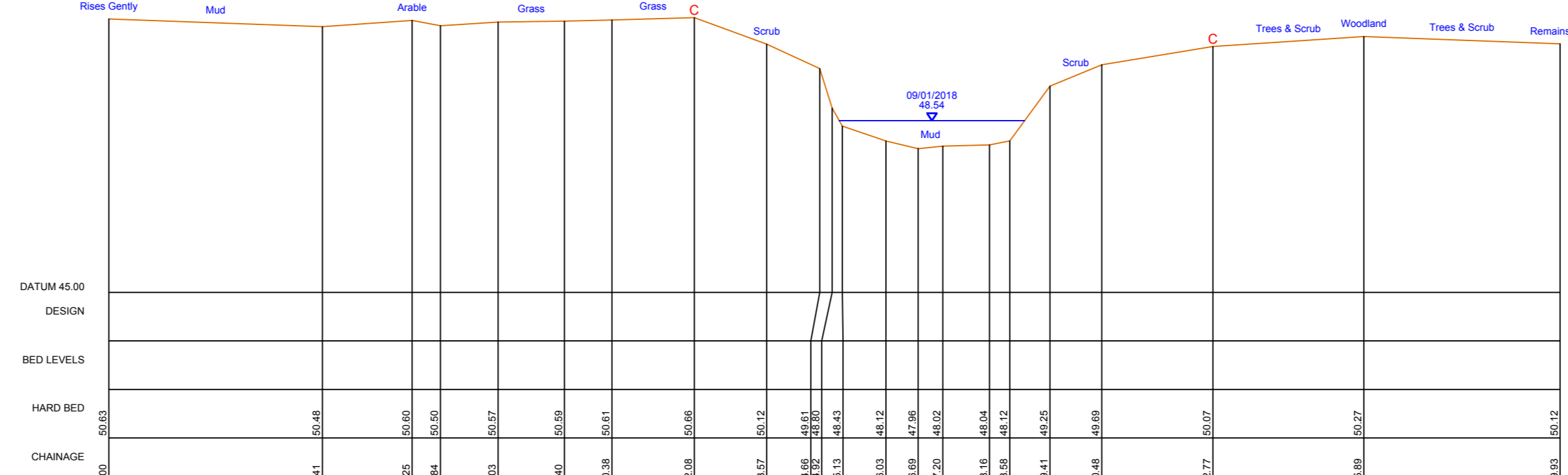
ESTO01_11734
607728.02mE 138126.74mN Brg 13
Open Channel



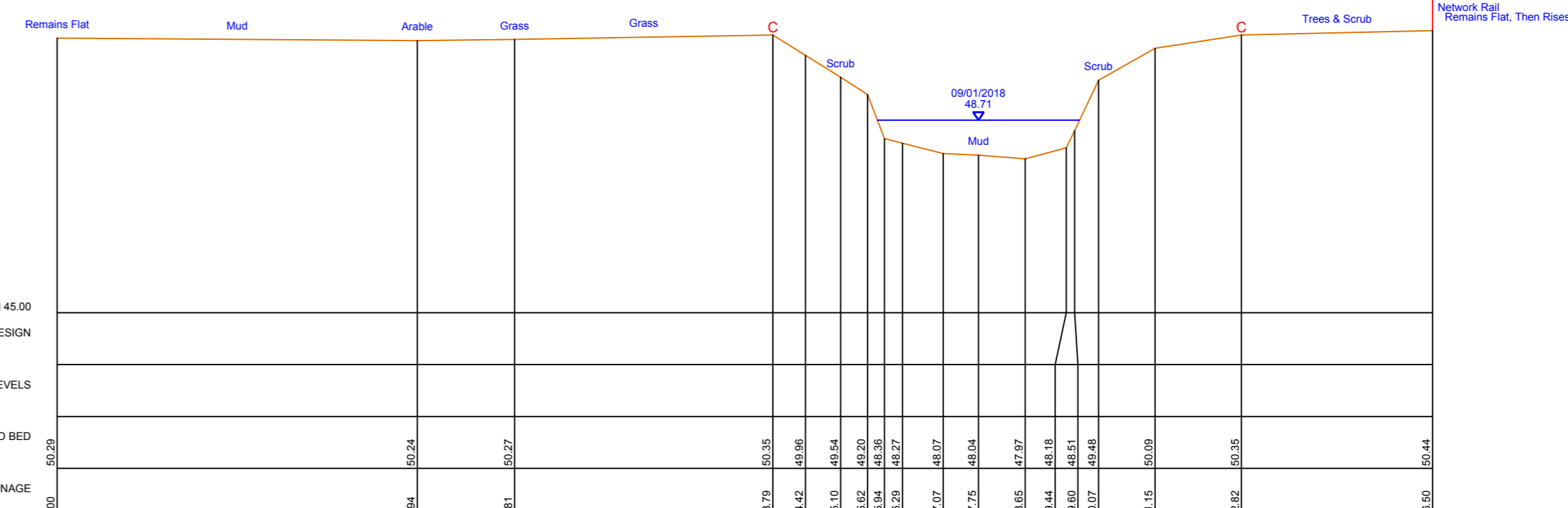
ESTO01_11808
607799.87mE 138110.6mN Brg 8
Access Bridge
Tunnel Length = 3.38m



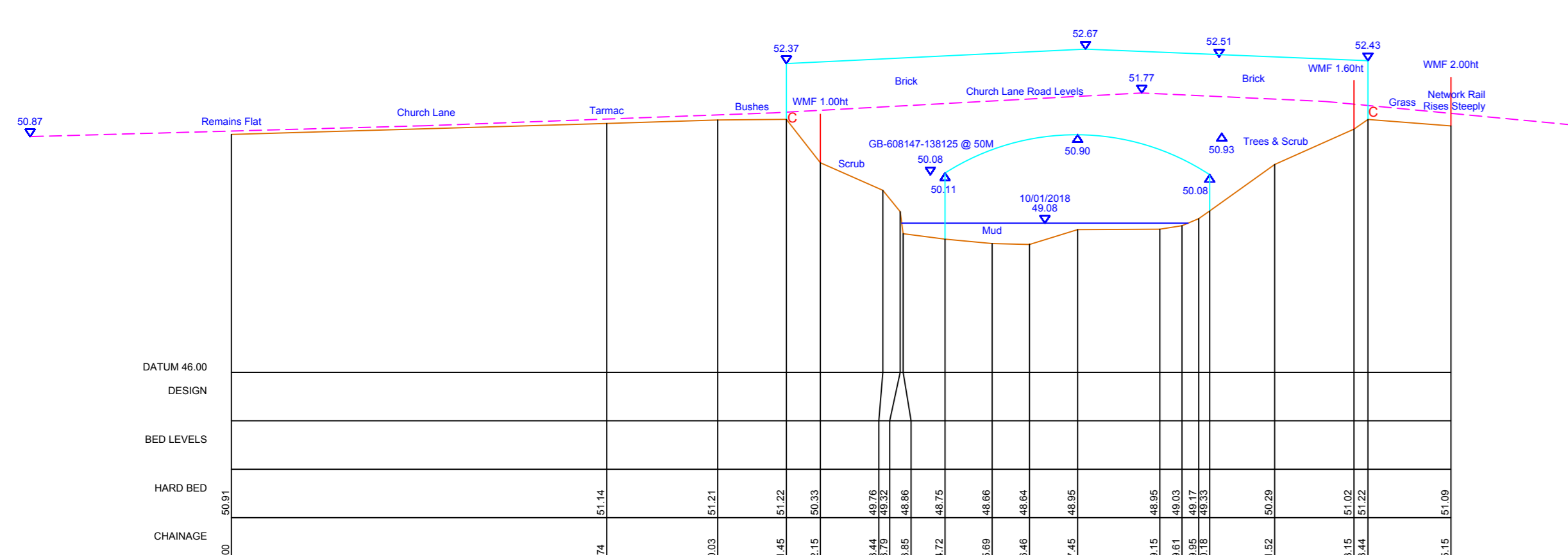
ESTO01_11923
607906.14mE 138103.68mN Brg 353
Open Channel



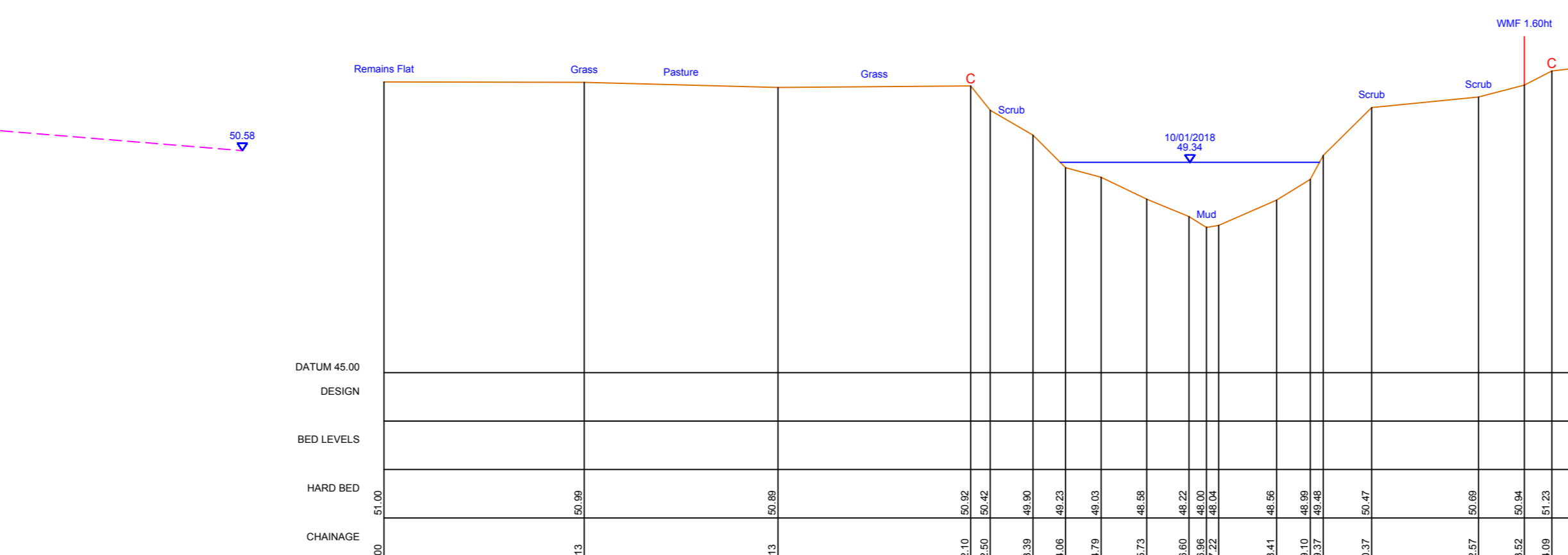
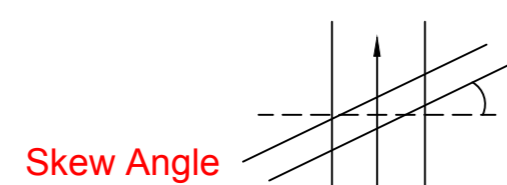
ESTO01_12029
607994.38mE 138116.03mN Brg 43
Open Channel



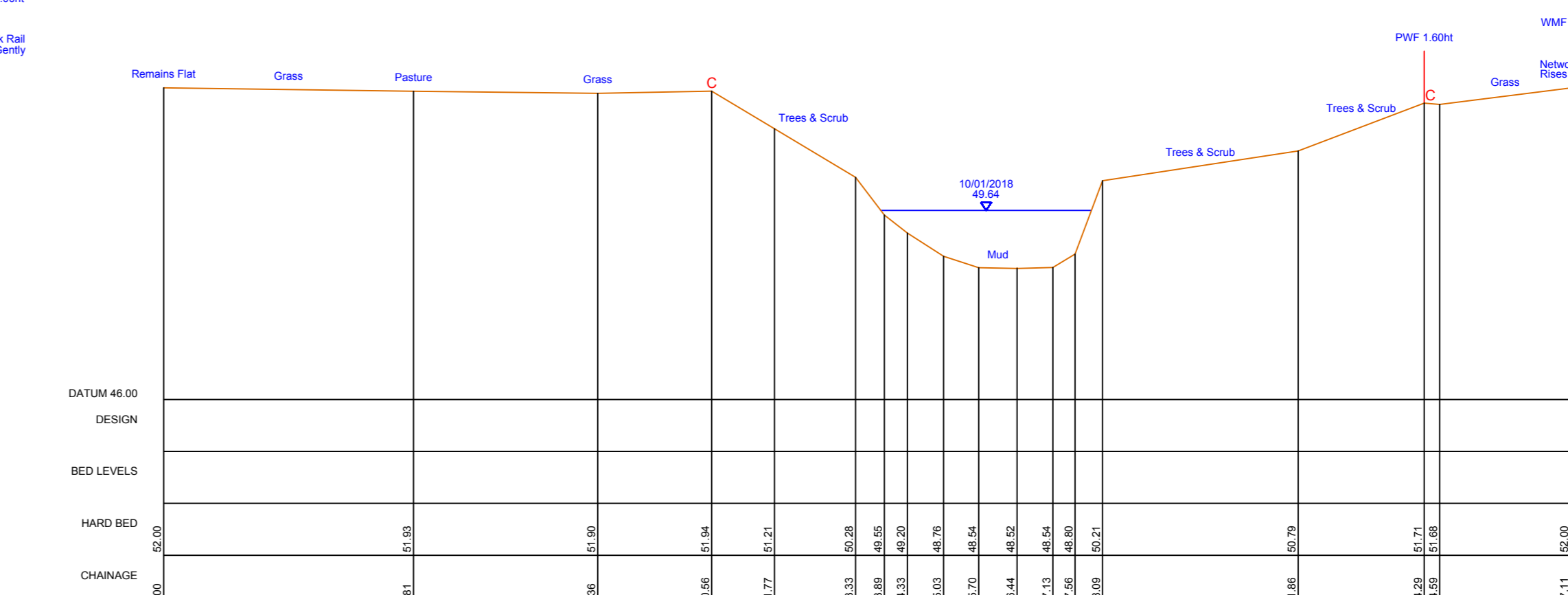
ESTO01_12130
608100.21mE 138110.23mN Brg 7
Open Channel



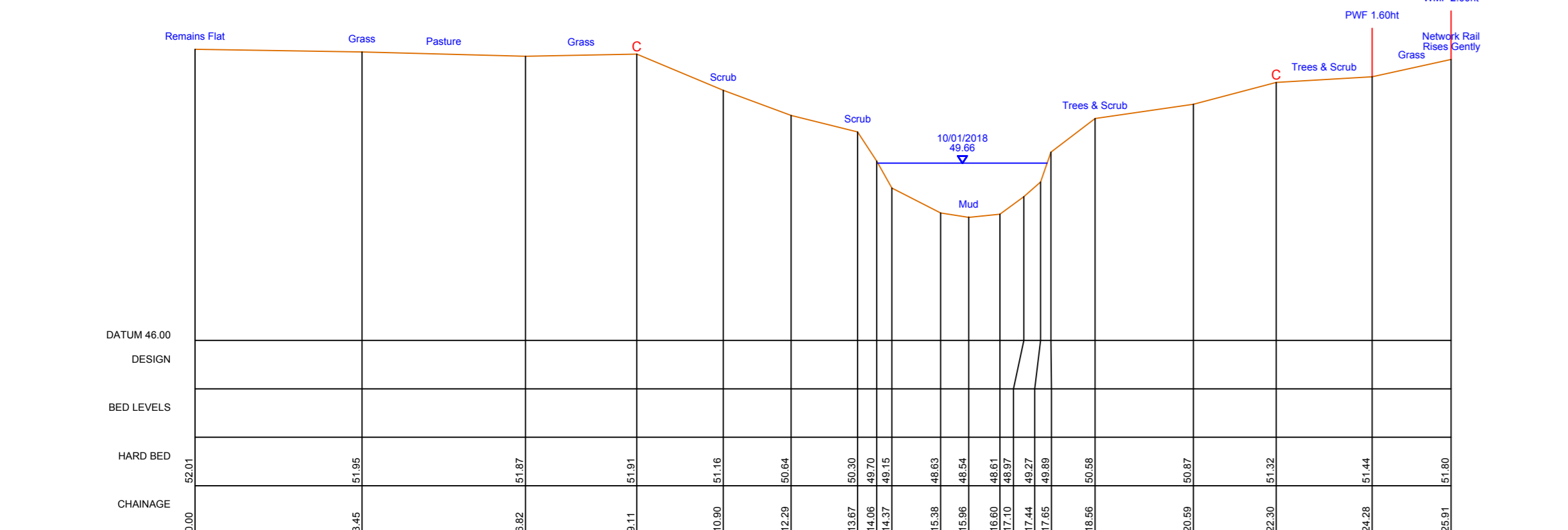
ESTO01_12172
608151.79mE 138111.42mN Brg 339
Church Lane Road Bridge
Tunnel Length = 5.78m
Skew Angle = 25°



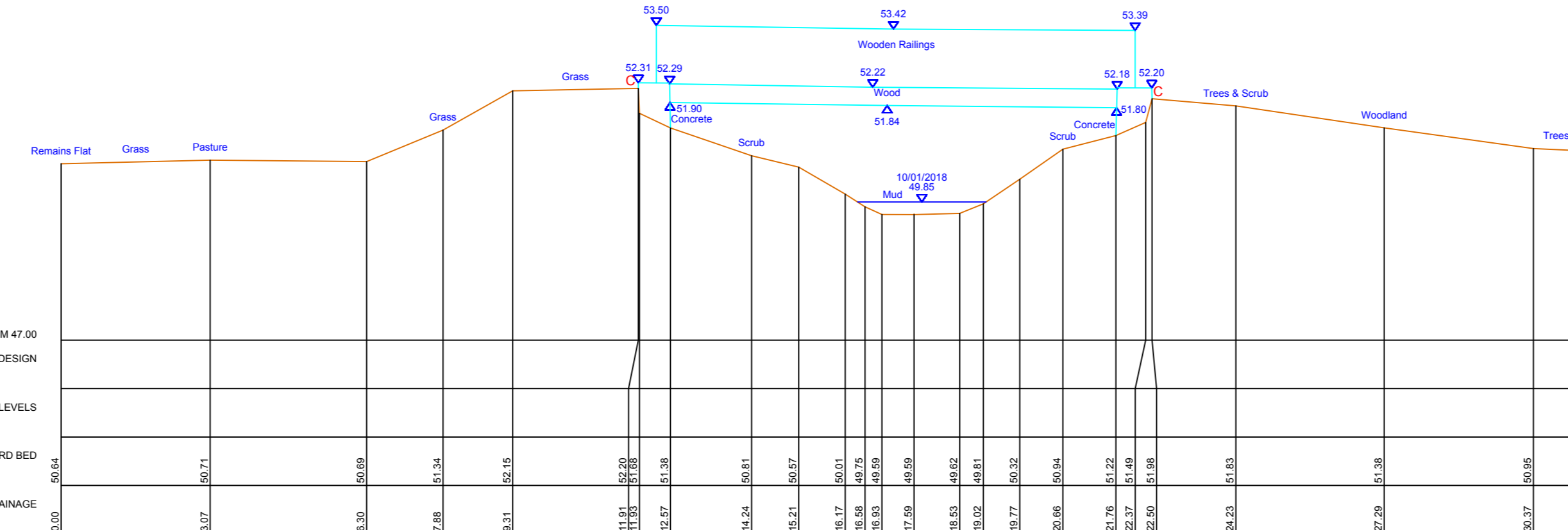
ESTO01_12232
608199.43mE 138099.36mN Brg 16
Open Channel



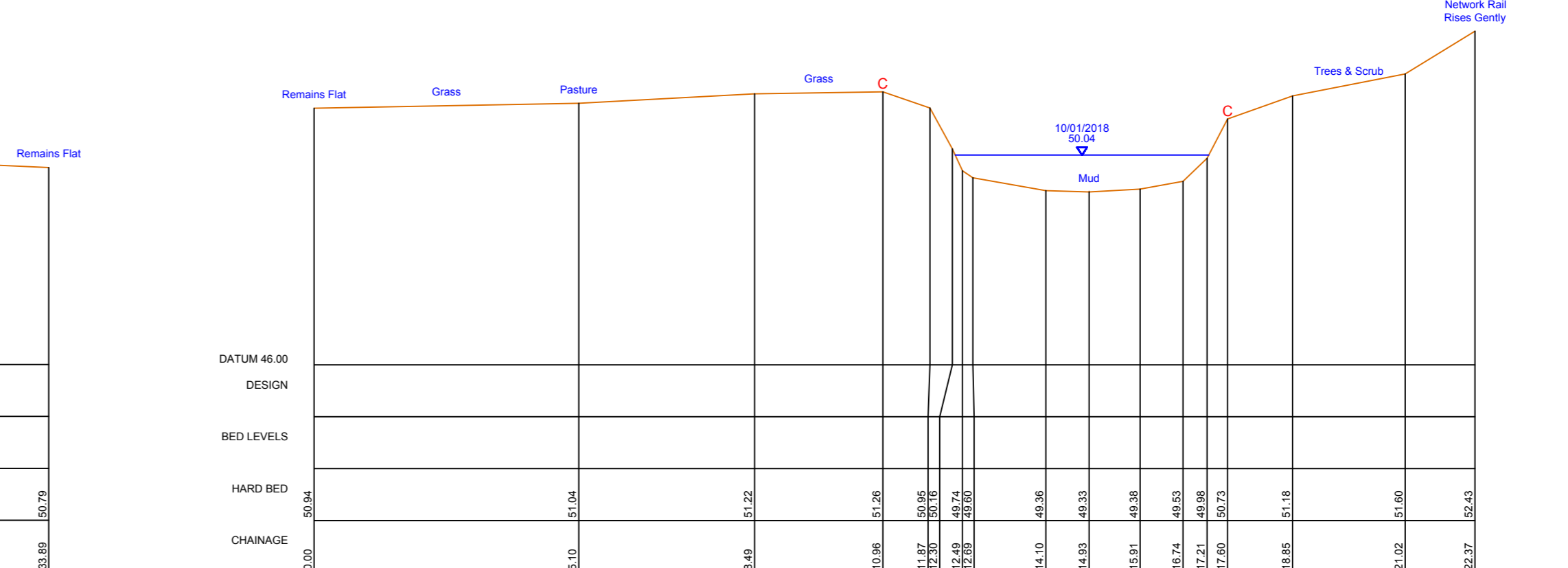
ESTO01_12333
608299.56mE 138080.43mN Brg 10
Open Channel



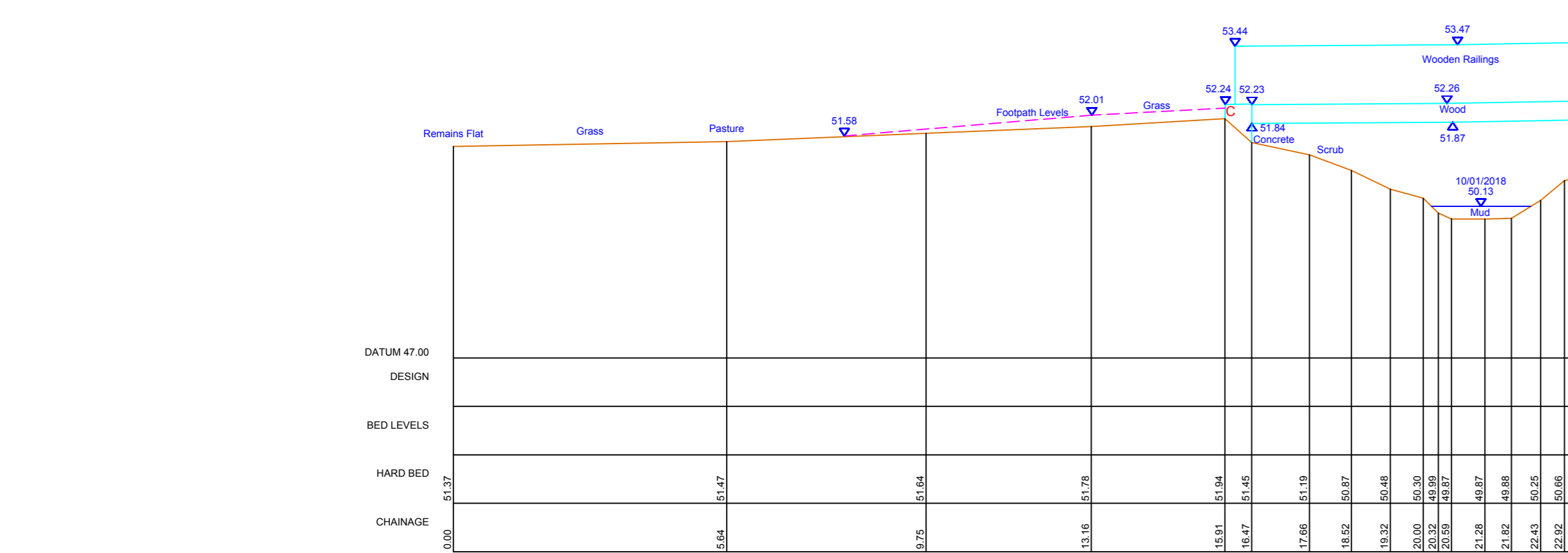
ESTO01_12435
608402.12mE 138062.55mN Brg 3
Open Channel



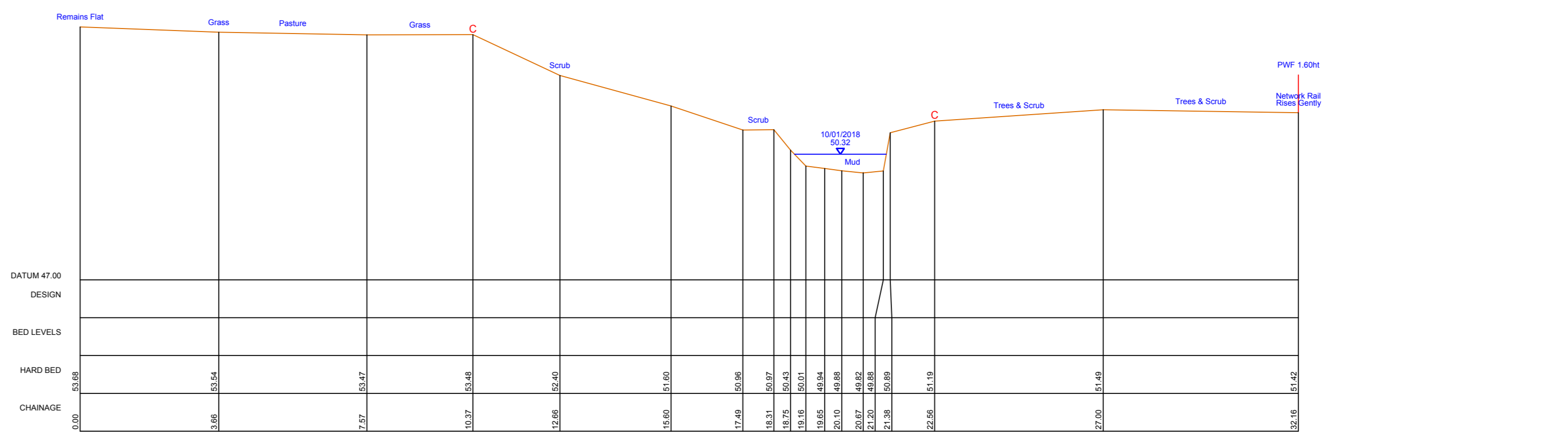
ESTO01_12552
608504.16mE 138034.25mN Brg 335
Footbridge
Tunnel Length = 1.84m



ESTO01_12606
608546.17mE 138033.58mN Brg 6
Open Channel



ESTO01_12641
608550.36mE 138025.77mN Brg 80
Footbridge
Tunnel Length = 1.78m



ESTO01_12722
608647.41mE 138003.44mN Brg 2
Open Channel

NOTES:
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SURVEY LEGEND		
AS	AS BENCH	AS POINT
BS	BENCH MARK	BS POINT
CH	CHANNEL	CH POINT
CL	CL	CL POINT
CP	CHANNEL POINT	CP POINT
CR	CROSSING	CR POINT
CS	CROSS SECTION	CS POINT
CSA	CROSS SECTION ADDITIONAL	CSA POINT
CSB	CROSS SECTION BOUNDARY	CSB POINT
CSL	CROSS SECTION LINE	CSL POINT
CSM	CROSS SECTION MARK	CSM POINT
CSN	CROSS SECTION NAME	CSN POINT
CSO	CROSS SECTION OBJECT	CSO POINT
CSR	CROSS SECTION REFERENCE	CSR POINT
CSU	CROSS SECTION UNIT	CSU POINT
CSV	CROSS SECTION VALUE	CSV POINT
CSW	CROSS SECTION WIDTH	CSW POINT
CSX	CROSS SECTION EXTENSION	CSX POINT
CSY	CROSS SECTION YAW	CSY POINT
CSZ	CROSS SECTION ZONE	CSZ POINT
CSAA	CROSS SECTION AREA	CSAA POINT
CSAB	CROSS SECTION AREA BOUNDARY	CSAB POINT
CSAC	CROSS SECTION AREA CENTER	CSAC POINT
CSAD	CROSS SECTION AREA DIRECTION	CSAD POINT
CSAE	CROSS SECTION AREA EXTENSION	CSAE POINT
CSAF	CROSS SECTION AREA FLOW	CSAF POINT
CSAG	CROSS SECTION AREA GRADE	CSAG POINT
CSAH	CROSS SECTION AREA HEIGHT	CSAH POINT
CSAI	CROSS SECTION AREA IDENTIFICATION	CSAI POINT
CSAJ	CROSS SECTION AREA JUNCTION	CSAJ POINT
CSAK	CROSS SECTION AREA KIND	CSAK POINT
CSAL	CROSS SECTION AREA LENGTH	CSAL POINT
CSAM	CROSS SECTION AREA MATERIAL	CSAM POINT
CSAN	CROSS SECTION AREA NAME	CSAN POINT
CSAO	CROSS SECTION AREA OBJECT	CSAO POINT
CSAP	CROSS SECTION AREA POSITION	CSAP POINT
CSAQ	CROSS SECTION AREA QUANTITY	CSAQ POINT
CSAR	CROSS SECTION AREA REFERENCE	CSAR POINT
CSAS	CROSS SECTION AREA SCALE	CSAS POINT
CSAT	CROSS SECTION AREA TOLERANCE	CSAT POINT
CSAU	CROSS SECTION AREA UNIT	CSAU POINT
CSAV	CROSS SECTION AREA VALUE	CSAV POINT
CSAW	CROSS SECTION AREA WIDTH	CSAW POINT
CSAX	CROSS SECTION AREA EXTENSION	CSAX POINT
CSAY	CROSS SECTION AREA YAW	CSAY POINT
CSAZ	CROSS SECTION AREA ZONE	CSAZ POINT
CSAA	CROSS SECTION AREA AREA	CSAA POINT
CSAB	CROSS SECTION AREA AREA BOUNDARY	CSAB POINT
CSAC	CROSS SECTION AREA AREA CENTER	CSAC POINT
CSAD	CROSS SECTION AREA AREA DIRECTION	CSAD POINT
CSAE	CROSS SECTION AREA AREA EXTENSION	CSAE POINT
CSAF	CROSS SECTION AREA AREA FLOW	CSAF POINT
CSAG	CROSS SECTION AREA AREA GRADE	CSAG POINT
CSAH	CROSS SECTION AREA AREA HEIGHT	CSAH POINT
CSAI	CROSS SECTION AREA AREA IDENTIFICATION	CSAI POINT
CSAJ	CROSS SECTION AREA AREA JUNCTION	CSAJ POINT
CSAK	CROSS SECTION AREA AREA KIND	CSAK POINT
CSAL	CROSS SECTION AREA AREA LENGTH	CSAL POINT
CSAM	CROSS SECTION AREA AREA MATERIAL	CSAM POINT
CSAN	CROSS SECTION AREA AREA NAME	CSAN POINT
CSAO	CROSS SECTION AREA AREA OBJECT	CSAO POINT
CSAP	CROSS SECTION AREA AREA POSITION	CSAP POINT
CSAQ	CROSS SECTION AREA AREA QUANTITY	CSAQ POINT
CSAR	CROSS SECTION AREA AREA REFERENCE	CSAR POINT
CSAS	CROSS SECTION AREA AREA SCALE	CSAS POINT
CSAT	CROSS SECTION AREA AREA TOLERANCE	CSAT POINT
CSAU	CROSS SECTION AREA AREA UNIT	CSAU POINT
CSAV	CROSS SECTION AREA AREA VALUE	CSAV POINT
CSAW	CROSS SECTION AREA AREA WIDTH	CSAW POINT
CSAX	CROSS SECTION AREA AREA EXTENSION	CSAX POINT
CSAY	CROSS SECTION AREA AREA YAW	CSAY POINT
CSAZ	CROSS SECTION AREA AREA ZONE	CSAZ POINT

AMENDMENT	DRN	CHD	DATE

CONTROL USED:	DESCRIPTION	LEVEL
E0720012	TR 0103 4107	35.975
E0720013	TR 0229 4227	36.280
E0720014	TR 0109 4202	36.480
E0720015	TR 0109 4202	36.480
E0720016	TR 0109 4202	36.480
E0720017	TR 0109 4202	36.480
E0720018	TR 0109 4202	36.480
E0720019	TR 0109 4202	36.480
E0720020	TR 0109 4202	36.480
E0720021	TR 0109 4202	36.480
E0720022	TR 0109 4202	36.480
E0720023	TR 0109 4202	36.480
E0720024	TR 0109 4202	36.480
E0720025	TR 0109 4202	36.480
E0720026	TR 0109 4202	36.480
E0720027	TR 0109 4202	36.480
E0720028	TR 0109 4202	36.480
E0720029	TR 0109 4202	36.480
E0720030	TR 0109 4202	36.480
E0720031	TR 0109 4202	36.480
E0720032	TR 0109 4202	36.480
E0720033	TR 0109 4202	36.480
E0720034	TR 0109 4202	36.480
E0720035	TR 0109 4202	36.480
E0720036	TR 0109 4202	36.480
E0720037	TR 0109 4202	36.480
E0720038	TR 0109 4202	36.480
E0720039	TR 0109 4202	36.480
E0720040	TR 0109 4202	36.480
E0720041	TR 0109 4202	36.480
E0720042	TR 0109 4202	36.480
E0720043	TR 0109 4202	36.480
E0720044	TR 0109 4202	36.480
E0720045	TR 0109 4202	36.480
E0720046	TR 0109 4202	36.480
E0720047	TR 0109 4202	36.480
E0720048	TR 0109 4202	36.480
E0720049	TR 0109 4202	36.480
E0720050	TR 0109 4202	36.480

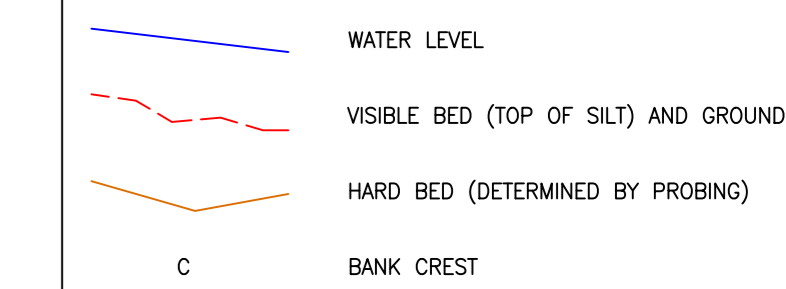
Environment Agency
KENT & SOUTH LONDON REGION
Orford House, Edenmore Park, London Road, Ashford, Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

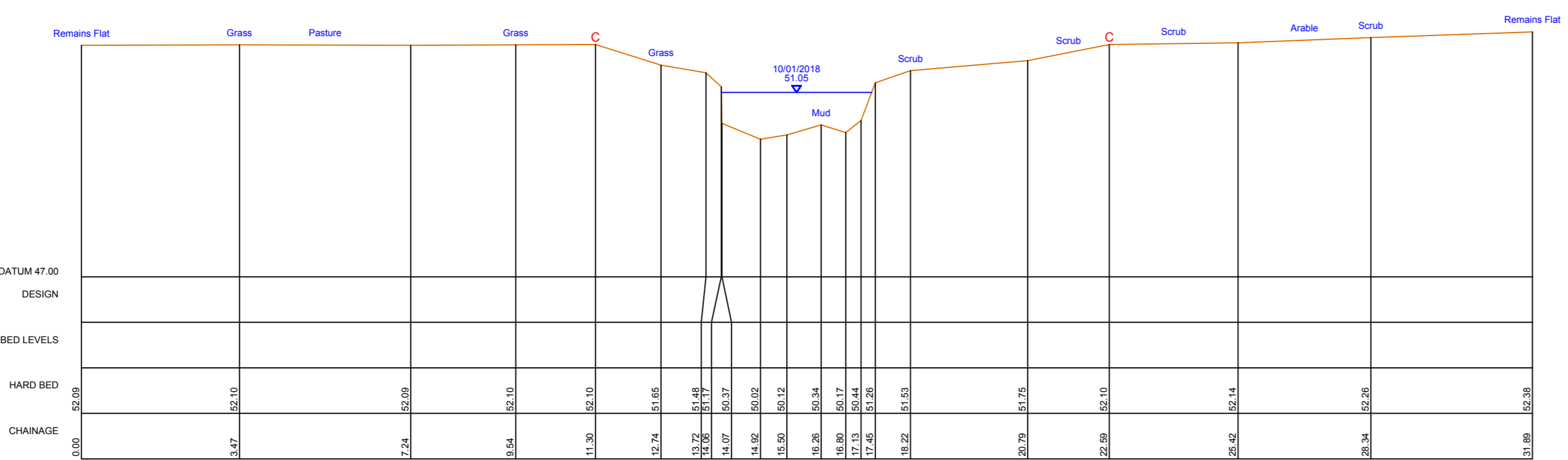
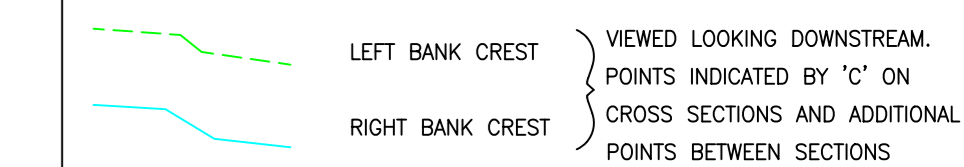
SITE/UMTS
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_11624 TO ESTO01_12722

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref: 12_152*
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100 DRN: RC CHKD: ITS
DATUM: OS GPS ACTIVE DATE: MAR 18 DATE: MAR 18
GRID: NATIONAL GRID DRAWING NO. REV. NO.
DWG FILENAME: F-2018-01-30.dwg X-J01058-19

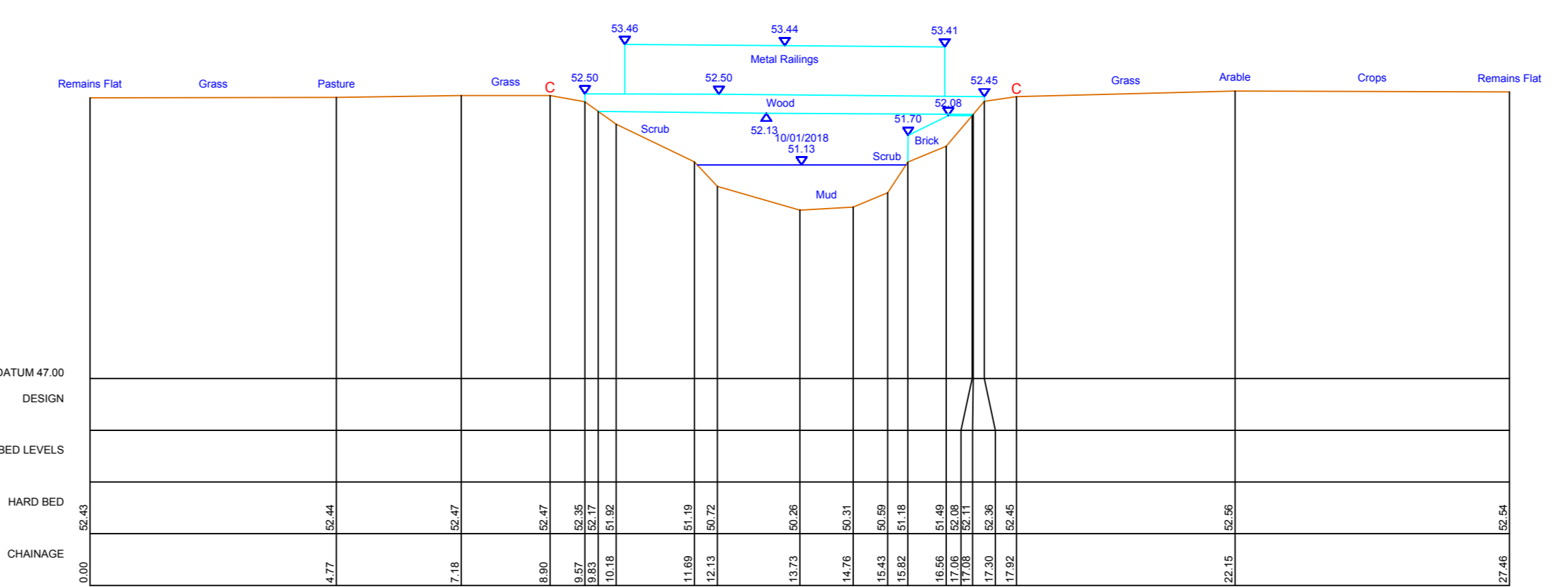
KEY TO SECTIONS:



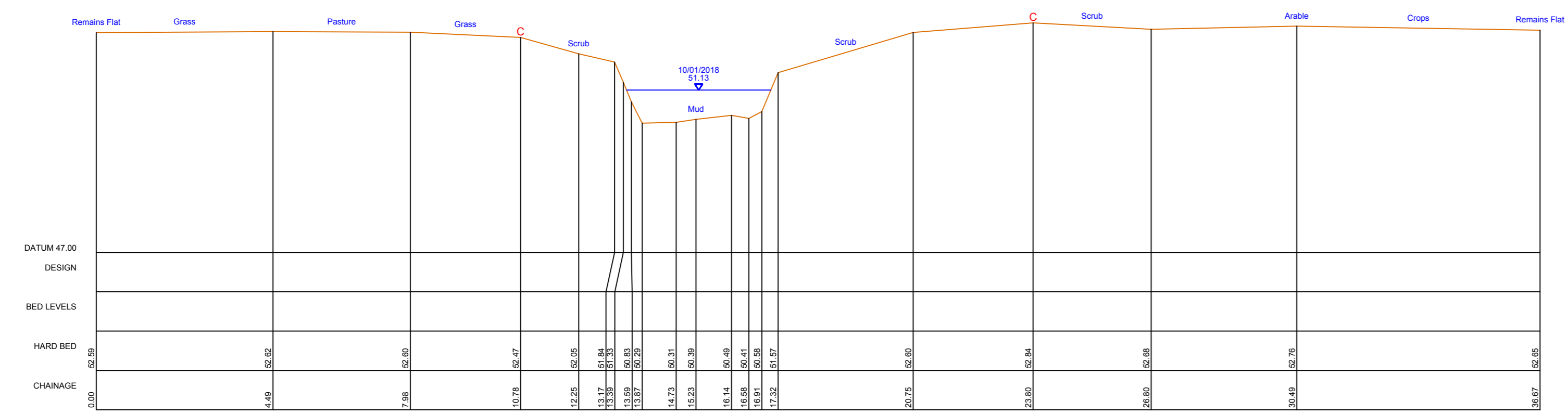
KEY TO LONGITUDINAL SECTION ONLY:



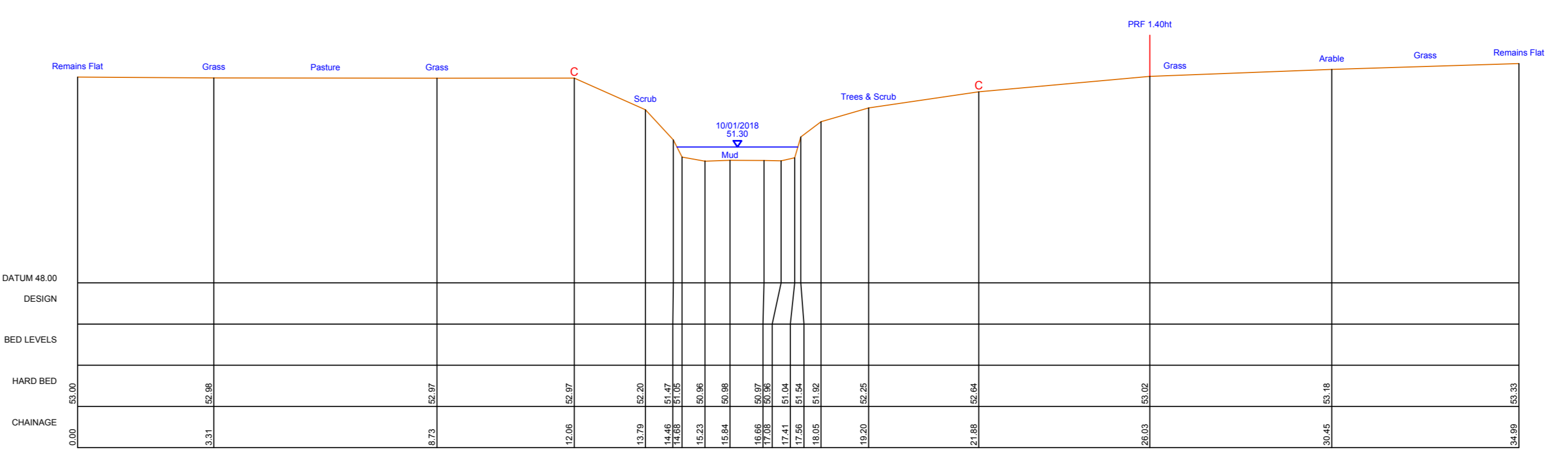
EST001_12839
608746.93mE 137997.83mN Brg 53
Open Channel



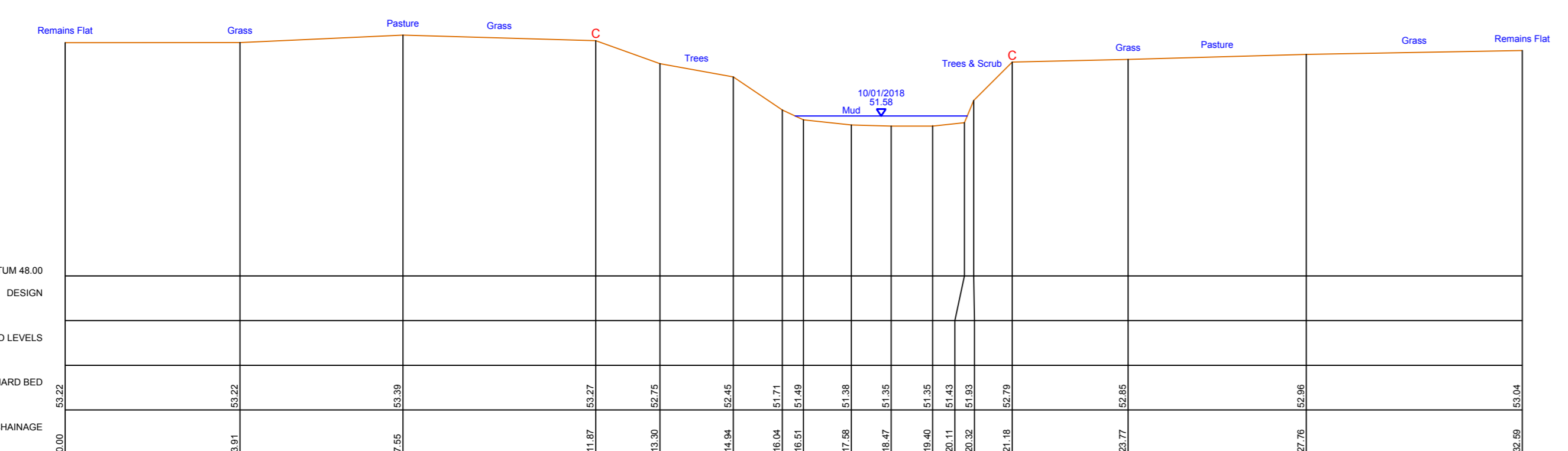
EST001_12946
608809.06mE 137913.52mN Brg 33
Footbridge
Tunnel Length = 0.81m



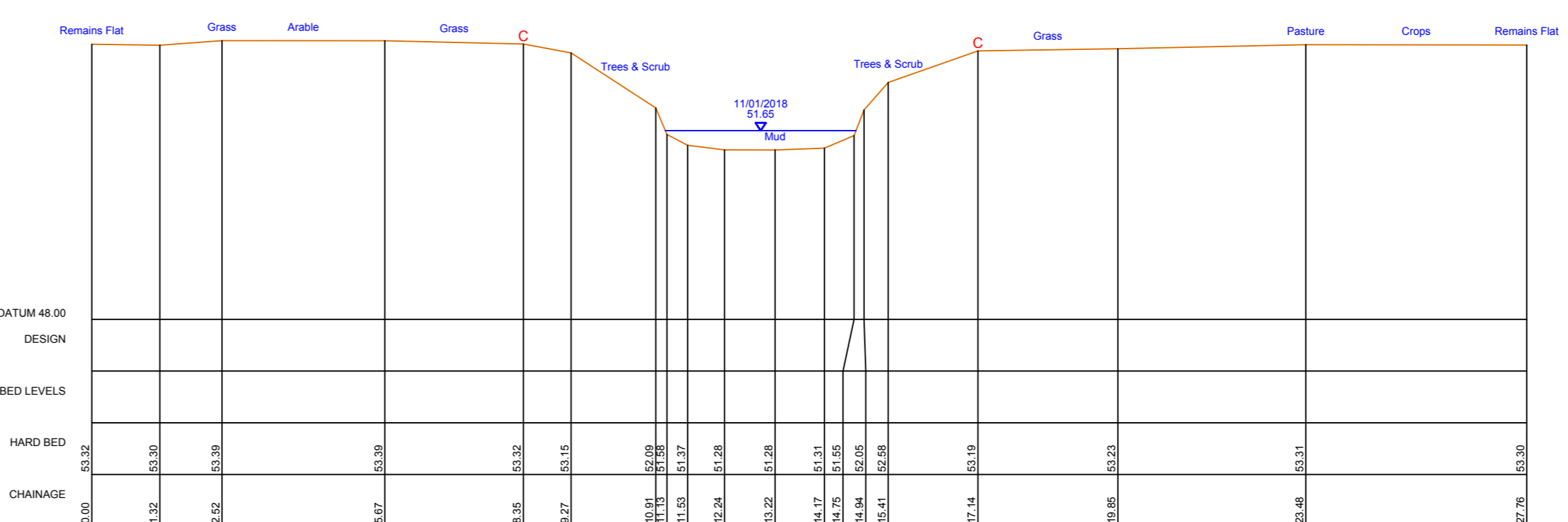
EST001_13012
608943.53mE 137863.14mN Brg 16
Open Channel



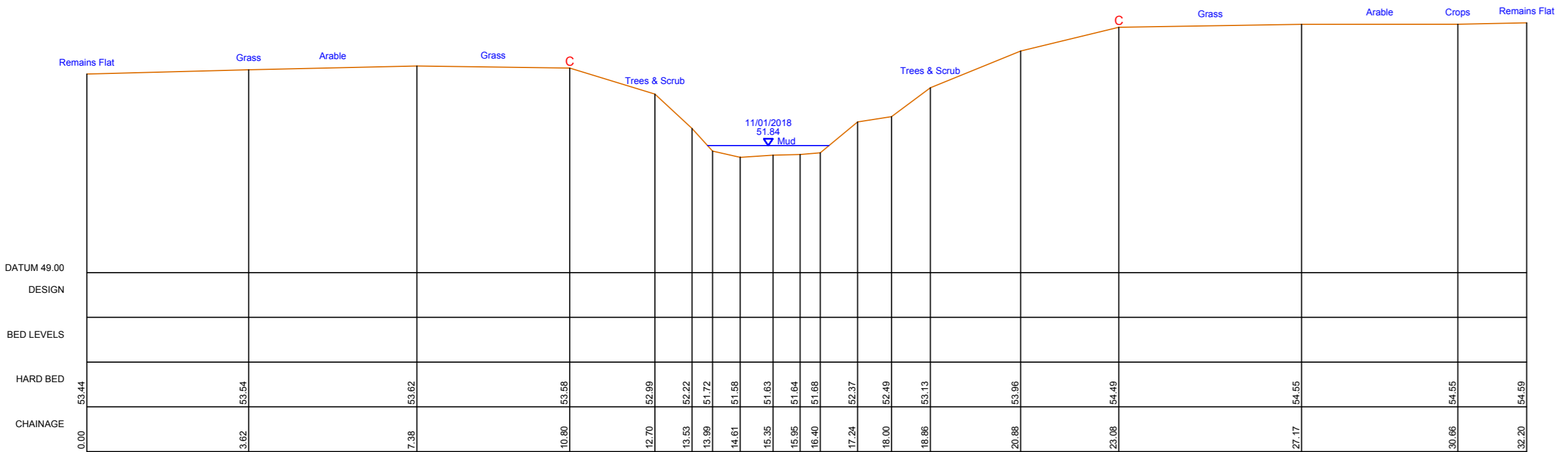
EST001_13123
608897.63mE 137789.22mN Brg 26
Open Channel



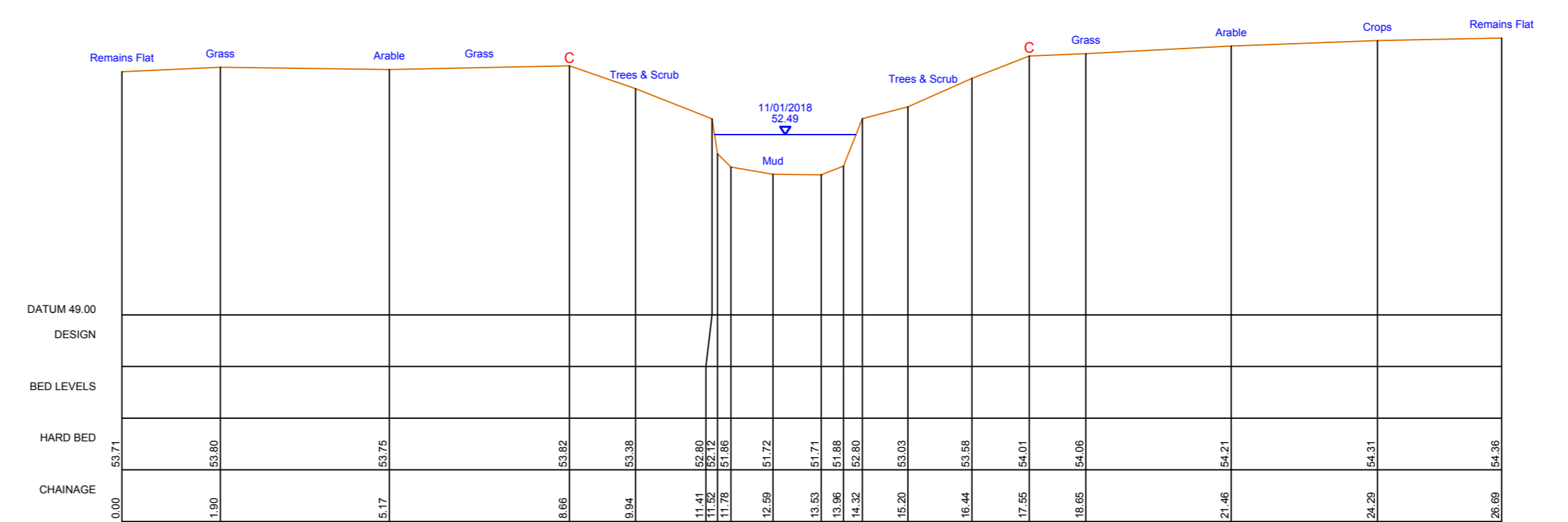
EST001_13264
608941.92mE 137753.79mN Brg 89
Open Channel



EST001_13327
609003.38mE 137727.55mN Brg 318
Open Channel



EST001_13421
609070.6mE 137757.72mN Brg 6
Open Channel



EST001_13509
609152.74mE 137753.04mN Brg 30
Open Channel

NOTES:
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SURVEY LEGEND

AB	AS BENCH	FW	WATER LEVEL	PW	POINT WELL
BP	BANK PIN	FM	FLOOD MARK	RE	REFL POINT
BR	BANK ROOM	FR	ROAD FENCE	RM	ROAD MARK
BS	BANK SIGN	FS	FIELD SIGN	RO	ROAD JUNCTION
BT	BANK TOWER	FT	FIELD TOWER	RP	ROAD PIN
BU	BANK UTM	FV	FLOOD VALVE	RS	ROAD SIGN
BY	BANK YARD	FV	FLOOD VALVE	RT	ROAD TOWER
CA	CANAL	GV	GRASS	RU	ROAD UTM
CB	CANAL BANK	HW	HARD BED	RV	ROAD VALVE
CC	CANAL CHAMBER	IW	IRON WORK	SW	STONE WALL
CD	CANAL DAM	LS	LEASURE	TR	TRAIL
CE	CANAL ENTRANCE	MS	MUD	TS	TRUCK SIGN
CF	CANAL FENCE	MT	METAL	TU	TURF
CG	CANAL GATE	NU	NUD	TV	TRUCK VALVE
CH	CANAL HEAD	OV	OVERFLOW	TW	TRUCK WALL
CI	CANAL INFLECT	OW	OIL WORK	TX	TRUCK
CJ	CANAL JUNCTION	PA	PINE	TY	TURF YARD
CK	CANAL KILL	PE	PEASE	UZ	UTM ZONE
CL	CANAL LIFT	PL	PINE	VA	VAULT
CM	CANAL MOUND	PM	PINE	VB	VALVE BOX
CN	CANAL NURSERY	PP	PINE	VC	VALVE CHAMBER
CO	CANAL OFFICE	PR	PINE	VD	VALVE DAM
CP	CANAL POND	PS	PINE	VE	VALVE ENTRANCE
CQ	CANAL QUAY	PT	PINE	VF	VALVE FENCE
CR	CANAL RACE	PV	PINE	VG	VALVE GATE
CS	CANAL SIGN	PW	PINE	VH	VALVE HEAD
CT	CANAL TOWER	PX	PINE	VI	VALVE INLET
CU	CANAL UTILITY	PY	PINE	VJ	VALVE JUNCTION
CV	CANAL VALVE	PZ	PINE	VK	VALVE KEYS
CW	CANAL WALL	QA	QUICK	VL	VALVE LIFT
CX	CANAL CROSS	QB	QUICK	VM	VALVE MOUNT
CY	CANAL YARD	QC	QUICK	VN	VALVE NETWORK
CZ	CANAL ZONE	QD	QUICK	VO	VALVE OPENING

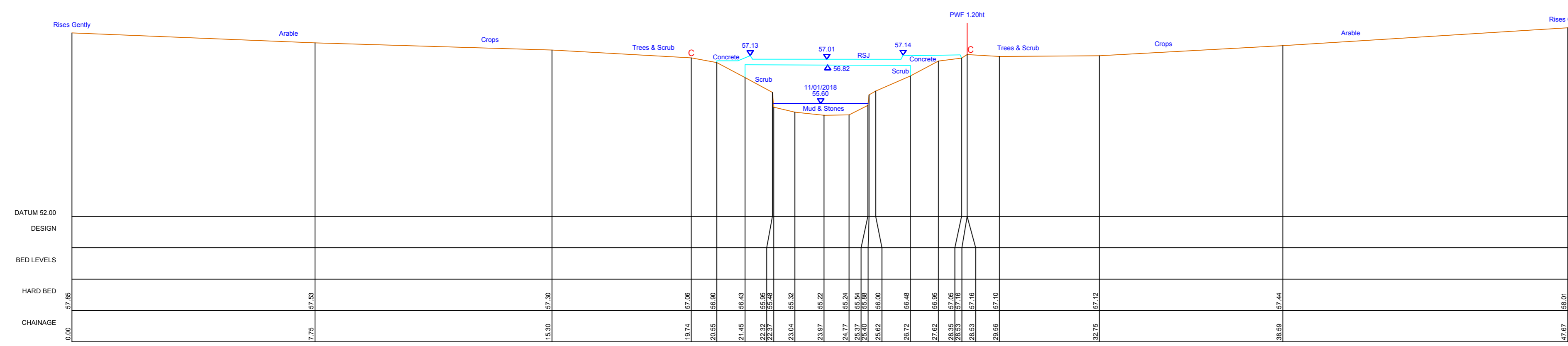
AMENDMENT	NO.	DATE

CONTROL USED:

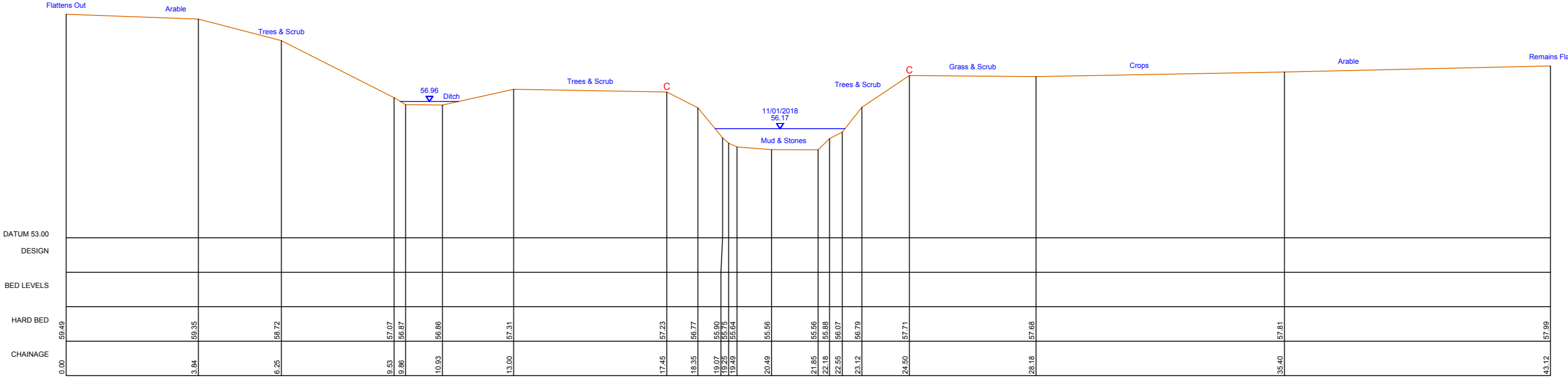
TYPE	DESCRIPTION	LEVEL
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E0730013	TR 0229 4827	36.480
E0730014	TR 0109 4202	36.480
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E0730016	TR 0109 4202	36.480
E0730017	TR 0109 4202	36.480
E0730018	TR 0109 4202	36.480
E0730019	TR 0109 4202	36.480
E0730020	TR 0109 4202	36.480
E0730021	TR 0109 4202	36.480
E0730022	TR 0109 4202	36.480
E0730023	TR 0109 4202	36.480
E0730024	TR 0109 4202	36.480
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E0730027	TR 0109 4202	36.480
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E0730029	TR 0109 4202	36.480
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E0730031	TR 0109 4202	36.480
E0730032	TR 0109 4202	36.480
E0730033	TR 0109 4202	36.480
E0730034	TR 0109 4202	36.480
E0730035	TR 0109 4202	36.480
E0730036	TR 0109 4202	36.480
E0730037	TR 0109 4202	36.480
E0730038	TR 0109 4202	36.480
E0730039	TR 0109 4202	36.480
E0730040	TR 0109 4202	36.480
E0730041	TR 0109 4202	36.480
E0730042	TR 0109 4202	36.480
E0730043	TR 0109 4202	36.480
E0730044	TR 0109 4202	36.480
E0730045	TR 0109 4202	36.480
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E0730047	TR 0109 4202	36.480
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E0730050	TR 0109 4202	36.480



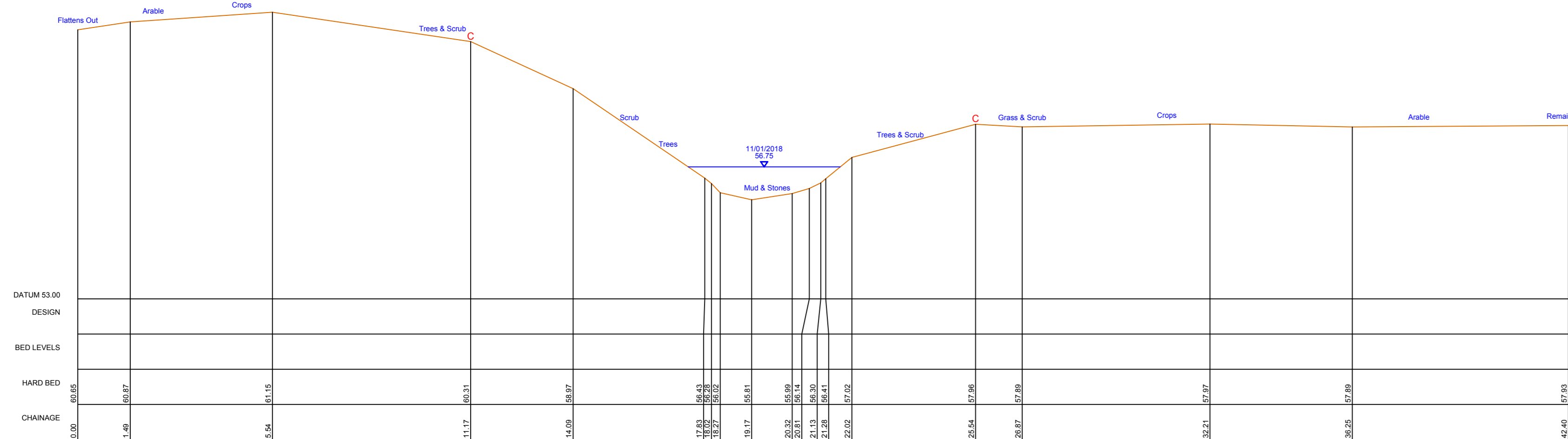
- KEY TO SECTIONS:**
- WATER LEVEL
 - VISIBLE BED (TOP OF SILT) AND GROUND
 - HARD BED (DETERMINED BY PROBING)
 - BANK CREST
- KEY TO LONGITUDINAL SECTION ONLY:**
- VIEWED LOOKING DOWNSTREAM
 - CROSS SECTIONS AND ADDITIONAL POINTS INDICATED BY 'C' ON
 - LEFT BANK CREST
 - RIGHT BANK CREST
 - POINTS BETWEEN SECTIONS



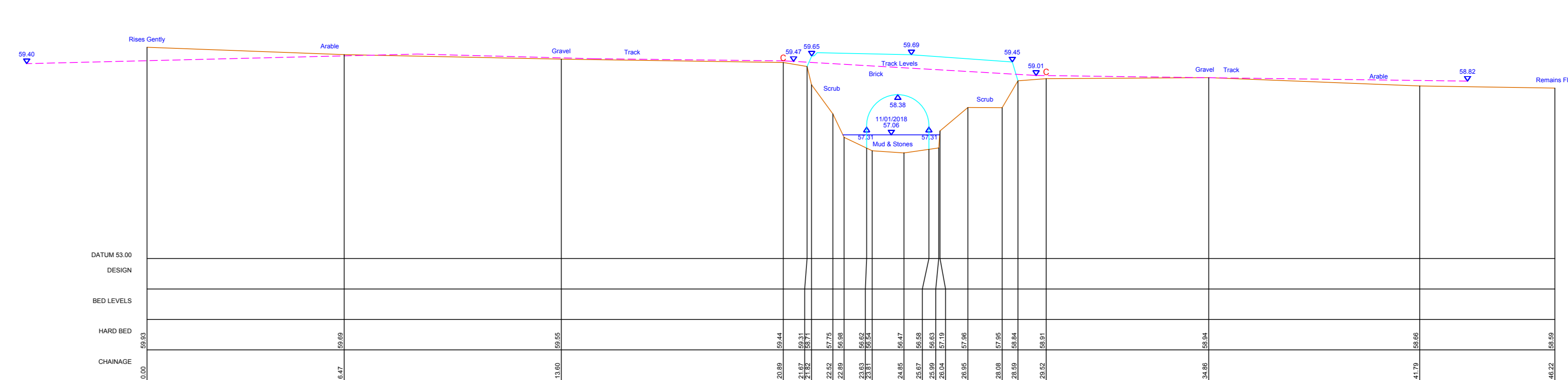
ESTO01_14527
610084.61mE 137553.17mN Brg 339
Dilapidated Footbridge
Tunnel Length = 2.14m



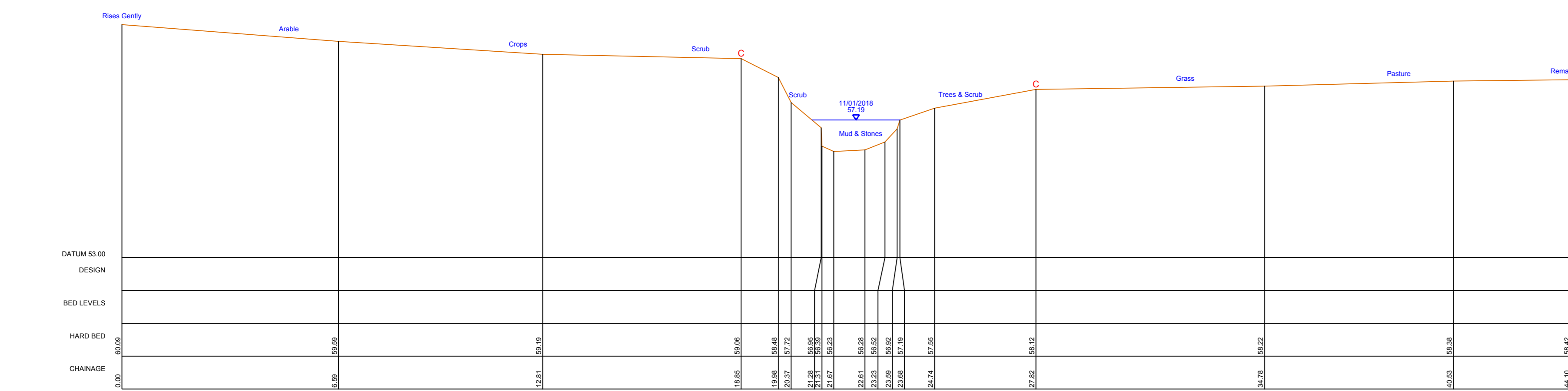
ESTO01_14631
610180.01mE 137588.2mN Brg 345
Open Channel



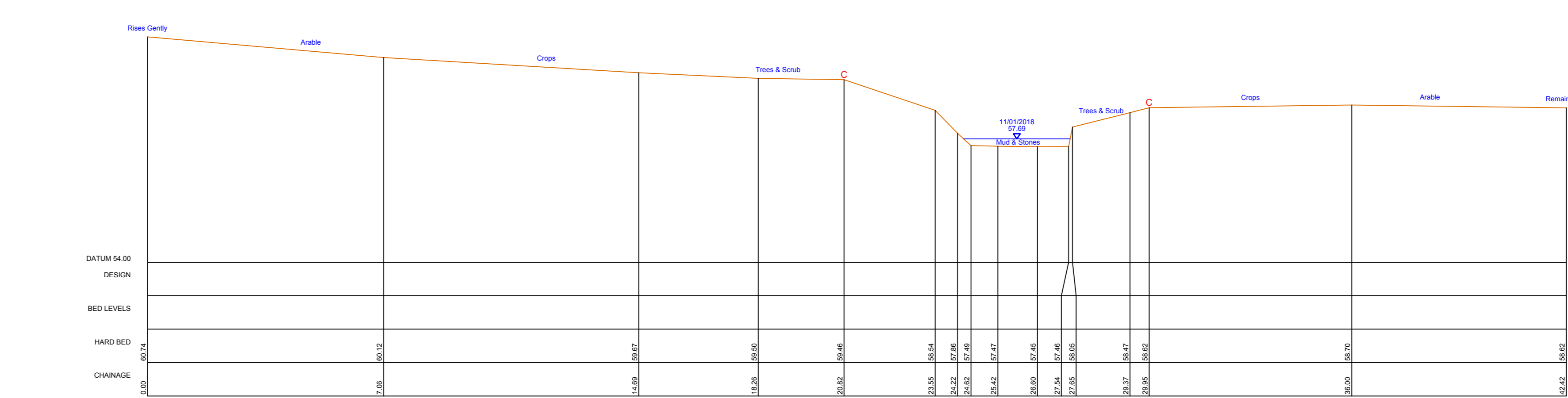
ESTO01_14725
610247.58mE 137608.18mN Brg 39
Open Channel



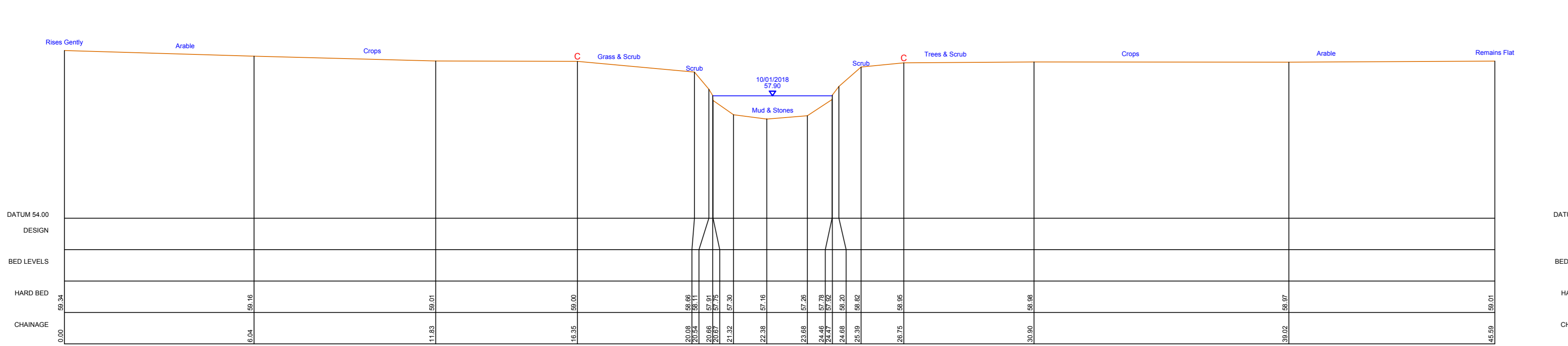
ESTO01_14781
610290.22mE 137567.02mN Brg 32
Access Bridge
Tunnel Length = 3.92m



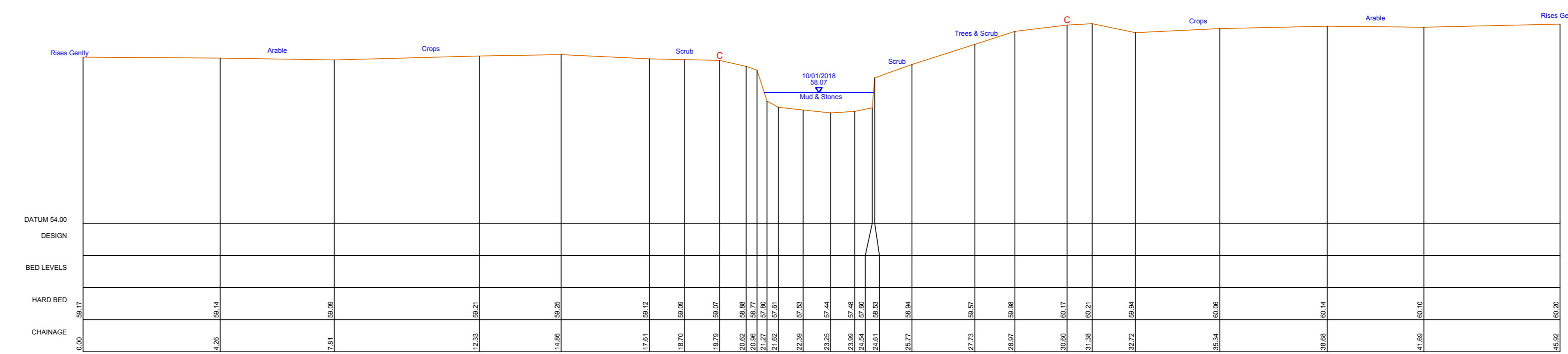
ESTO01_14833
610330.35mE 137535.82mN Brg 35
Open Channel



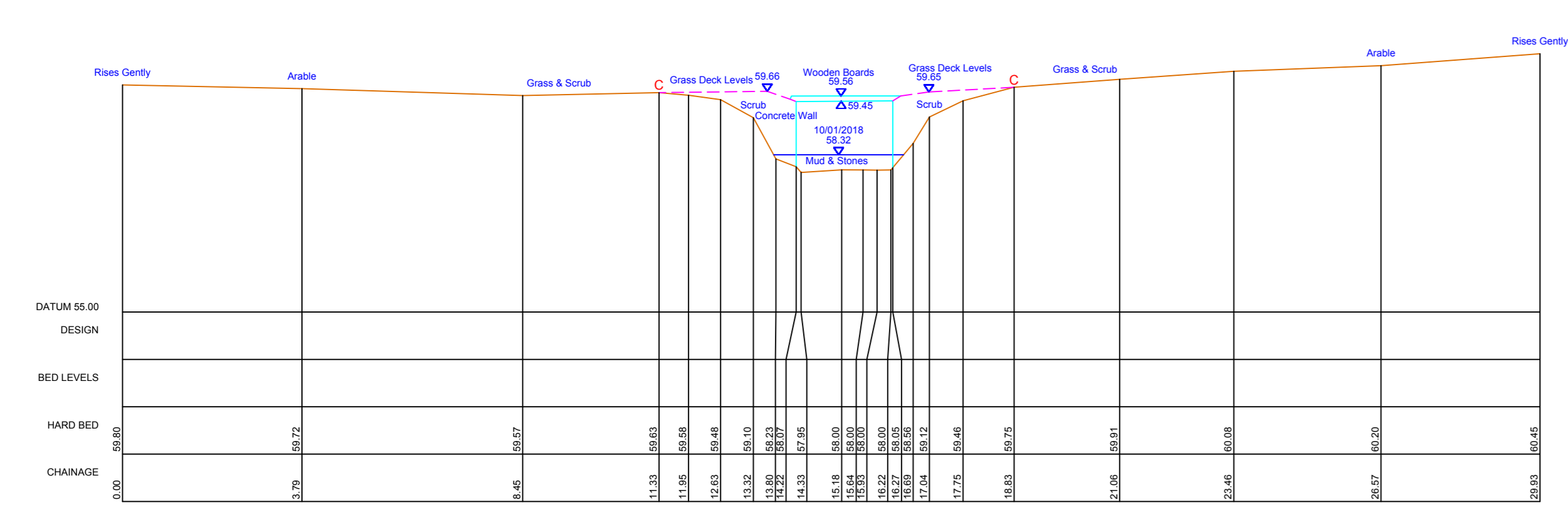
ESTO01_14932
610394.67mE 137474.12mN Brg 52
Open Channel



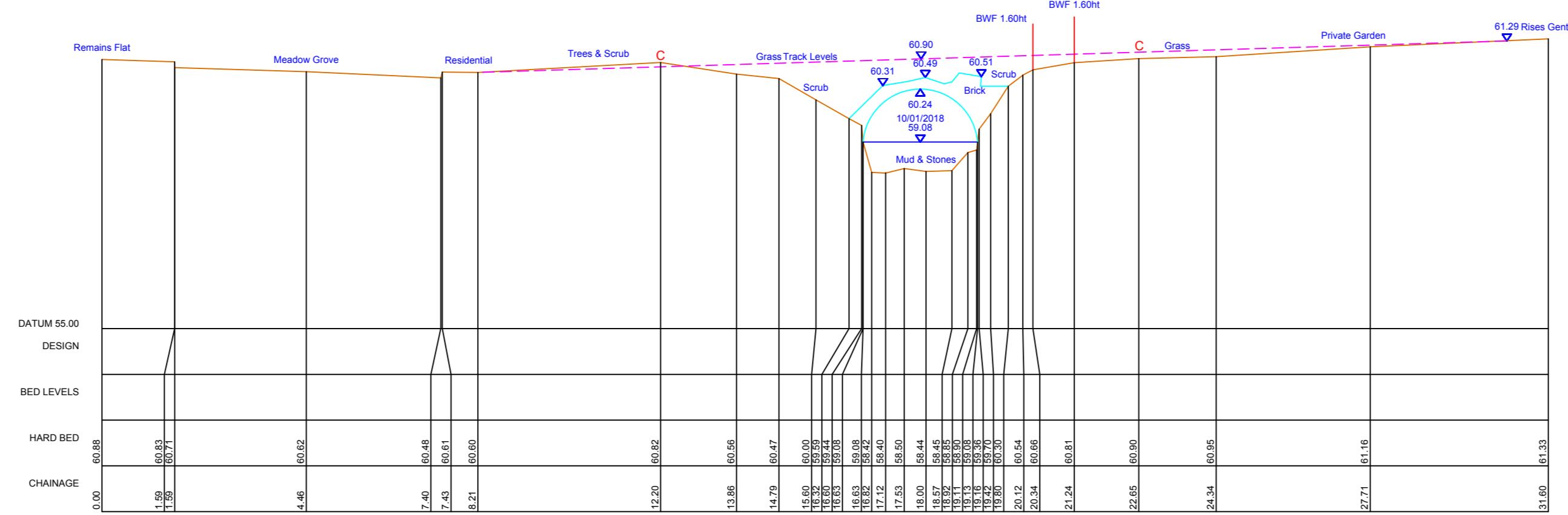
ESTO01_15026
610453.59mE 137407.07mN Brg 14
Open Channel



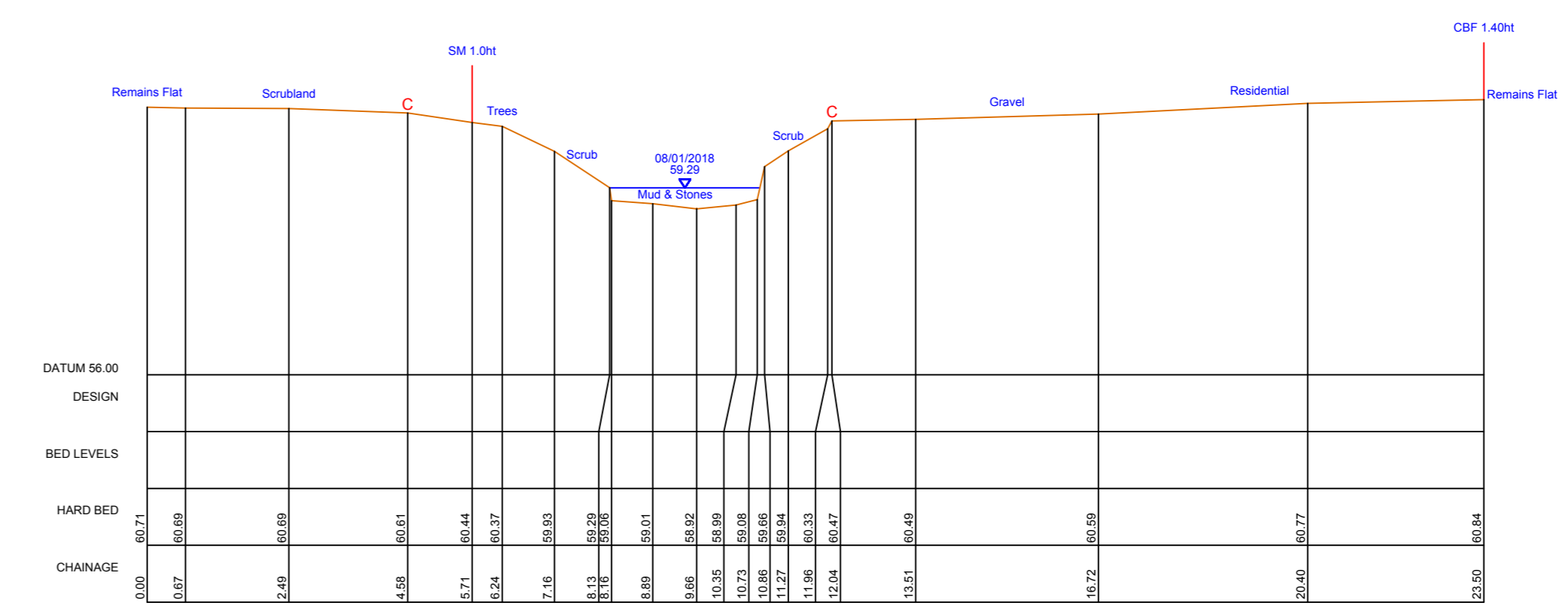
ESTO01_15121
610552.52mE 137414.87mN Brg 341
Open Channel



ESTO01_15220
610630.35mE 137481.74mN Brg 311
Footbridge
Tunnel Length = 3.69m



ESTO01_15347
610698.75mE 137593.18mN Brg 296
Footbridge
Tunnel Length = 2.64m



ESTO01_15375
610706.45mE 137612.14mN Brg 329
Open Channel

NOTES:

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SURVEY LEGEND

SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
...

AMENDMENT

NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E207230012	TR 0103 4107	35.925
E207234013	TR 0259 4227	36.280
E207234014	TR 0109 4202	36.480
E207234015	TR 0109 4202	36.480
E207234016	TR 0109 4202	36.480
E207234017	TR 0109 4202	36.480
E207234018	TR 0109 4202	36.480
E207234019	TR 0109 4202	36.480
E207234020	TR 0109 4202	36.480
E207234021	TR 0109 4202	36.480
E207234022	TR 0109 4202	36.480
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E207234028	TR 0109 4202	36.480
E207234029	TR 0109 4202	36.480
E207234030	TR 0109 4202	36.480
E207234031	TR 0109 4202	36.480
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E207234035	TR 0109 4202	36.480
E207234036	TR 0109 4202	36.480
E207234037	TR 0109 4202	36.480
E207234038	TR 0109 4202	36.480
E207234039	TR 0109 4202	36.480
E207234040	TR 0109 4202	36.480
E207234041	TR 0109 4202	36.480
E207234042	TR 0109 4202	36.480
E207234043	TR 0109 4202	36.480
E207234044	TR 0109 4202	36.480
E207234045	TR 0109 4202	36.480
E207234046	TR 0109 4202	36.480
E207234047	TR 0109 4202	36.480
E207234048	TR 0109 4202	36.480
E207234049	TR 0109 4202	36.480
E207234050	TR 0109 4202	36.480

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Endersburg Park, London Road, Addington, West Malling, Kent, ME19 5QH

PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS:
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_14527 TO ESTO01_15375

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref: 12_152*

SURVEY DATE: DECEMBER 2017 – MARCH 2018

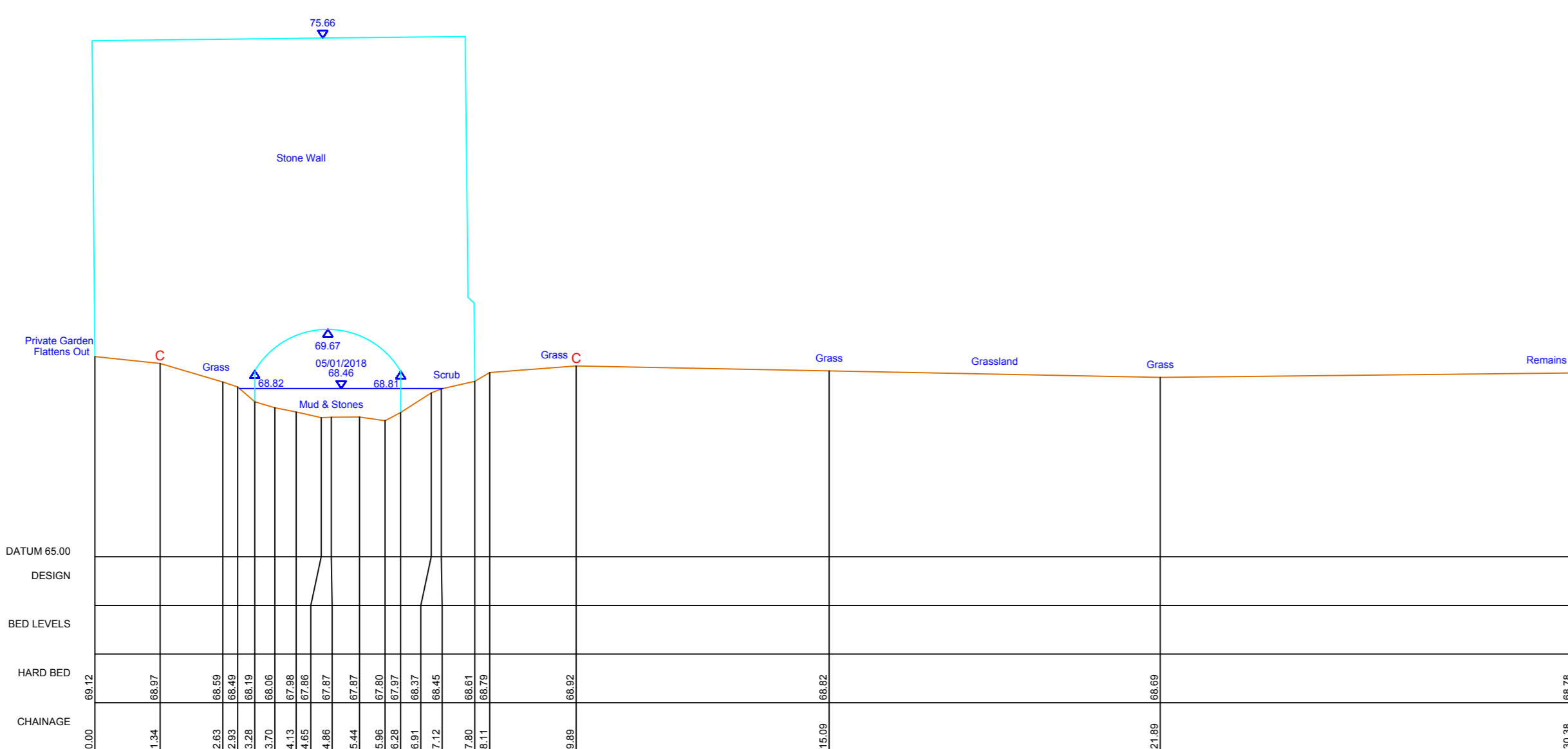
SCALE: 1:100 **DRN:** RC **CHKD:** ITS

DATUM: OS GPS ACTIVE **DATE:** MAR 18 **DATE:** MAR 18

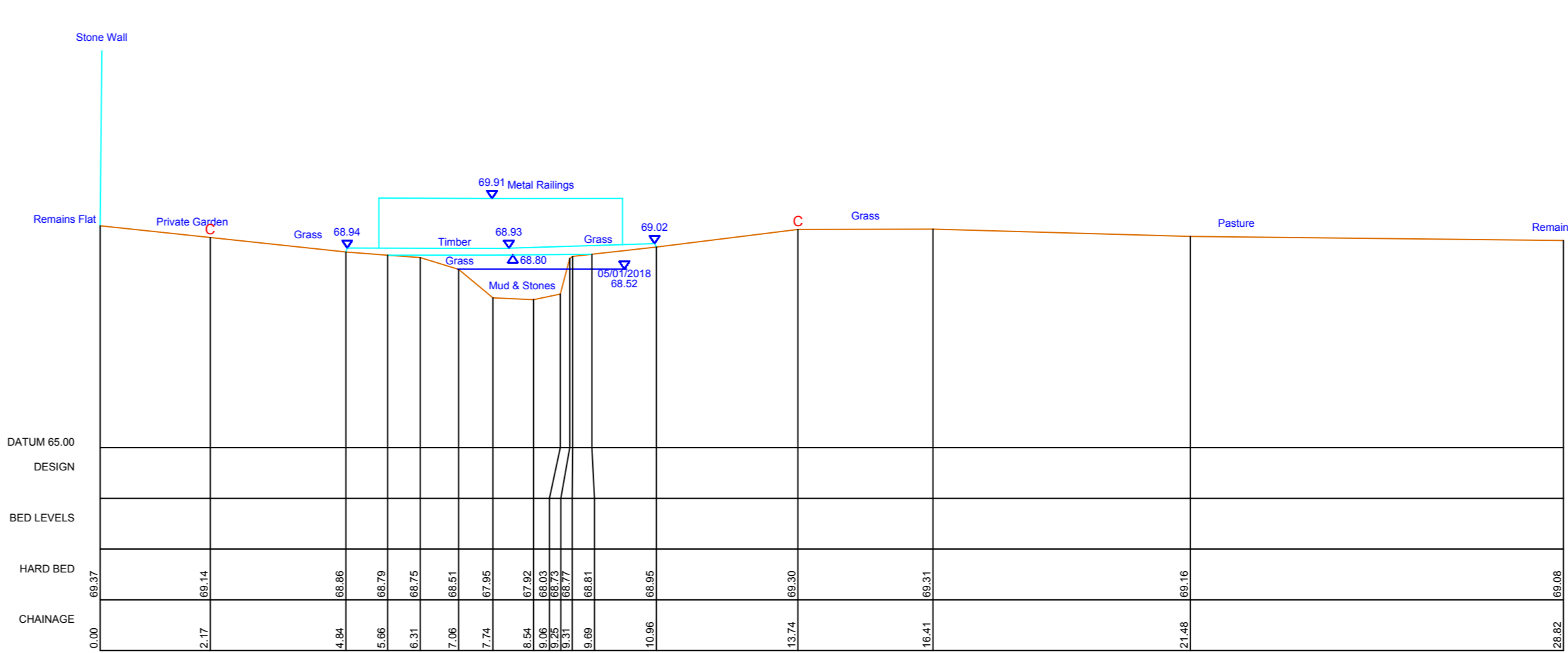
GRID: NATIONAL GRID **DRAWING NO.:** X-J01058-22 **REV.:** 1

DWG FILENAME: E-2018-01-20.dwg

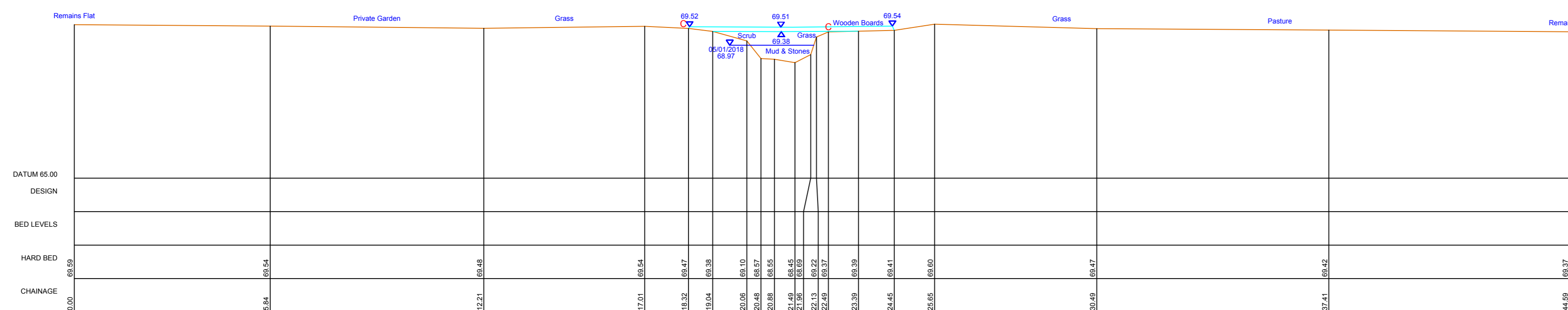
- KEY TO SECTIONS:**
- WATER LEVEL
 - VISIBLE BED (TOP OF SILT) AND GROUND
 - HARD BED (DETERMINED BY PROBING)
 - BANK CREST
- KEY TO LONGITUDINAL SECTION ONLY:**
- VIEWED LOOKING DOWNSTREAM
 - LEFT BANK CREST
 - CROSS SECTIONS AND ADDITIONAL POINTS INDICATED BY 'C' ON
 - RIGHT BANK CREST
 - CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



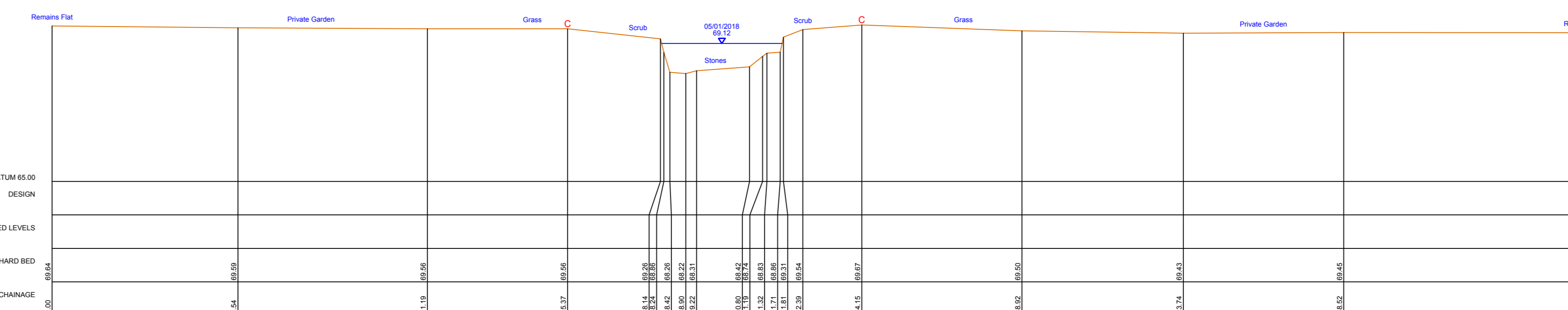
ESTO01_18307
612261.16mE 137218.29mN Brg 9
Culvert Entrance
Tunnel Length = 9.58m



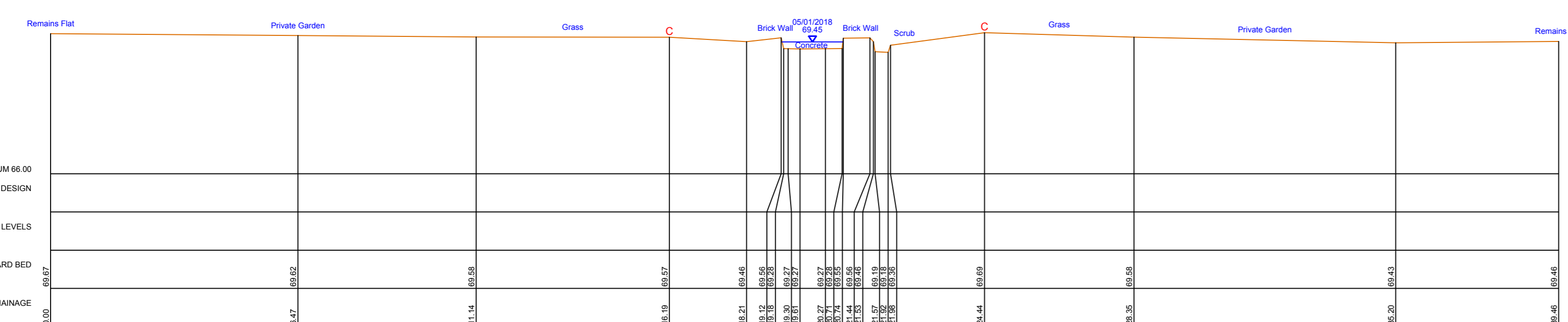
ESTO01_18328
612286.77mE 137211.57mN Brg 334
Footbridge
Tunnel Length = 0.81m



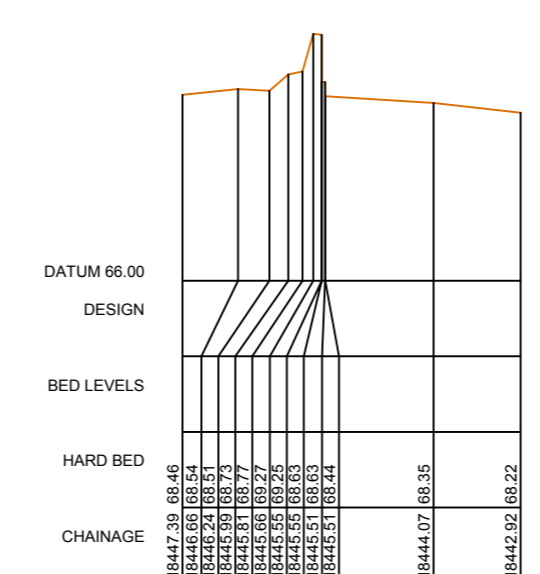
ESTO01_18435
612382.99mE 137225.92mN Brg 338
Footbridge
Tunnel Length = 0.93m



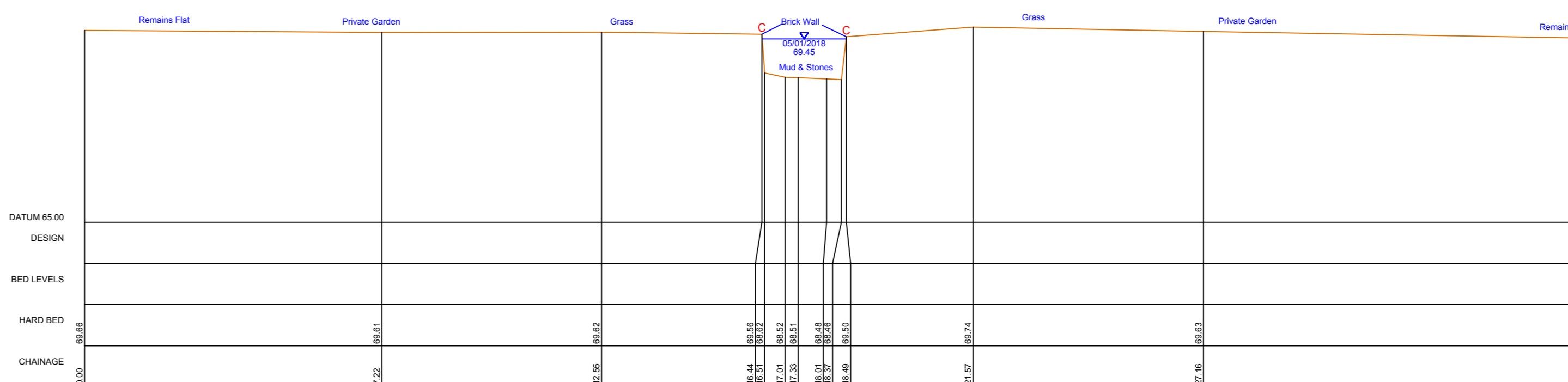
ESTO01_18443
612389.91mE 137230.47mN Brg 336
Weir Toe



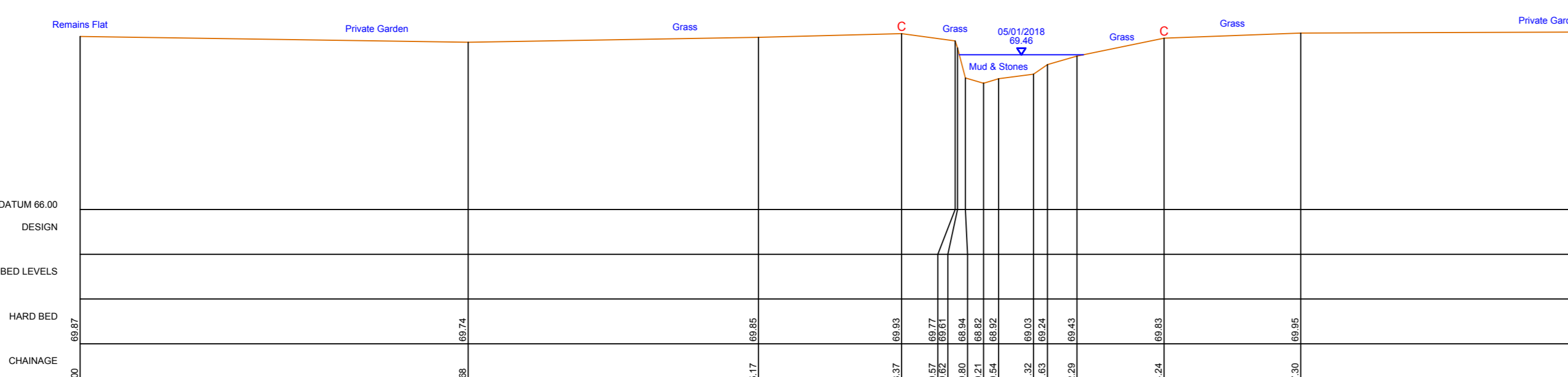
ESTO01_18446
612392.04mE 137231.05mN Brg 337
Weir Crest



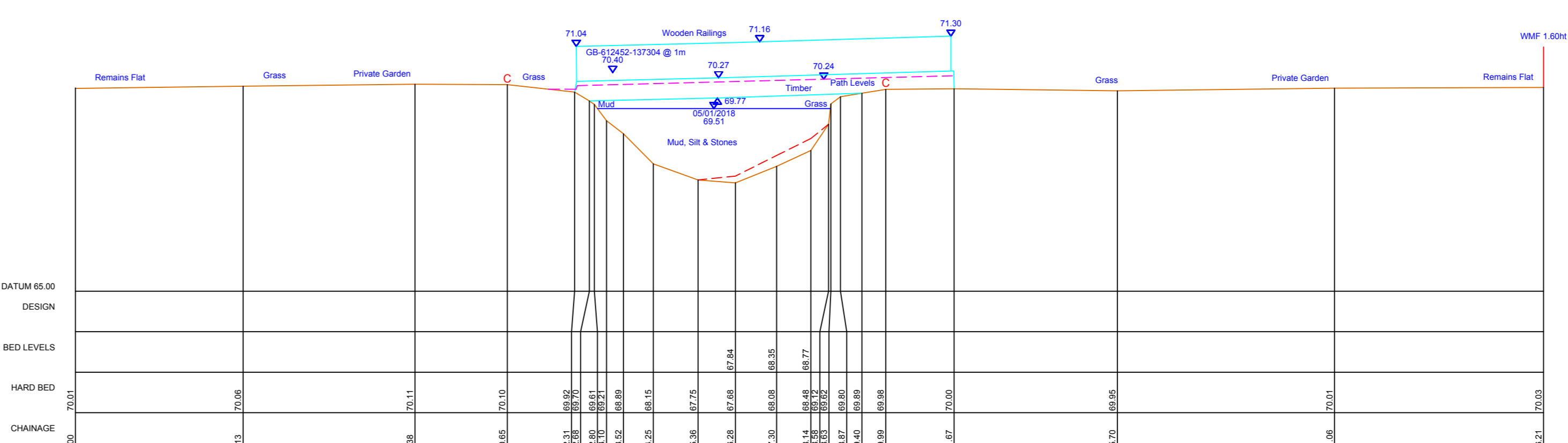
Through Section ESTO01_18446
Weir Crest



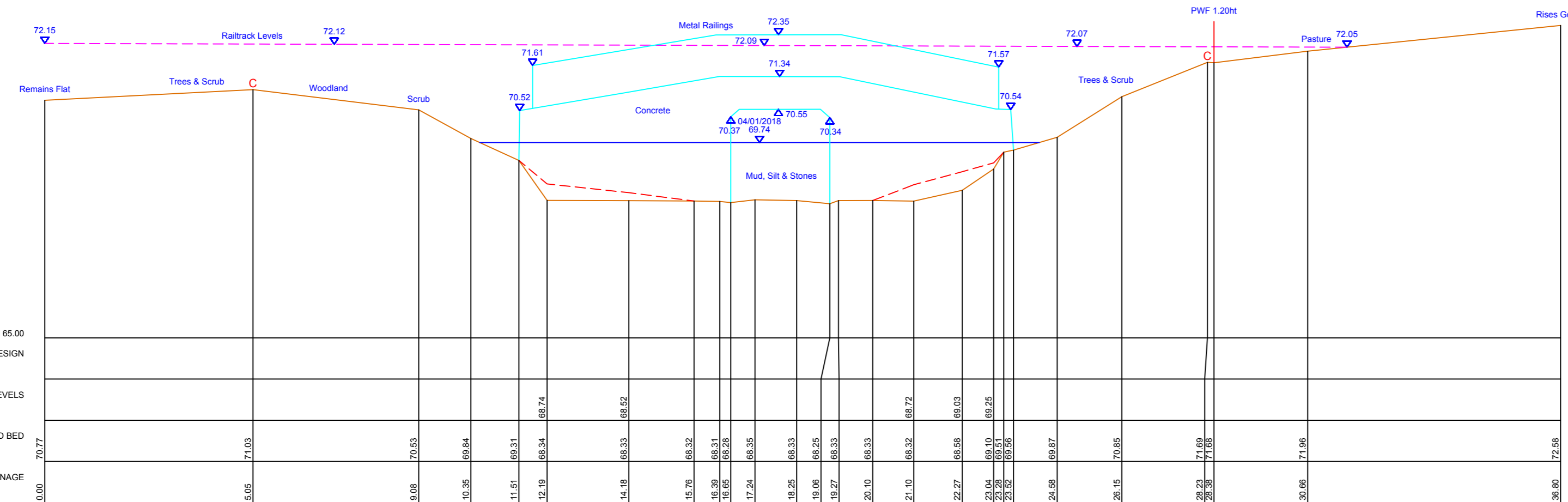
ESTO01_18447
612391.62mE 137233.89mN Brg 340
Weir Heel



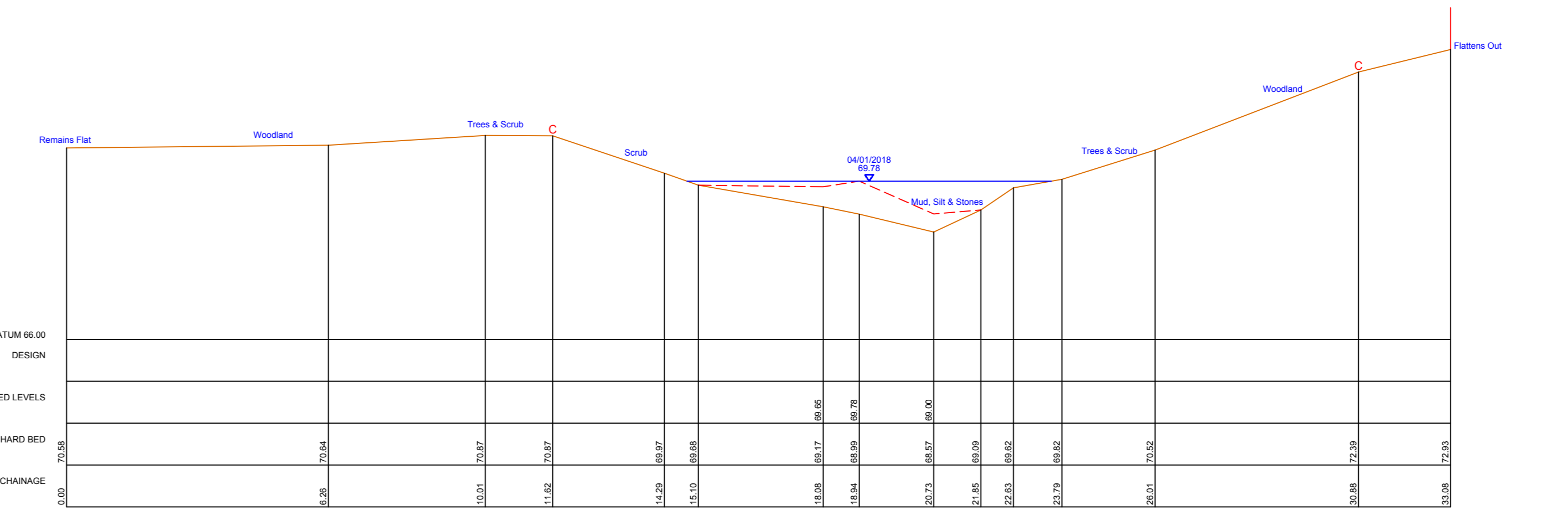
ESTO01_18509
612449.6mE 137270.65mN Brg 317
Open Channel



ESTO01_18526
612462.48mE 137293.66mN Brg 286
Footbridge
Tunnel Length = 0.76m



ESTO01_18583
612479.25mE 137349.56mN Brg 283
Culvert Entrance
Tunnel Length = 50.42m



ESTO01_18620
612494.24mE 137352.33mN Brg 351
Open Channel

NOTES:

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- UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
...

AMENDMENT	NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
...

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Leisure Park, London Road, Ashford, Kent, ME19 5QH

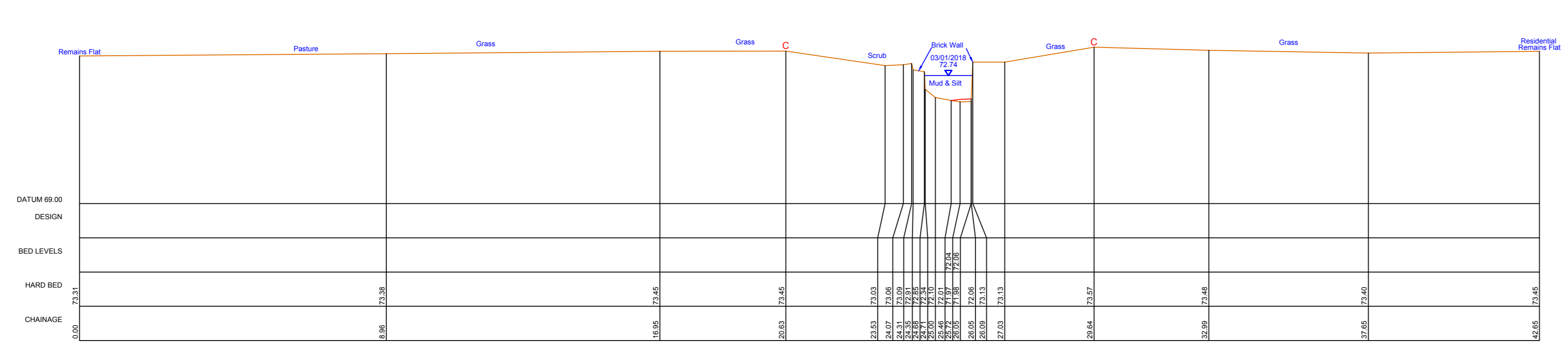
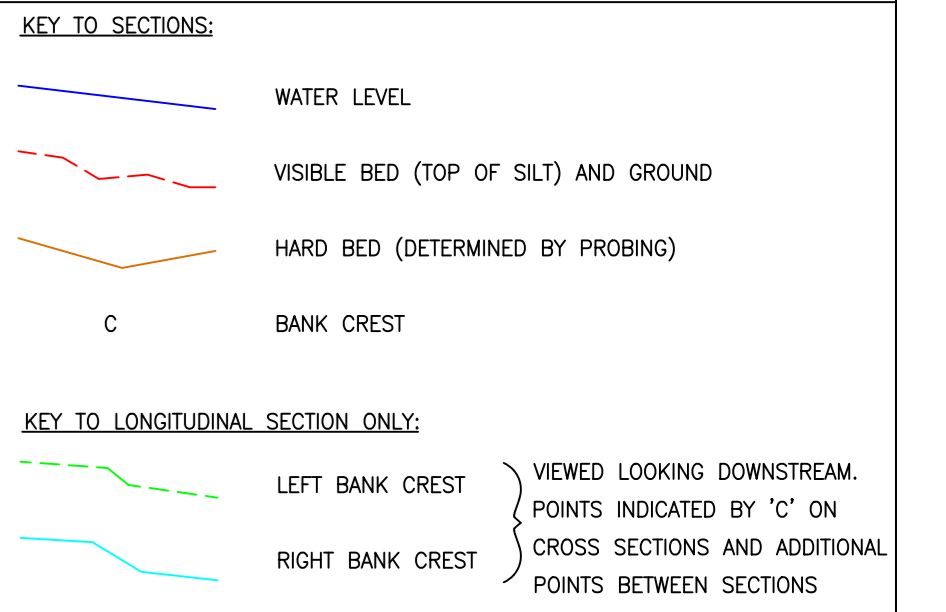
PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_18307 TO ESTO01_18620

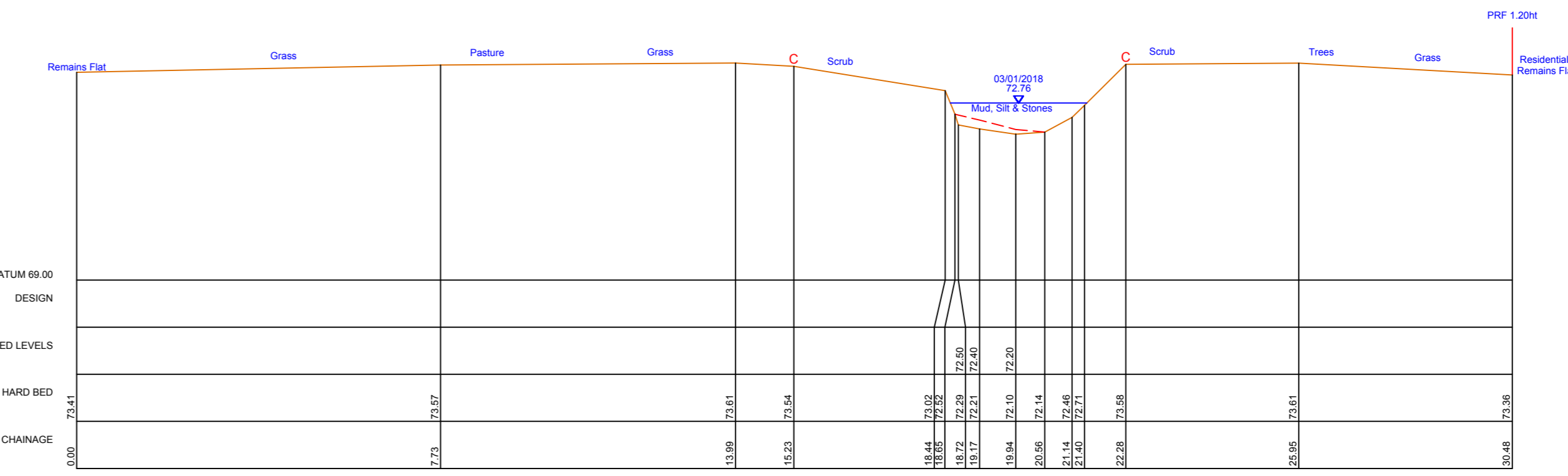
SURVEYED BY: MALTBY LAND SURVEYS LTD
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100
DATUM: OS GPS ACTIVE
GRID: NATIONAL GRID

DRN: RC
DATE: MAR 18
DRAWING NO. X-J01058-27

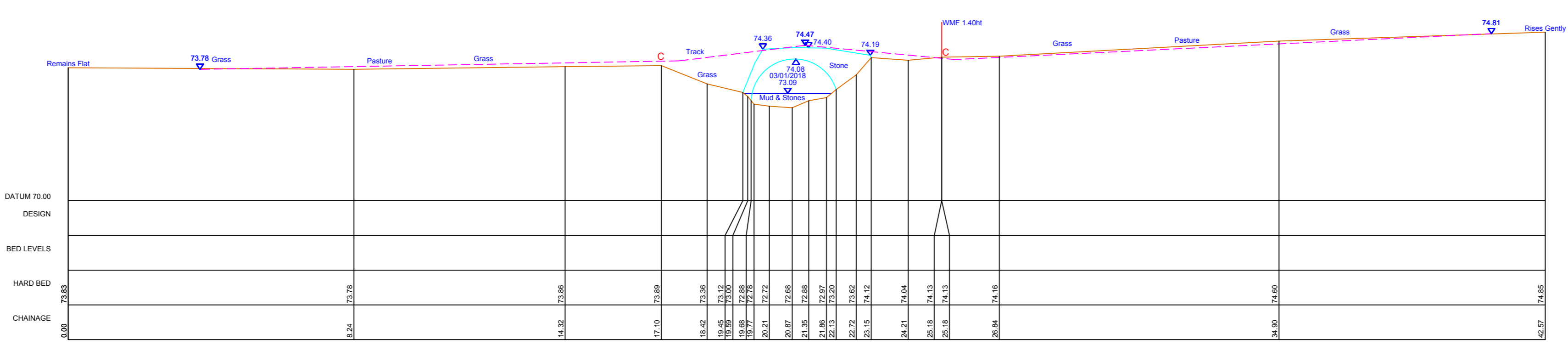
CHKD: ITS
DATE: MAR 18
REV.:



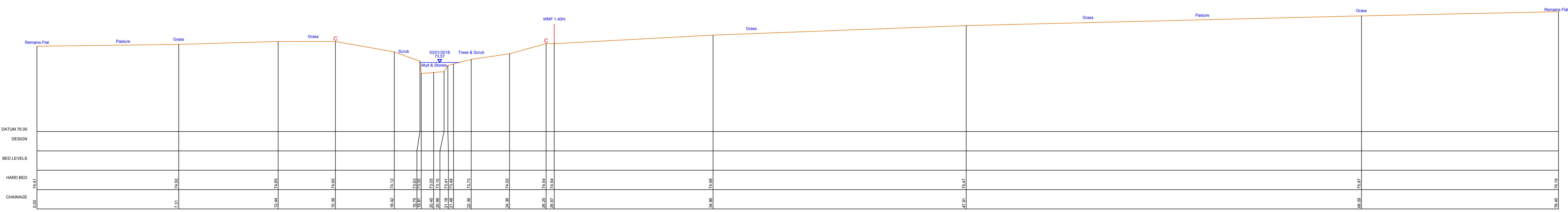
ESTO01_19399
613068.21mE 137672.01mN Brg 256
Open Channel



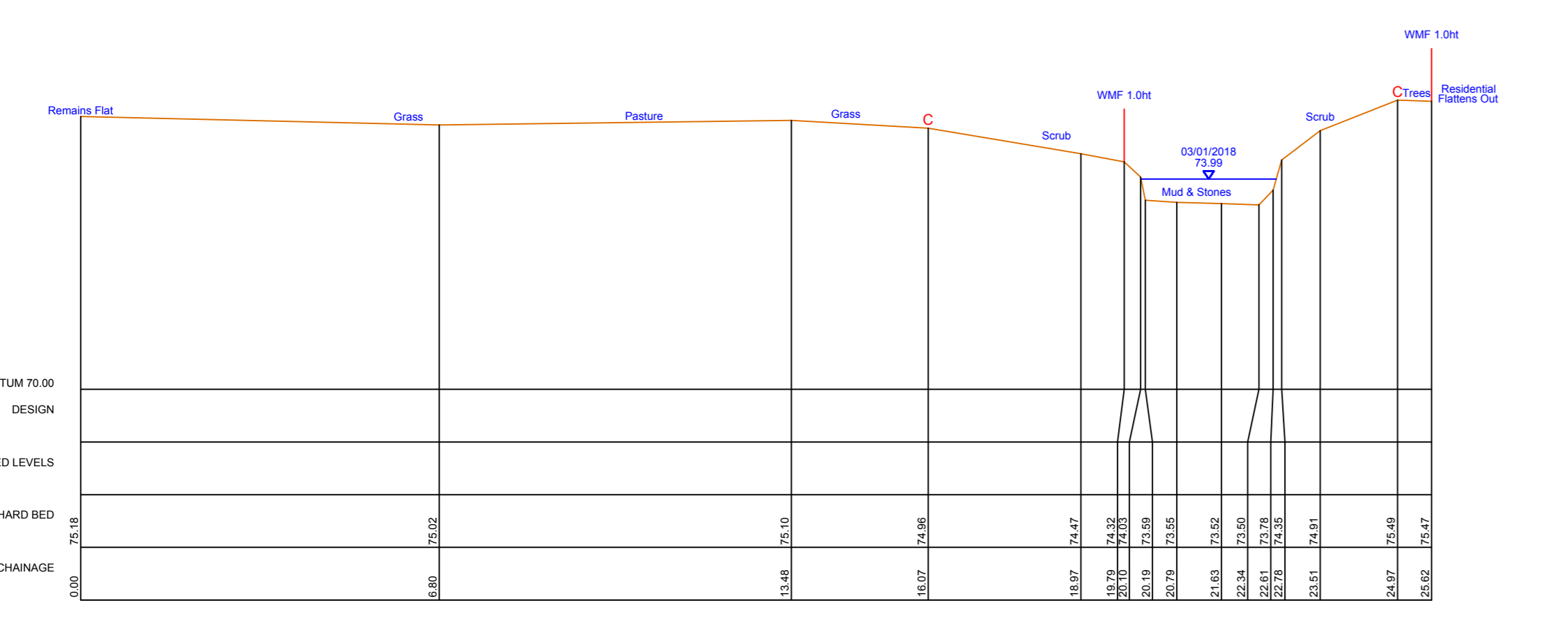
ESTO01_19411
613066.71mE 137681.38mN Brg 254
Open Channel



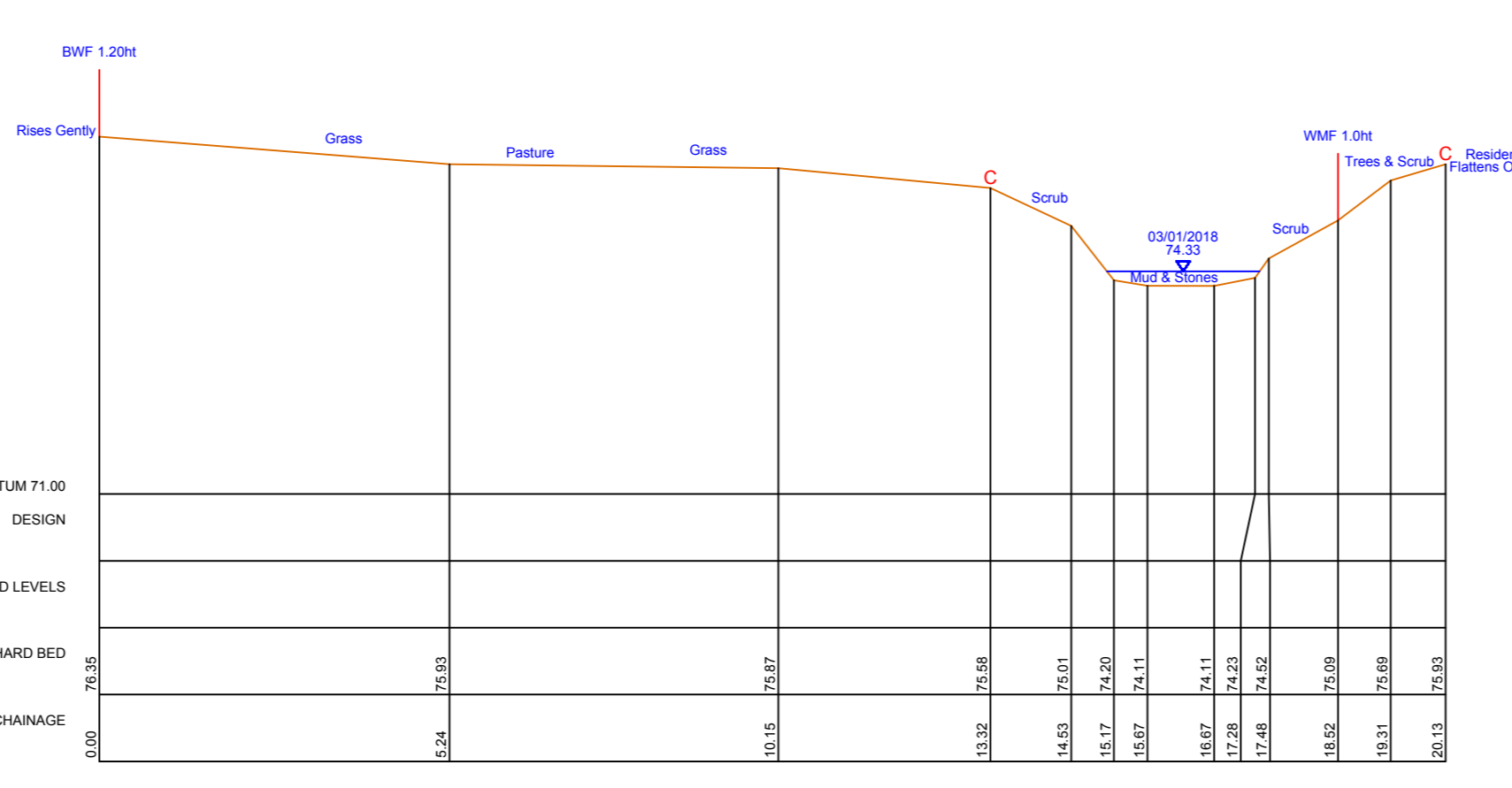
ESTO01_19524
613050.39mE 137784.9mN Brg 276
Footbridge
Tunnel Length = 2.82m



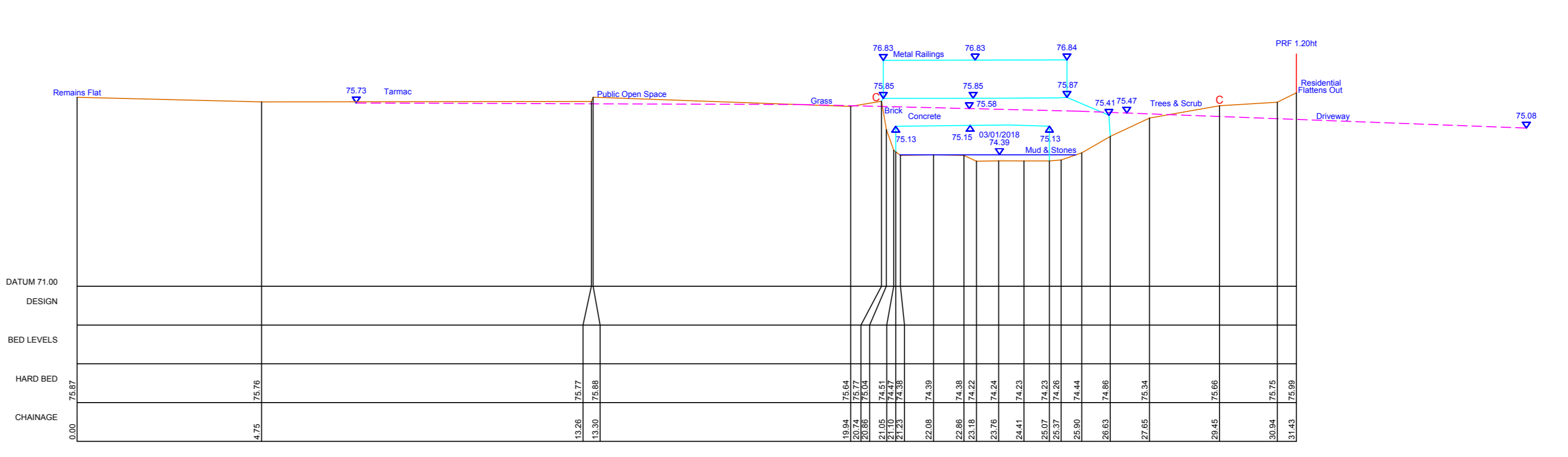
ESTO01_19632
613045.39mE 137890.96mN Brg 274
Open Channel



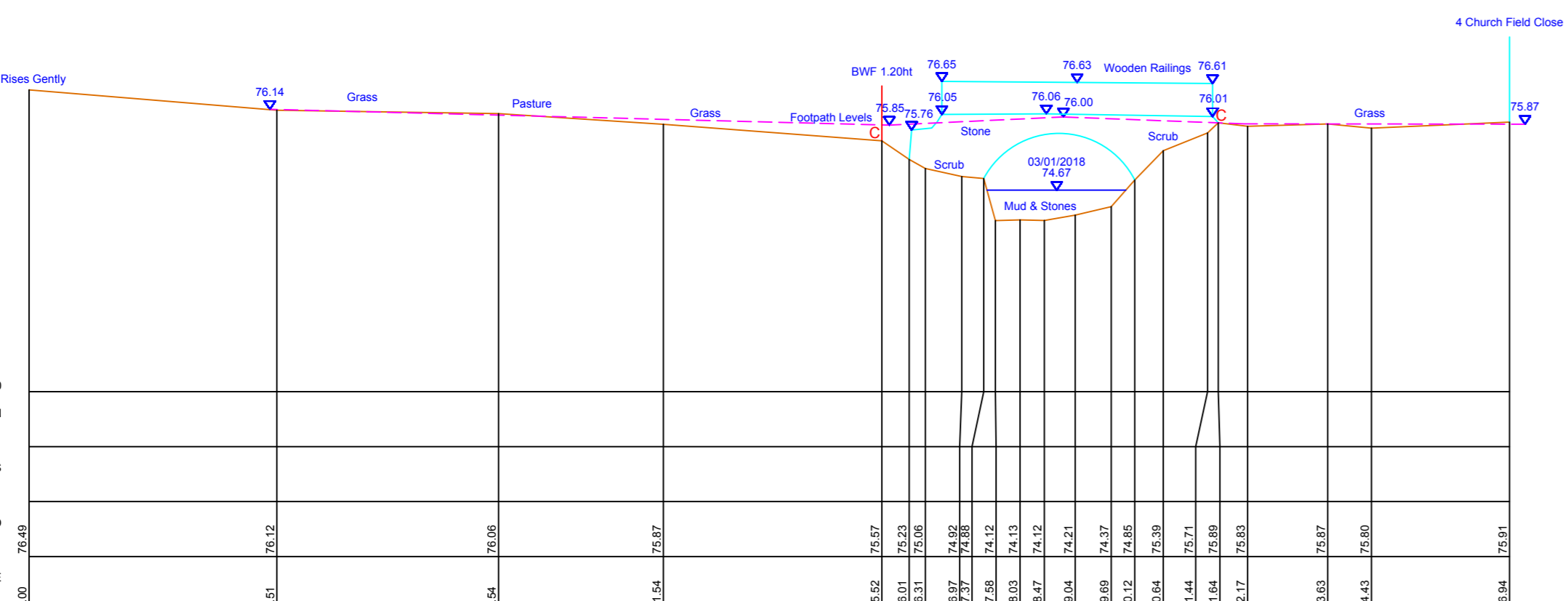
ESTO01_19727
613017.93mE 137992.94mN Brg 241
Open Channel



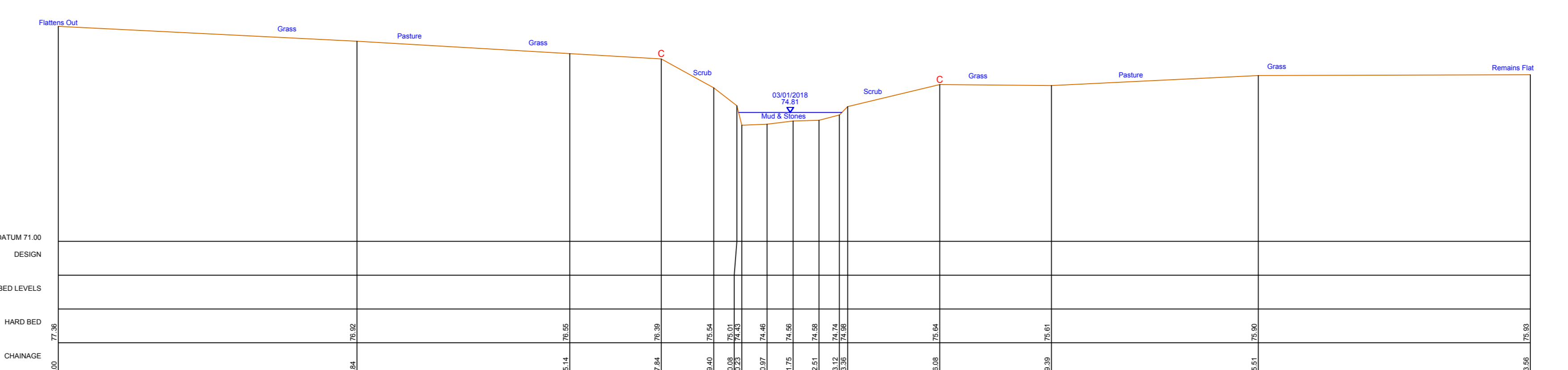
ESTO01_19930
612999.06mE 138078.28mN Brg 269
Open Channel



ESTO01_19852
613003.96mE 138087.48mN Brg 303
Access Bridge
Tunnel Length = 7.37m



ESTO01_19902
613028.95mE 138129.15mN Brg 300
Footbridge
Tunnel Length = 2.35m



ESTO01_19939
613064.37mE 138144.86mN Brg 295
Open Channel

NOTES:
1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
2. THIS MAP IS REPRODUCED FROM THE OS MAP BY THE ENVIRONMENT AGENCY WITH PERMISSION OF ORDNANCE SURVEY ON BEHALF OF THE CONTROLLER OF HER MAJESTY'S STATIONERY OFFICE. © CROWN COPYRIGHT LICENCE. ALL RIGHTS RESERVED. UNAUTHORISED REPRODUCTION INFRINGES CROWN COPYRIGHT AND MAY LEAD TO PROSECUTION OR CIVIL PROCEEDINGS. LICENCE NO. 100026330.
3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

AS BRCK	FW	THE CHANNEL	FW	PROTECT WALL
AW WALL	FW	THE CHANNEL	FW	SOFT GRTS
AS BRCK	FW	THE CHANNEL	FW	SOFT GRTS
AW WALL	FW	THE CHANNEL	FW	SOFT GRTS
AS BRCK	FW	THE CHANNEL	FW	SOFT GRTS
AW WALL	FW	THE CHANNEL	FW	SOFT GRTS
AS BRCK	FW	THE CHANNEL	FW	SOFT GRTS
AW WALL	FW	THE CHANNEL	FW	SOFT GRTS
AS BRCK	FW	THE CHANNEL	FW	SOFT GRTS
AW WALL	FW	THE CHANNEL	FW	SOFT GRTS

AMENDMENT	DRN	CHD	DATE

TYPE	DESCRIPTION	LEVEL
E02730012	TR 0103 4107	56.975
E02730403	TR 0229 4627	58.480
E02730019	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
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E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480
E02730021	TR 0109 4202	58.480

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Ordnance Park, London Road, Aldington, Maidstone, Kent, ME18 5QH

PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

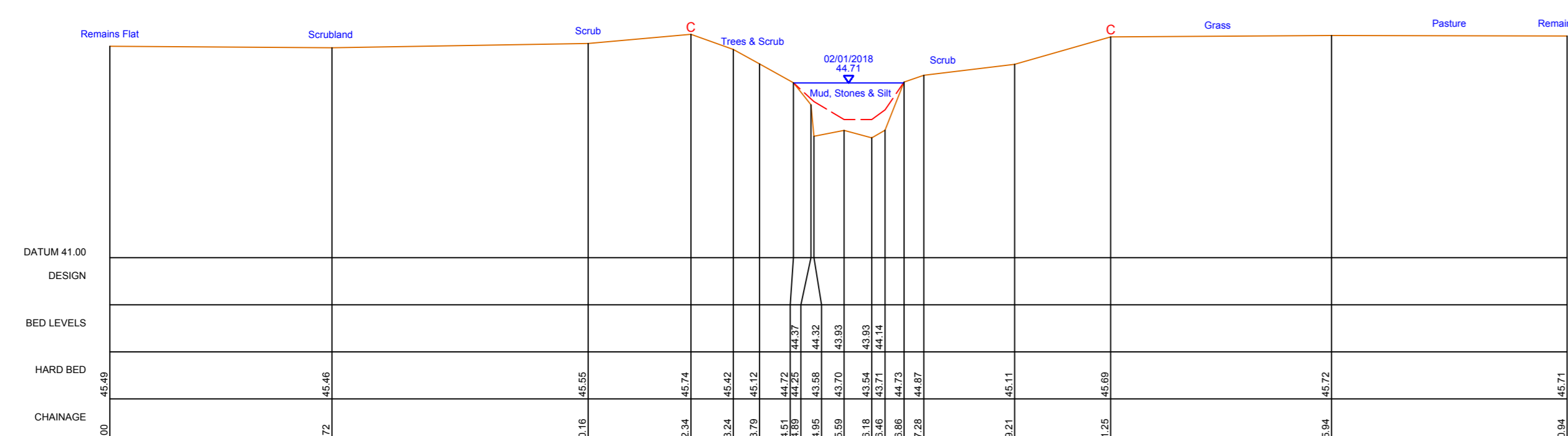
SITE/UMTS:
EAST STOUR (ESTO01)
CROSS SECTIONS
ESTO01_19399 TO ESTO01_19939

SURVEYED BY: MALBY LAND SURVEYS LTD *Ref: 12_107*
SURVEY DATE: DECEMBER 2017 - MARCH 2018
SCALE: 1:100
DATUM: OS GPS ACTIVE
GRID: NATIONAL GRID

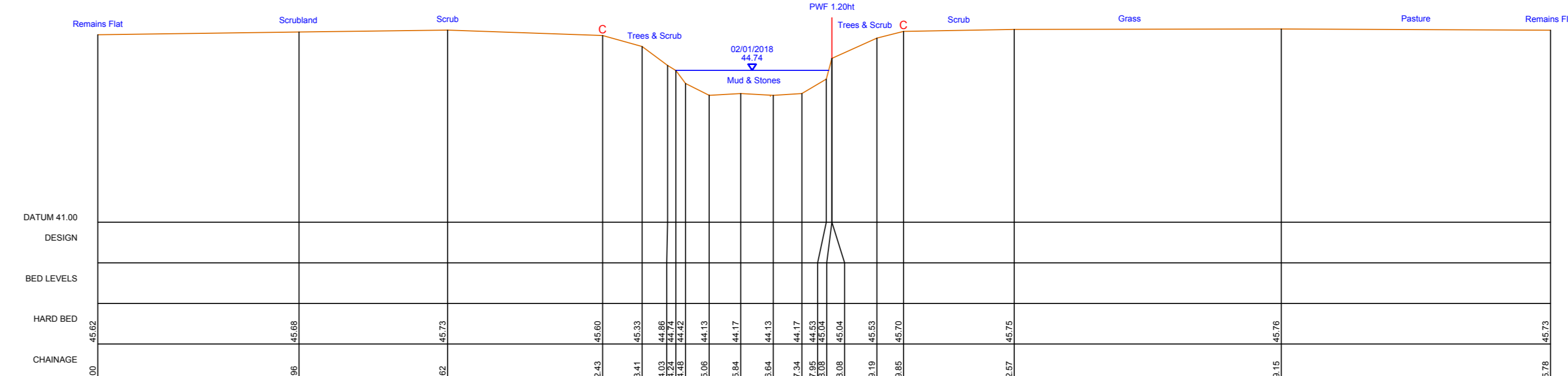
DRN: RC
CHKD: ITS
DATE: MAR 18
DATE: MAR 18

DWG FILENAME: X-2018-01-20.dwg
DRAWING NO.: X-J01058-29
REV.:

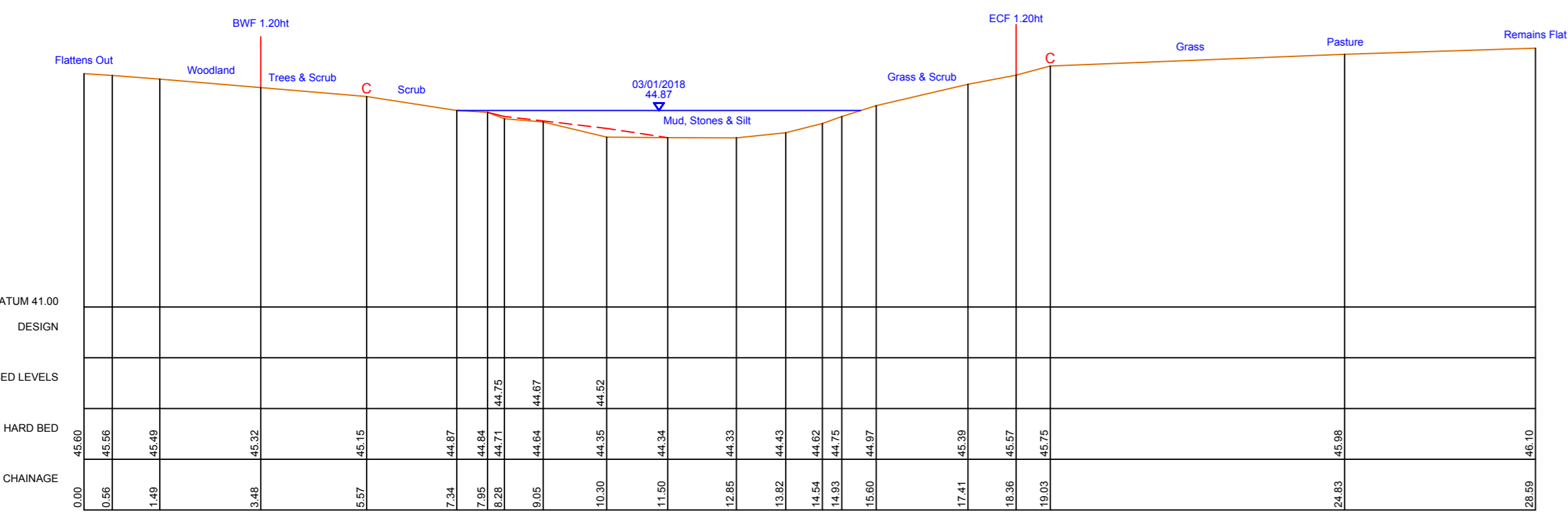
- KEY TO SECTIONS:**
- WATER LEVEL
 - VISIBLE BED (TOP OF SILT) AND GROUND
 - HARD BED (DETERMINED BY PROBING)
 - C BANK CREST
- KEY TO LONGITUDINAL SECTION ONLY:**
- VIEWED LOOKING DOWNSTREAM
 - LEFT BANK CREST
 - RIGHT BANK CREST
- POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



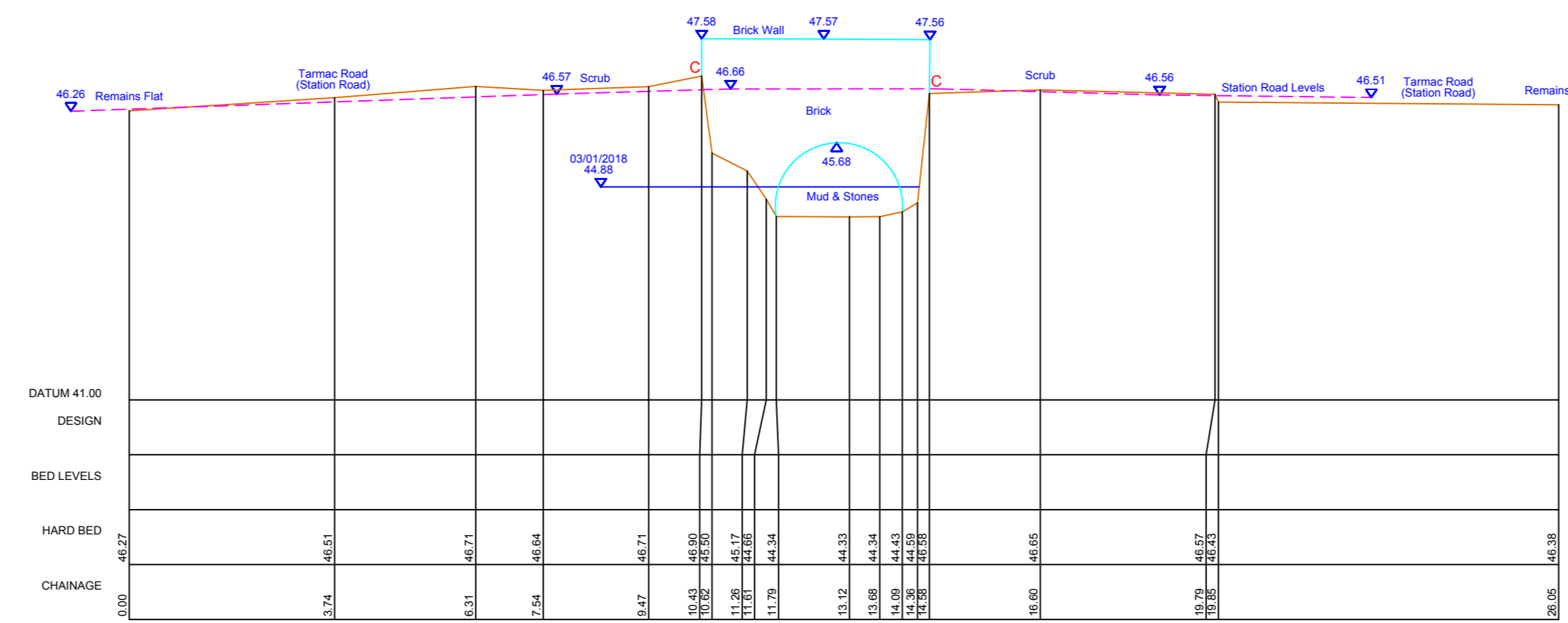
ESTO06_0036
606187.2mE 138053.63mN Brg 58
Open Channel



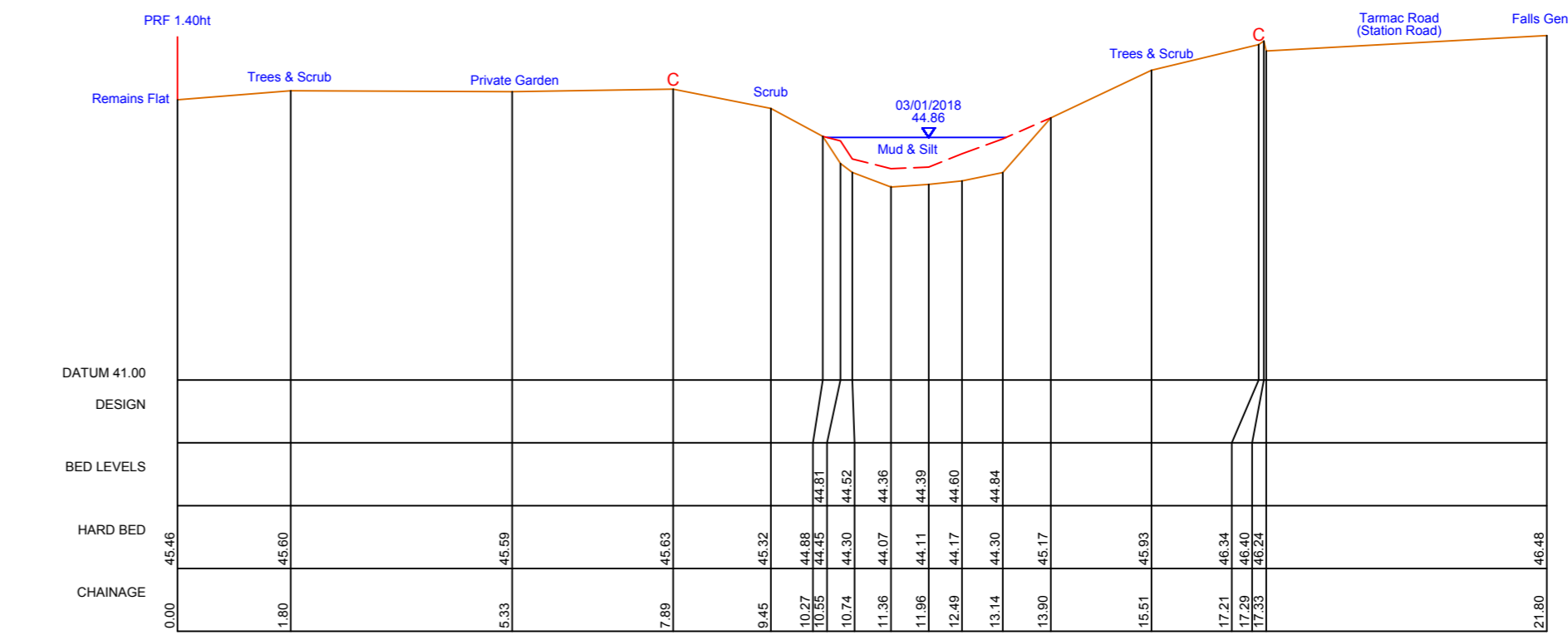
ESTO06_0096
606234.48mE 138025.55mN Brg 77
Open Channel



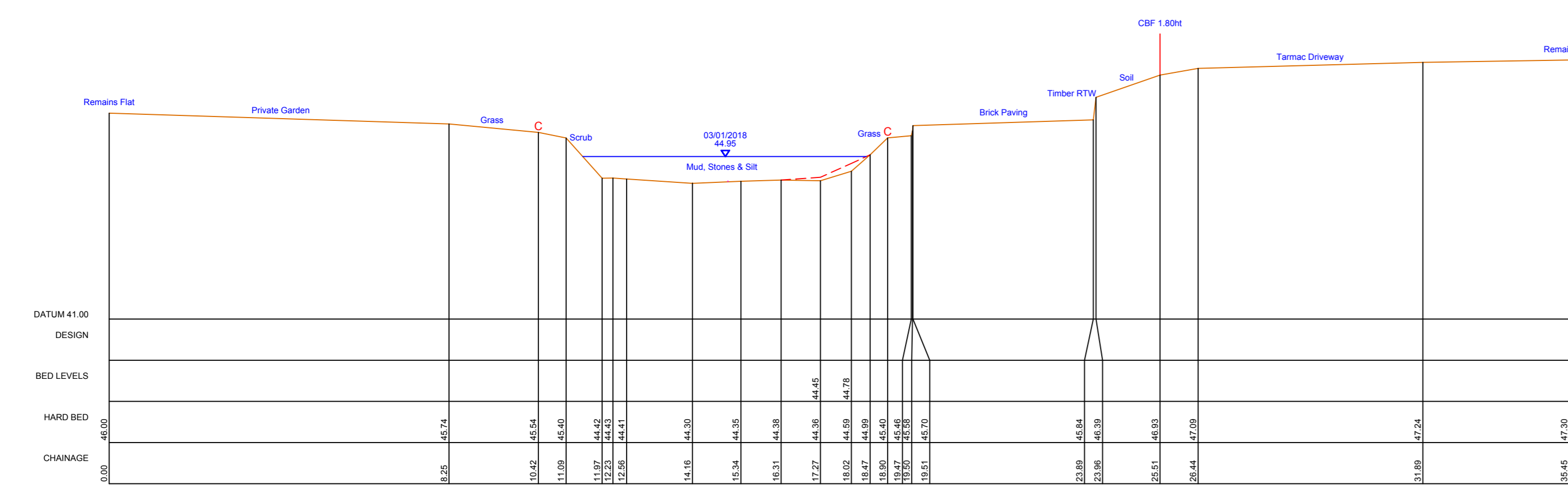
ESTO06_0216
606330.75mE 138029.61mN Brg 312
Open Channel



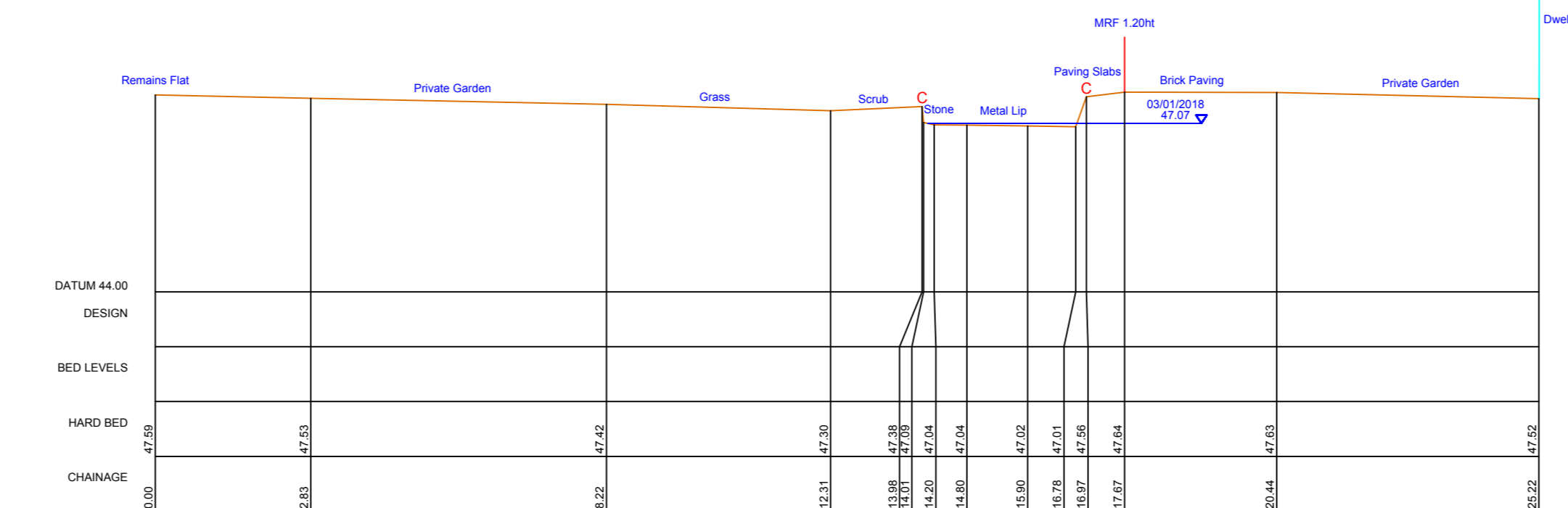
ESTO06_0229
606336.68mE 138031.05mN Brg 346
Station Road Bridge
Tunnel Length = 5.86m



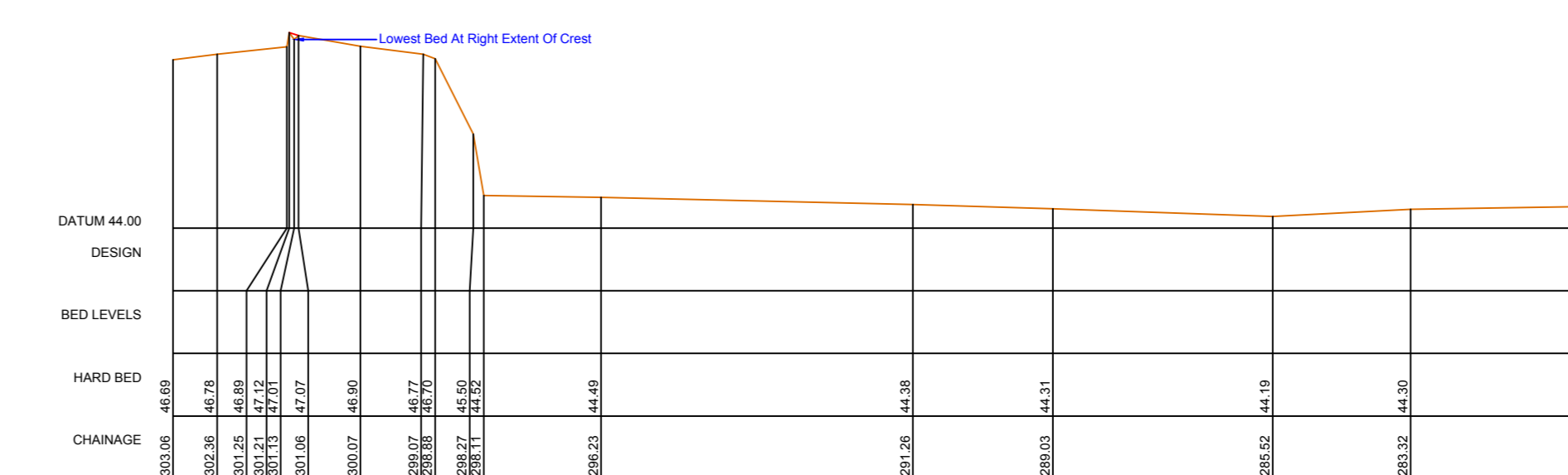
ESTO06_0242
606347.2mE 138047.84mN Brg 310
Open Channel



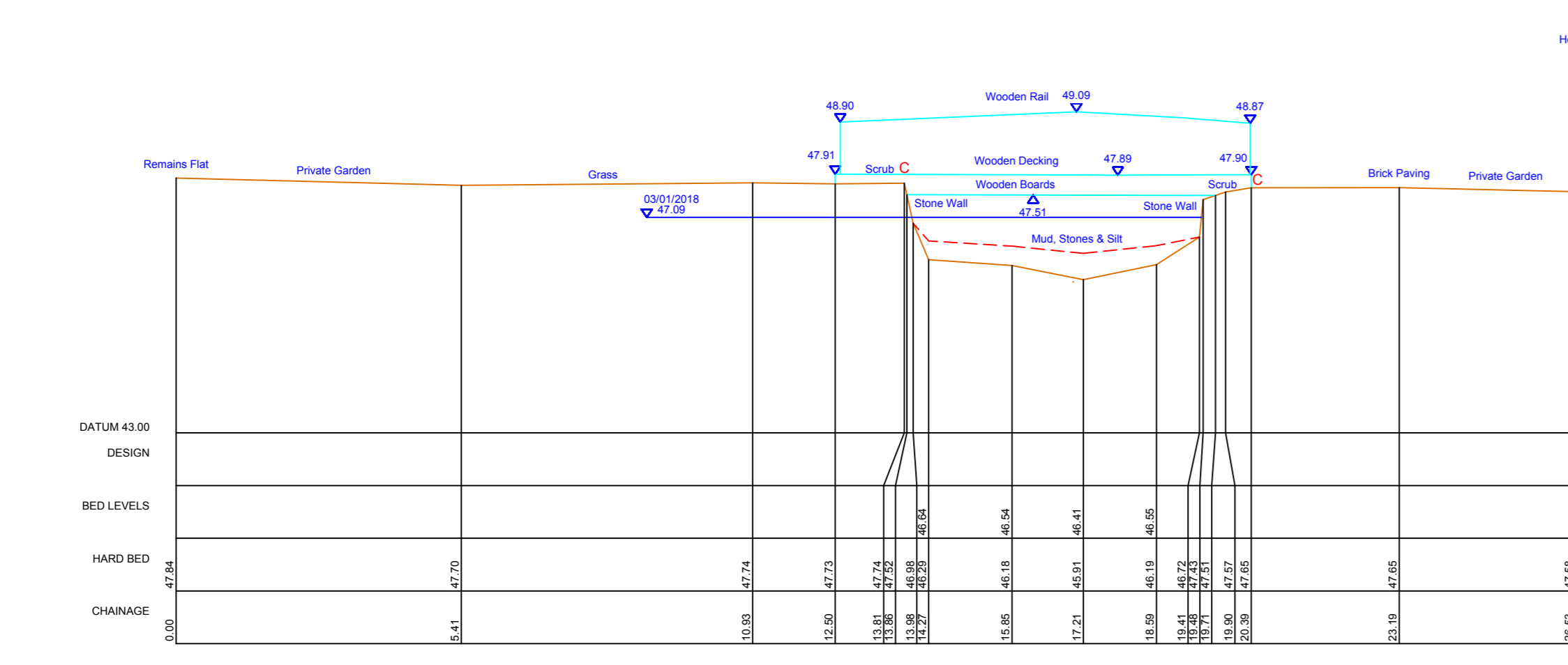
ESTO06_0280
606375.64mE 138067.88mN Brg 324
Open Channel



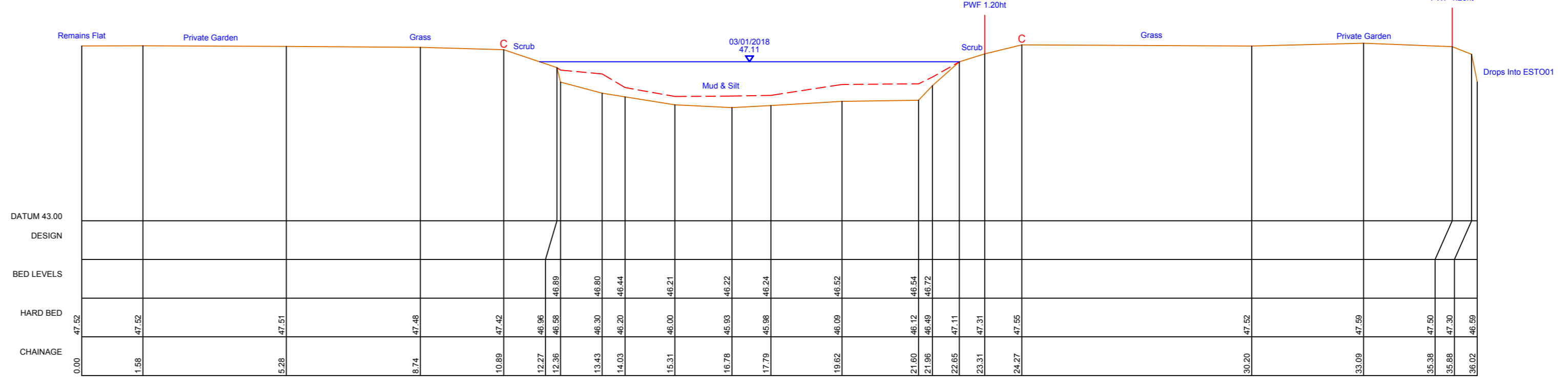
ESTO06_0301
606391.28mE 138081.53mN Brg 322
Disused Mill Wheel



ESTO06_0301
Disused Mill Wheel



ESTO06_0307
606394.92mE 138081.79mN Brg 331
Footbridge
Tunnel Length = 1.88m



ESTO06_0350
606433.86mE 138011.48mN Brg 328
Open Channel

NOTES:

1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
2. THIS MAP IS REPRODUCED FROM THE OS MAP BY THE ENVIRONMENT AGENCY WITH PERMISSION OF HER MAJESTY'S STATISTICAL OFFICE. A CROWN COPYRIGHT LICENCE. ALL RIGHTS RESERVED. UNAUTHORISED REPRODUCTION INFRINGES CROWN COPYRIGHT AND MAY LEAD TO PROSECUTION OR CIVIL PROCEEDINGS. LICENCE NO. 100026380.
3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

AP	AS BENCH	FB	THE CHANNEL	FW	THE CHANNEL
AW	WATER LEVEL	FW	THE CHANNEL	FW	THE CHANNEL
AW	WATER LEVEL	FW	THE CHANNEL	FW	THE CHANNEL
AW	WATER LEVEL	FW	THE CHANNEL	FW	THE CHANNEL
AW	WATER LEVEL	FW	THE CHANNEL	FW	THE CHANNEL

AMENDMENT

NO.	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E20730012	TR 0103 4107	35.975
E20730013	TR 0229 4627	36.480
E20730014	TR 0199 4202	36.480
E20730015	TR 0199 4202	36.480
E20730016	TR 0199 4202	36.480
E20730017	TR 0199 4202	36.480
E20730018	TR 0199 4202	36.480
E20730019	TR 0199 4202	36.480
E20730020	TR 0199 4202	36.480
E20730021	TR 0199 4202	36.480
E20730022	TR 0199 4202	36.480
E20730023	TR 0199 4202	36.480
E20730024	TR 0199 4202	36.480
E20730025	TR 0199 4202	36.480
E20730026	TR 0199 4202	36.480
E20730027	TR 0199 4202	36.480
E20730028	TR 0199 4202	36.480
E20730029	TR 0199 4202	36.480
E20730030	TR 0199 4202	36.480
E20730031	TR 0199 4202	36.480
E20730032	TR 0199 4202	36.480
E20730033	TR 0199 4202	36.480
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E20730039	TR 0199 4202	36.480
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E20730041	TR 0199 4202	36.480
E20730042	TR 0199 4202	36.480
E20730043	TR 0199 4202	36.480
E20730044	TR 0199 4202	36.480
E20730045	TR 0199 4202	36.480
E20730046	TR 0199 4202	36.480
E20730047	TR 0199 4202	36.480
E20730048	TR 0199 4202	36.480
E20730049	TR 0199 4202	36.480
E20730050	TR 0199 4202	36.480

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Ordnance Park, London Road, Addiscombe, Mool Valley, Kent, ME19 5QH

PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS:
EAST STOUR (ESTO06)
CROSS SECTIONS
ESTO06_0036 TO ESTO06_0350

SURVEYED BY: MALTBY LAND SURVEYS LTD Ref: T2_157
SURVEY DATE: JANUARY 2018

SCALE: 1:100 **DRN:** RP **CHKD:** ITS

DATUM: OS GPS ACTIVE **DATE:** JAN 18 **DATE:** MAR 18

GRID: NATIONAL GRID **DRAWING NO.:** X-J01058-36 **REV.:**

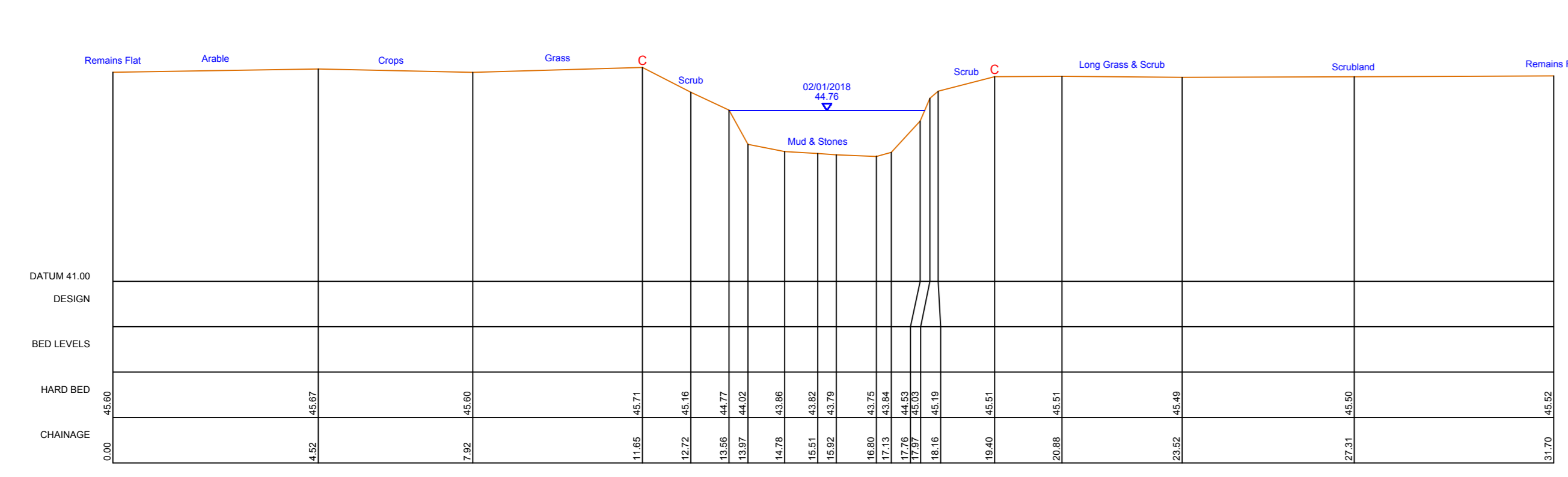
CDP FILENAME: X-21058-36.dwg

KEY TO SECTIONS:

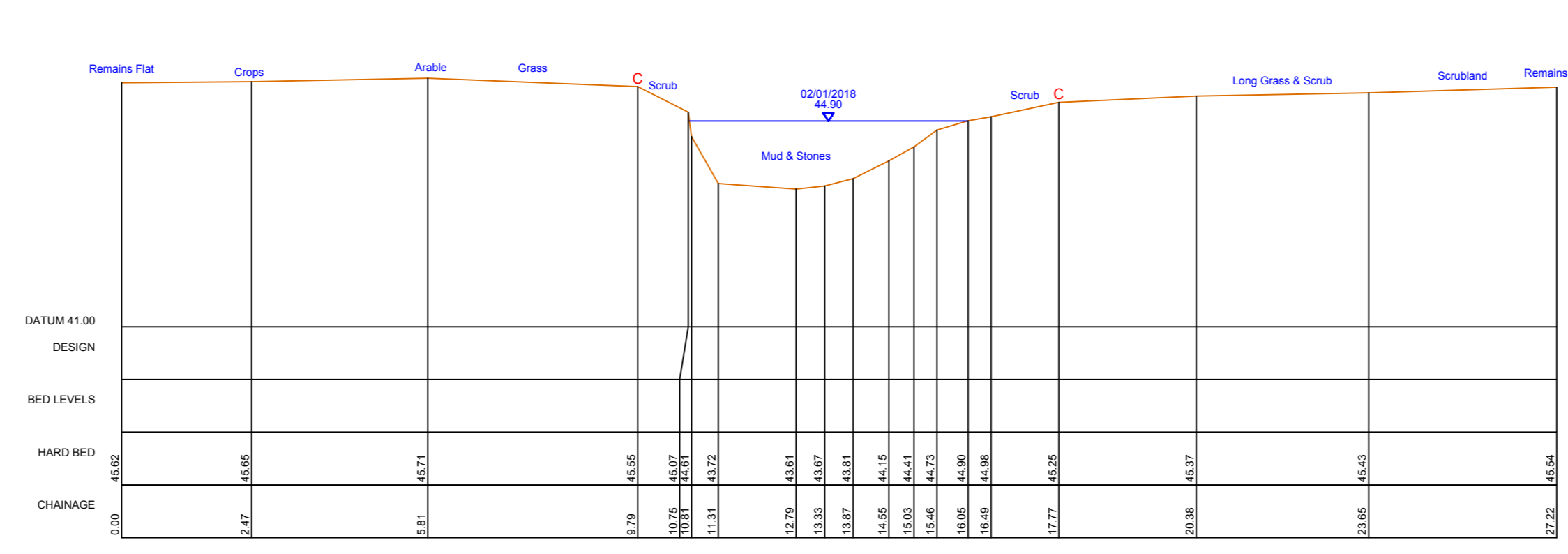
- WATER LEVEL
- VISIBLE BED (TOP OF SILT) AND GROUND
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

KEY TO LONGITUDINAL SECTION ONLY:

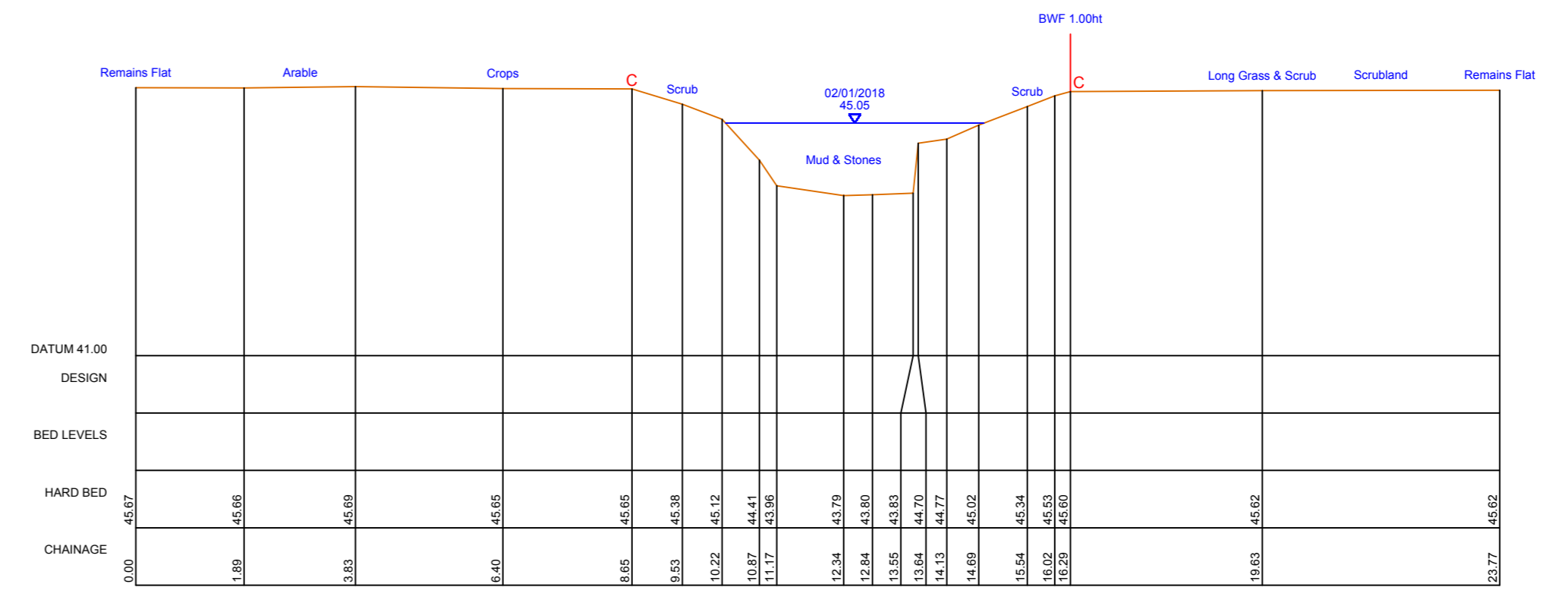
- VIEWED LOOKING DOWNSTREAM
- POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS
- LEFT BANK CREST
- RIGHT BANK CREST



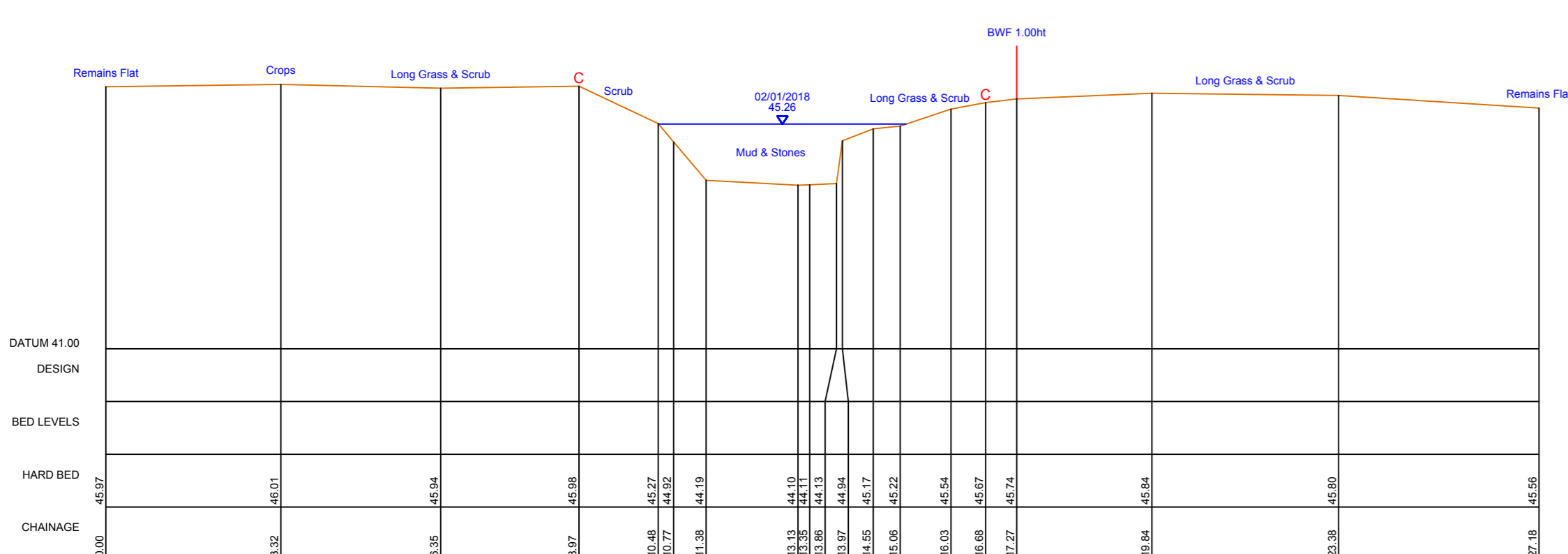
ESTO07_0034
606162.33mE 138064.92mN Brg 130
Open Channel



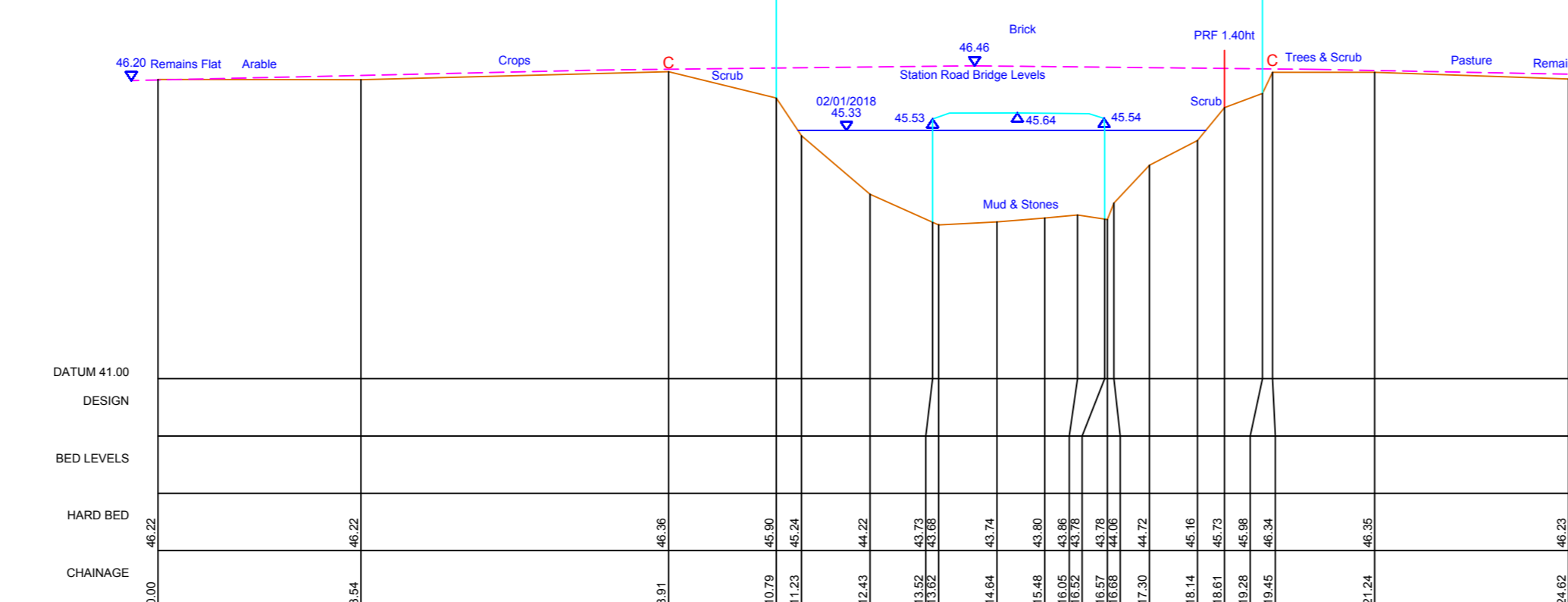
ESTO07_0098
606175.31mE 137997.45mN Brg 76
Open Channel



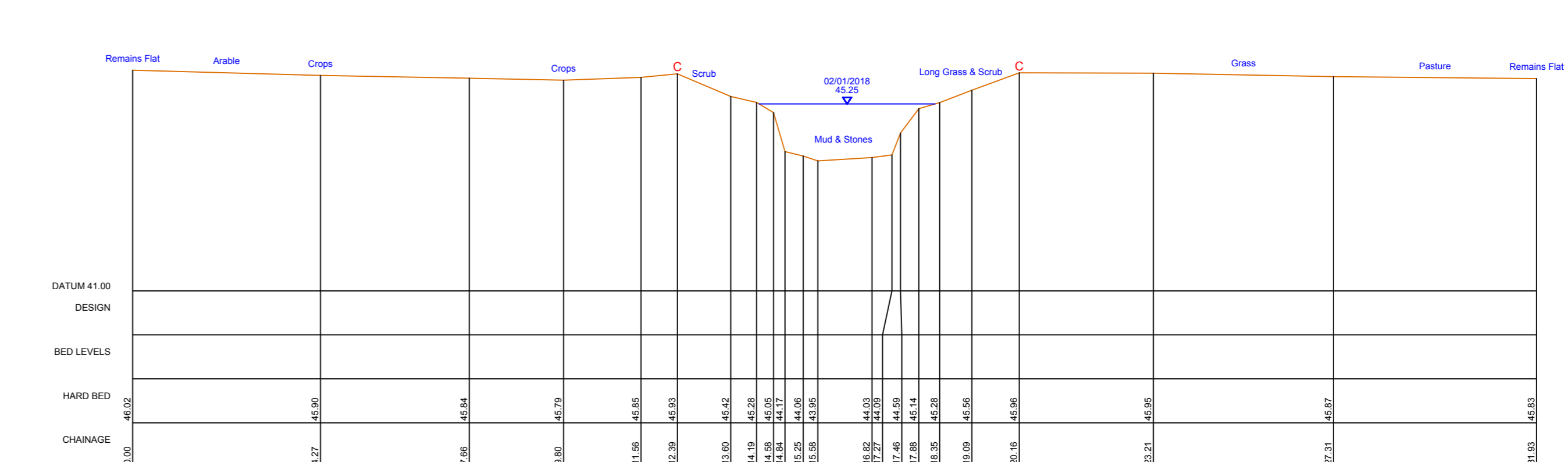
ESTO07_0190
606243.54mE 137967.63mN Brg 29
Open Channel



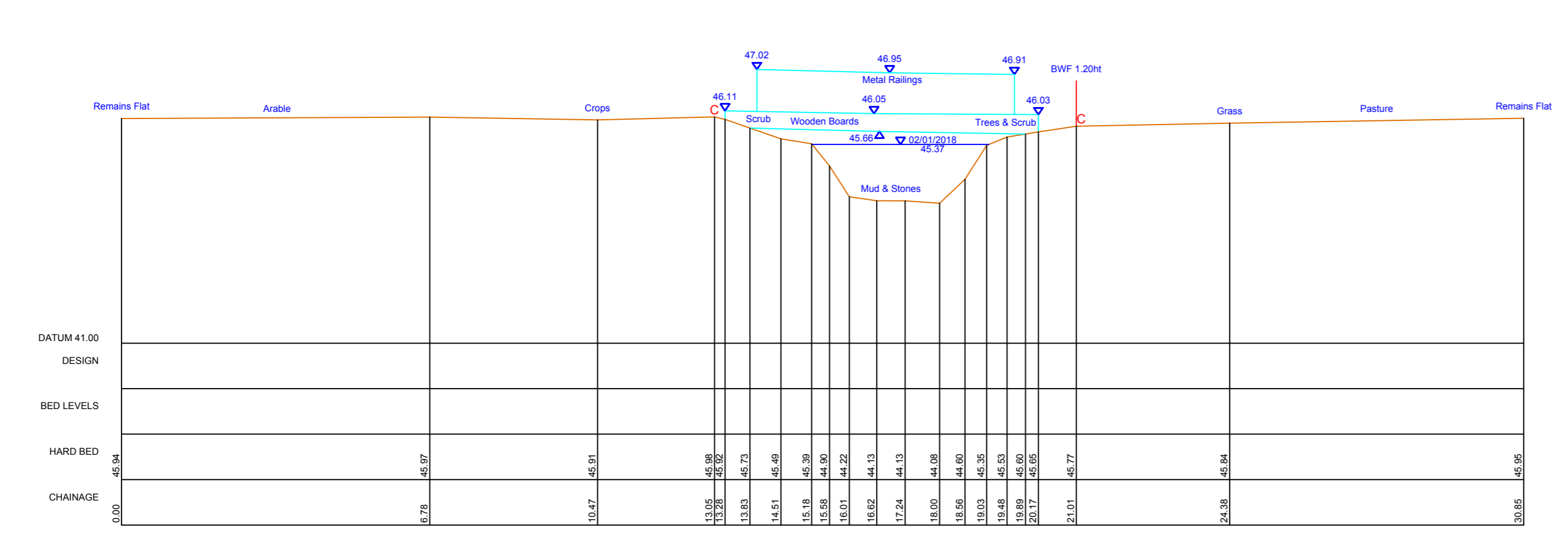
ESTO07_0321
606329.1mE 137881.68mN Brg 48
Open Channel



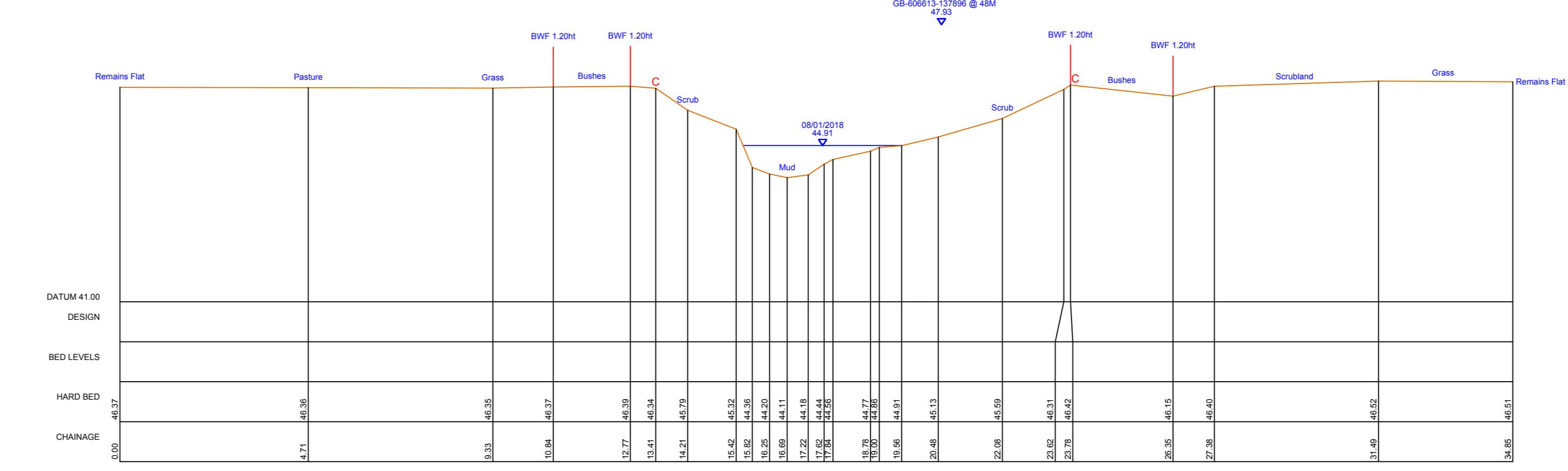
ESTO07_0354
606368.55mE 137877.88mN Brg 350
Station Road Bridge
Tunnel Length = 7.96m



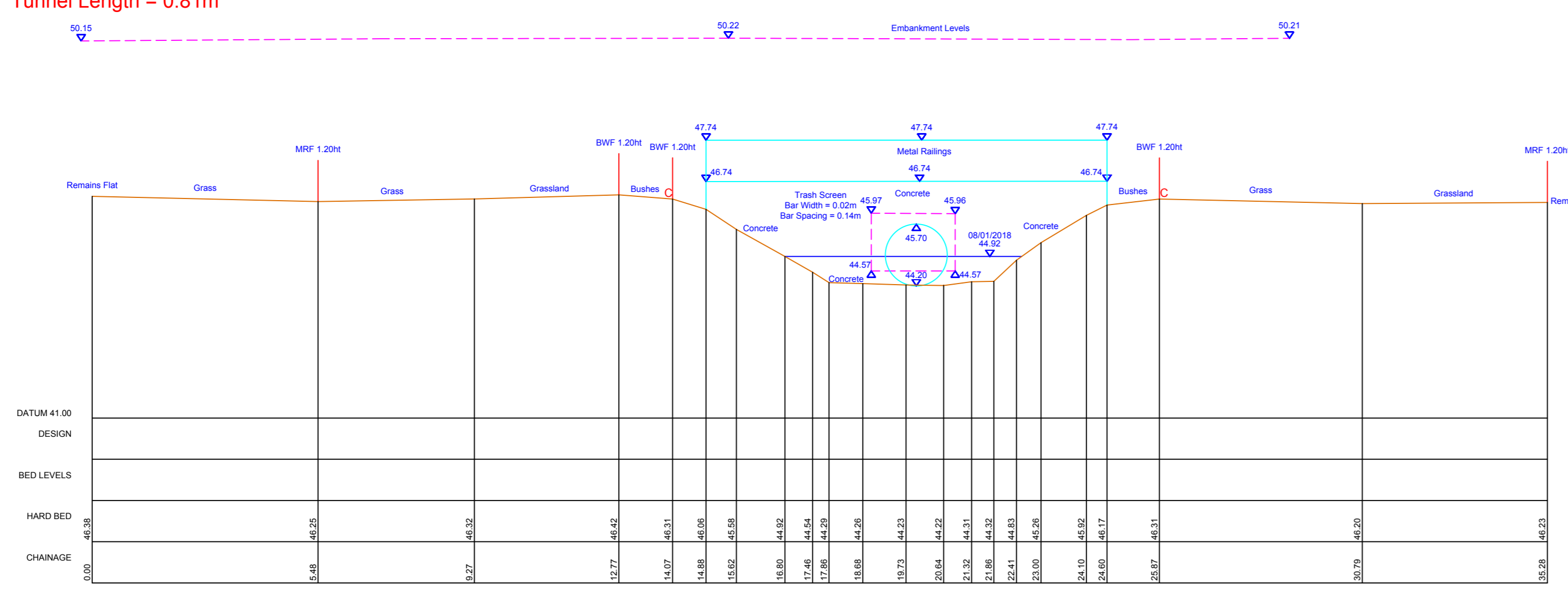
ESTO07_0378
606391.4mE 137884.07mN Brg 349
Open Channel



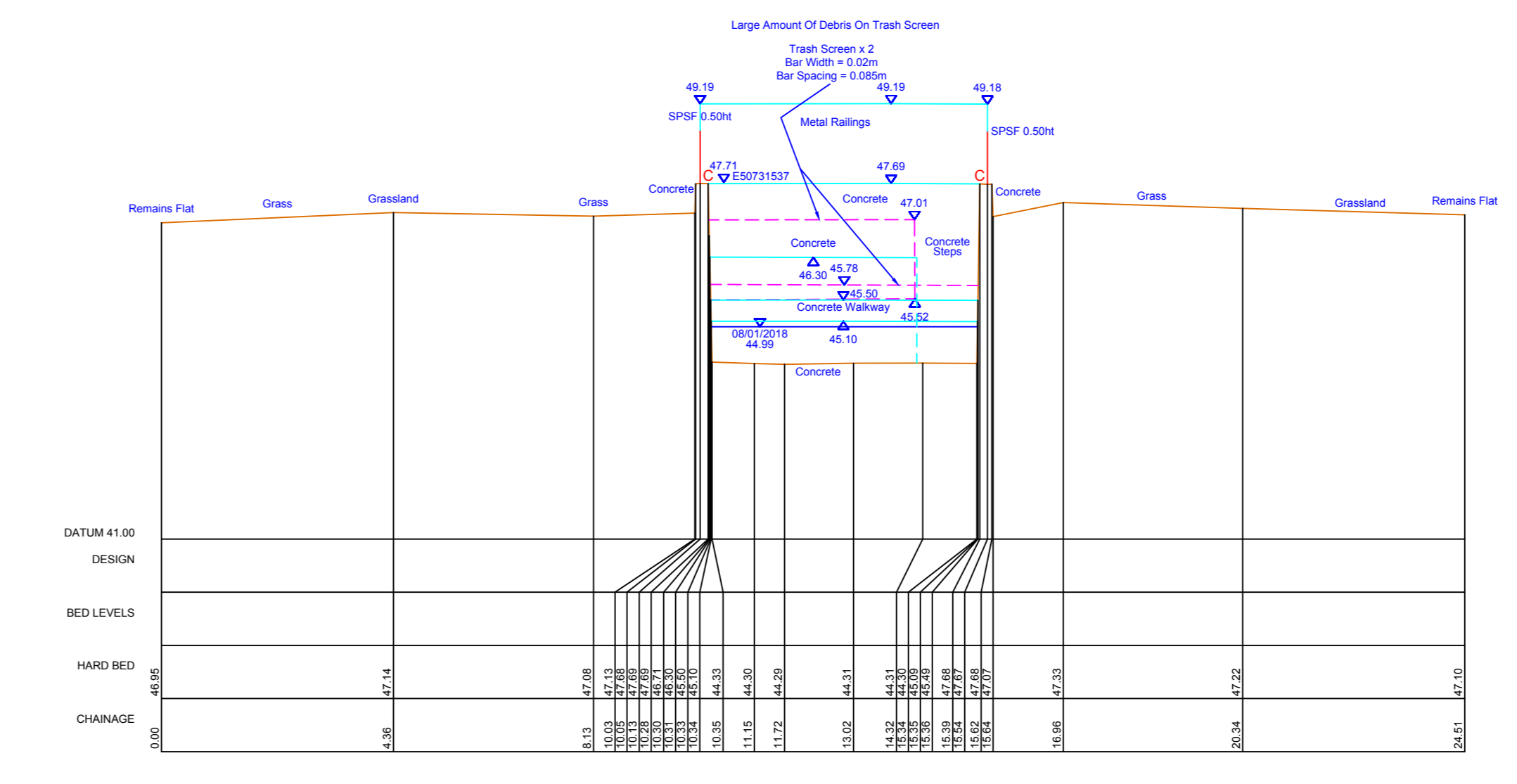
ESTO07_0534
606494.95mE 137822.45mN Brg 339
Footbridge
Tunnel Length = 0.81m



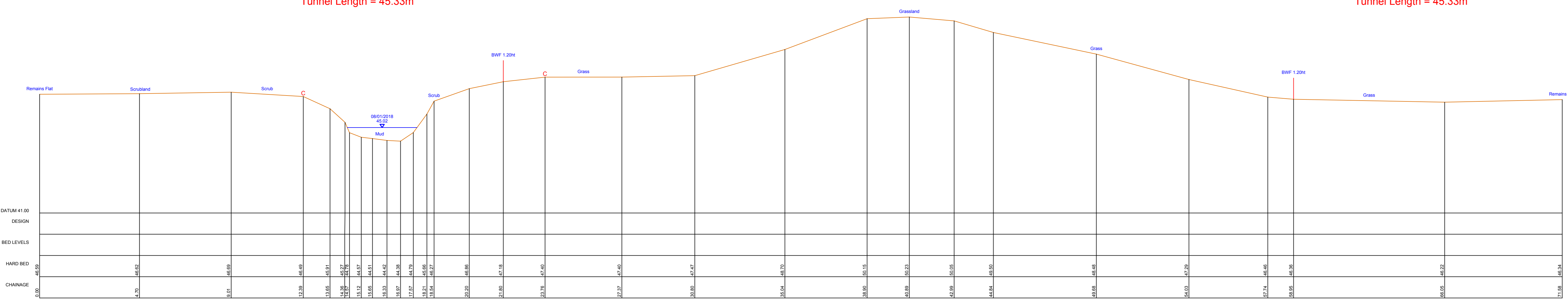
ESTO07_0665
606615.37mE 137874.41mN Brg 333
Open Channel



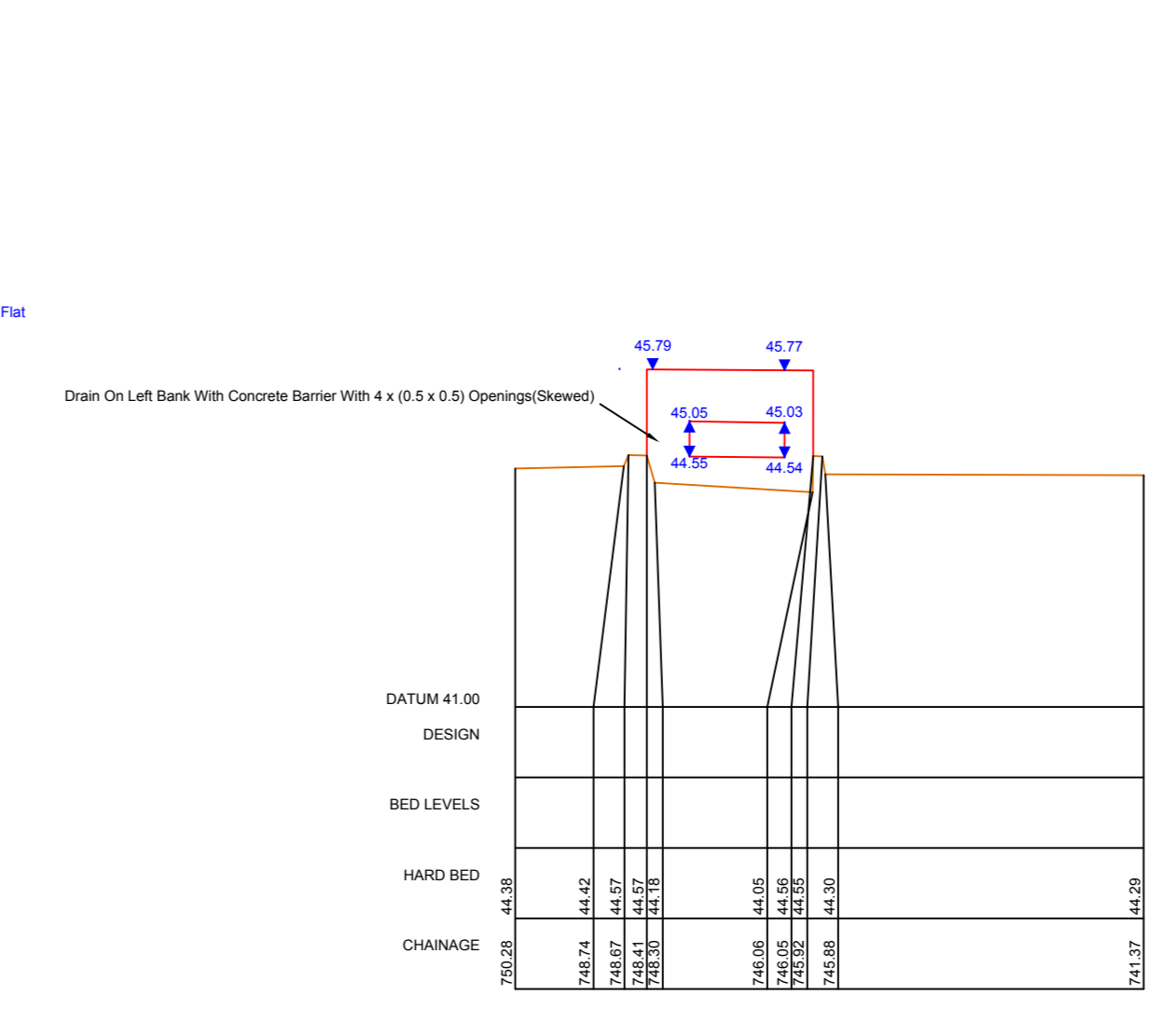
ESTO07_0696
606644.13mE 137884.64mN Brg 335
Culvert Exit
Tunnel Length = 45.33m



ESTO07_0741
606682.53mE 137908.65mN Brg 336
Culvert Entrance
Tunnel Length = 45.33m



ESTO07_0750
606695.33mE 137932.49mN Brg 248
Open Channel



Through Section ESTO07_0750
Open Channel

NOTES:

- A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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- UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

AS BRK	AS BRK	AS BRK	AS BRK
...

AMENDMENT

NO.	DESCRIPTION	DRN	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E07230012	TR 0103 4107	35.925
E07230013	TR 0103 4108	35.925
E07230014	TR 0103 4109	35.925
E07230015	TR 0103 4110	35.925
E07230016	TR 0103 4111	35.925
E07230017	TR 0103 4112	35.925
E07230018	TR 0103 4113	35.925
E07230019	TR 0103 4114	35.925
E07230020	TR 0103 4115	35.925
E07230021	TR 0103 4116	35.925
E07230022	TR 0103 4117	35.925
E07230023	TR 0103 4118	35.925
E07230024	TR 0103 4119	35.925
E07230025	TR 0103 4120	35.925
E07230026	TR 0103 4121	35.925
E07230027	TR 0103 4122	35.925
E07230028	TR 0103 4123	35.925
E07230029	TR 0103 4124	35.925
E07230030	TR 0103 4125	35.925
E07230031	TR 0103 4126	35.925
E07230032	TR 0103 4127	35.925
E07230033	TR 0103 4128	35.925
E07230034	TR 0103 4129	35.925
E07230035	TR 0103 4130	35.925
E07230036	TR 0103 4131	35.925
E07230037	TR 0103 4132	35.925
E07230038	TR 0103 4133	35.925
E07230039	TR 0103 4134	35.925
E07230040	TR 0103 4135	35.925
E07230041	TR 0103 4136	35.925
E07230042	TR 0103 4137	35.925
E07230043	TR 0103 4138	35.925
E07230044	TR 0103 4139	35.925
E07230045	TR 0103 4140	35.925
E07230046	TR 0103 4141	35.925
E07230047	TR 0103 4142	35.925
E07230048	TR 0103 4143	35.925
E07230049	TR 0103 4144	35.925
E07230050	TR 0103 4145	35.925

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Endersburg Park, London Road, Addiscombe, West Malling, Kent, ME19 5QH

PROJECT/WATERCOURSE:
EAST STOUR, ASHFORD TO STANFORD

SITE/VISITS:
EAST STOUR (ESTO07)
CROSS SECTIONS
ESTO07_0034 TO ESTO07_0750

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref: 12_157*
SURVEY DATE: JANUARY 2018

SCALE: 1:100 **DRN:** JB **CHKD:** ITS

DATUM: OS GPS ACTIVE **DATE:** JAN 18 **DATE:** MAR 18

GRID: NATIONAL GRID **DRAWING NO.:** X-J01058-37 **REV.:** 1

CAD FILENAME: X-2018-37-28.dwg

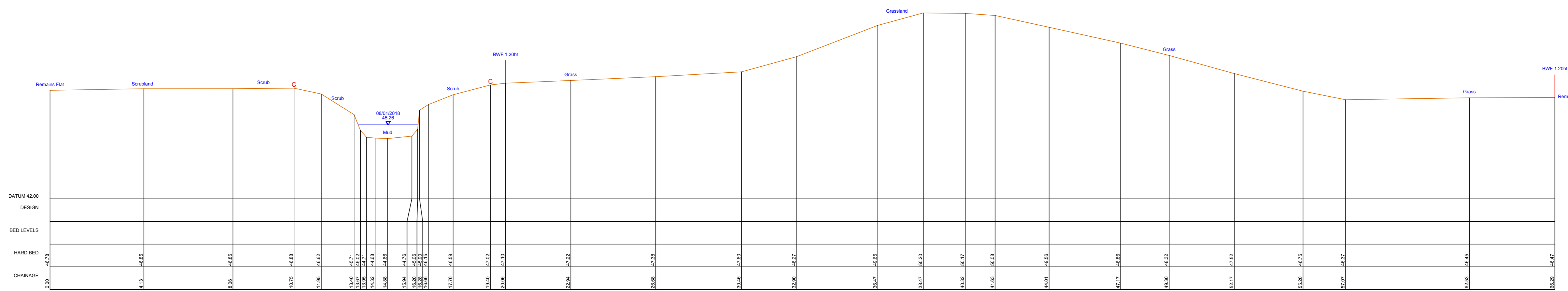
KEY TO SECTIONS:

- WATER LEVEL
- VISBLE BED (TOP OF SILT AND GROUND)
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

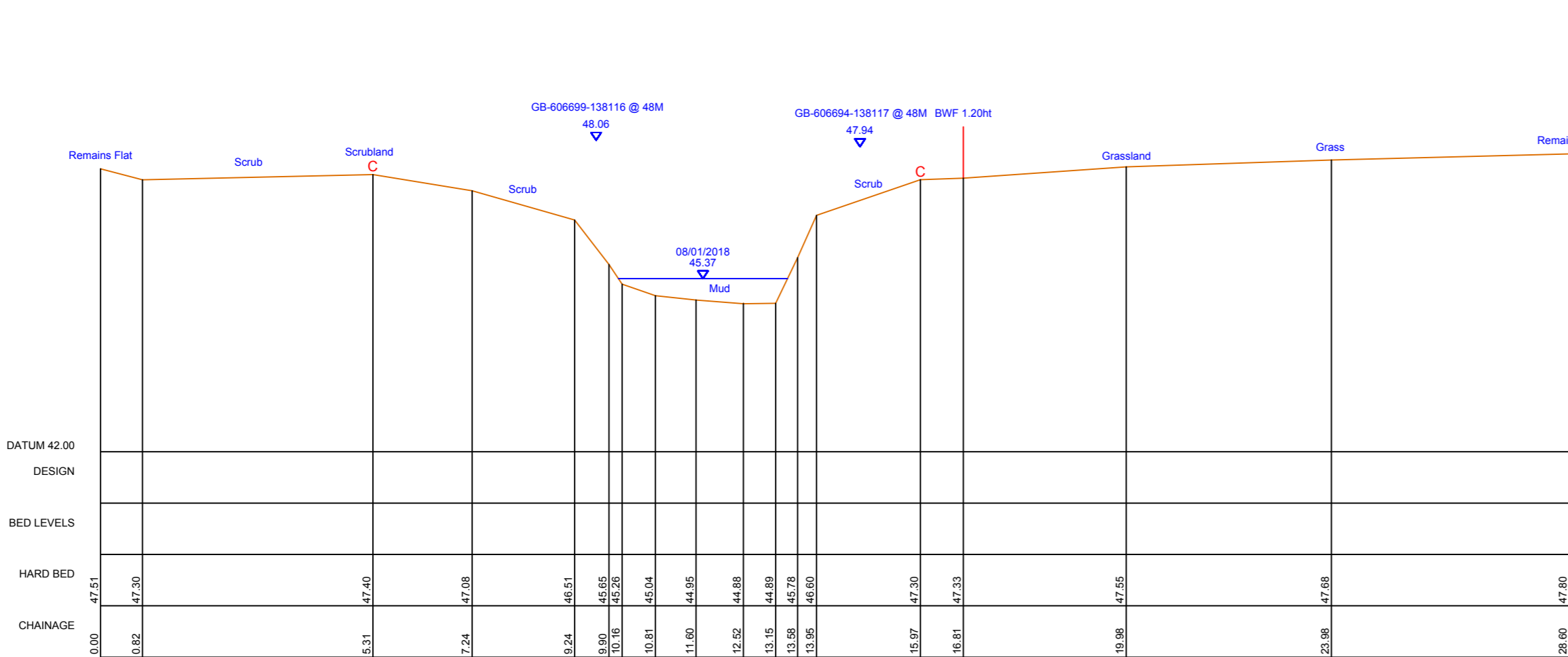
KEY TO LONGITUDINAL SECTION ONLY:

- LEFT BANK CREST
- RIGHT BANK CREST

POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



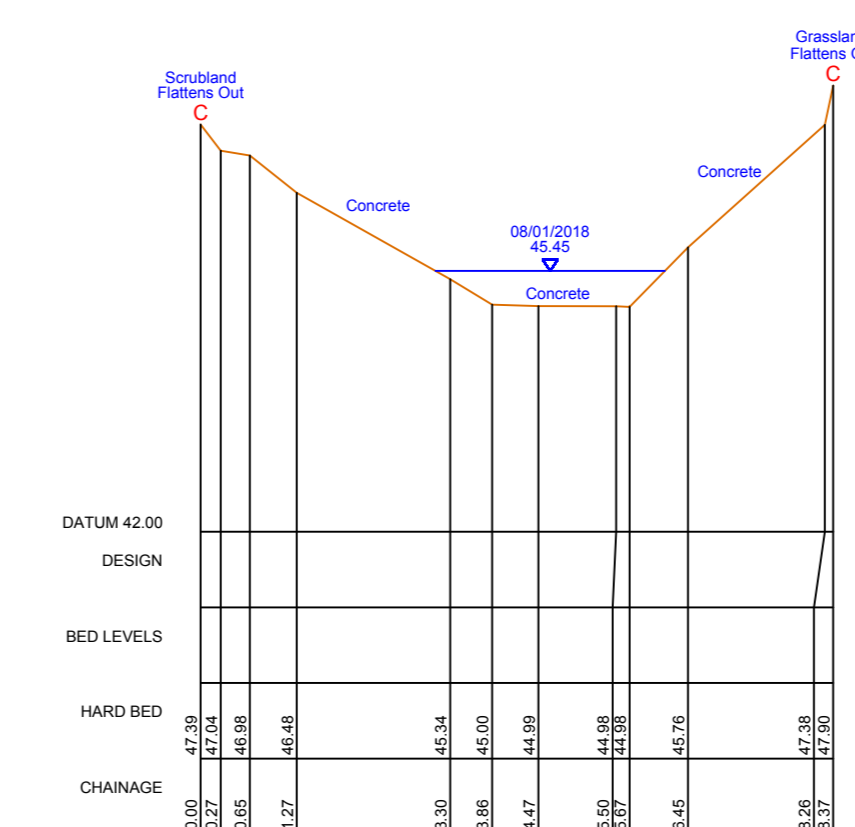
ESTO07_0855
606680.25mE 138028.69mN Brg 272
Open Channel



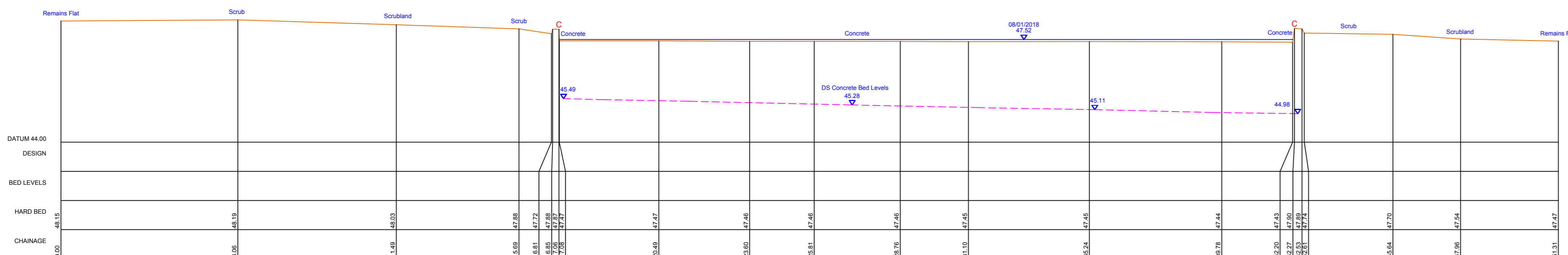
ESTO07_0957
606710.52mE 138115.8mN Brg 306
Open Channel

41.52
CR-606674-138124 @ 50M

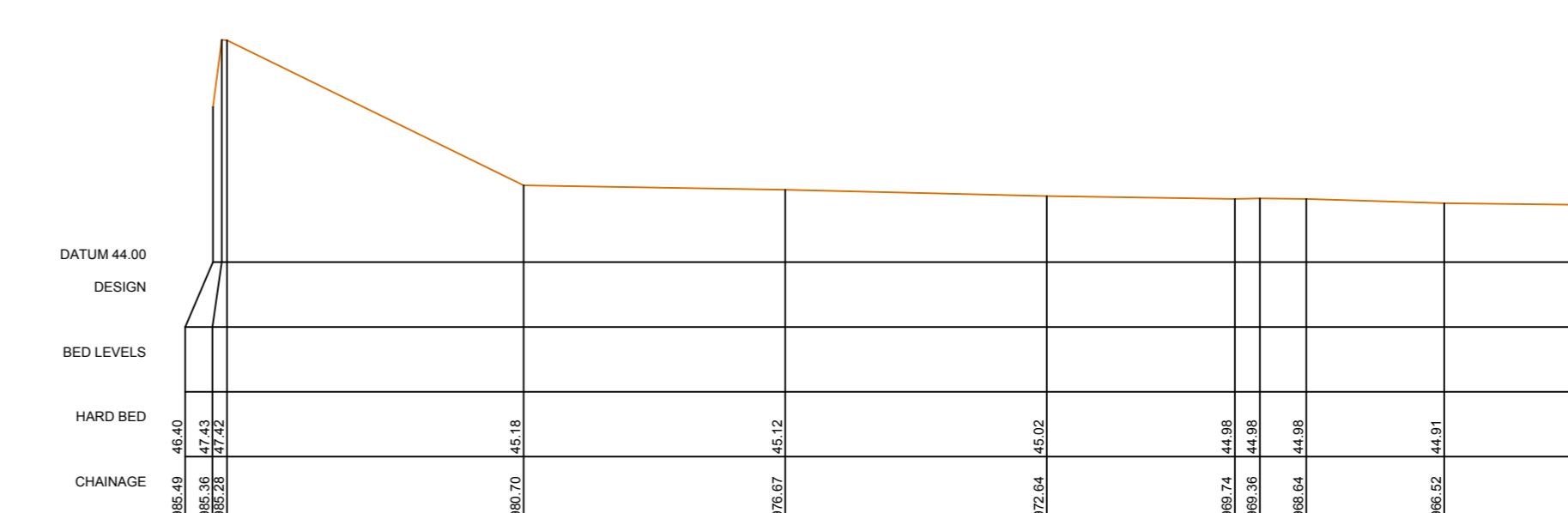
41.90
CR-606664-138125 @ 50M



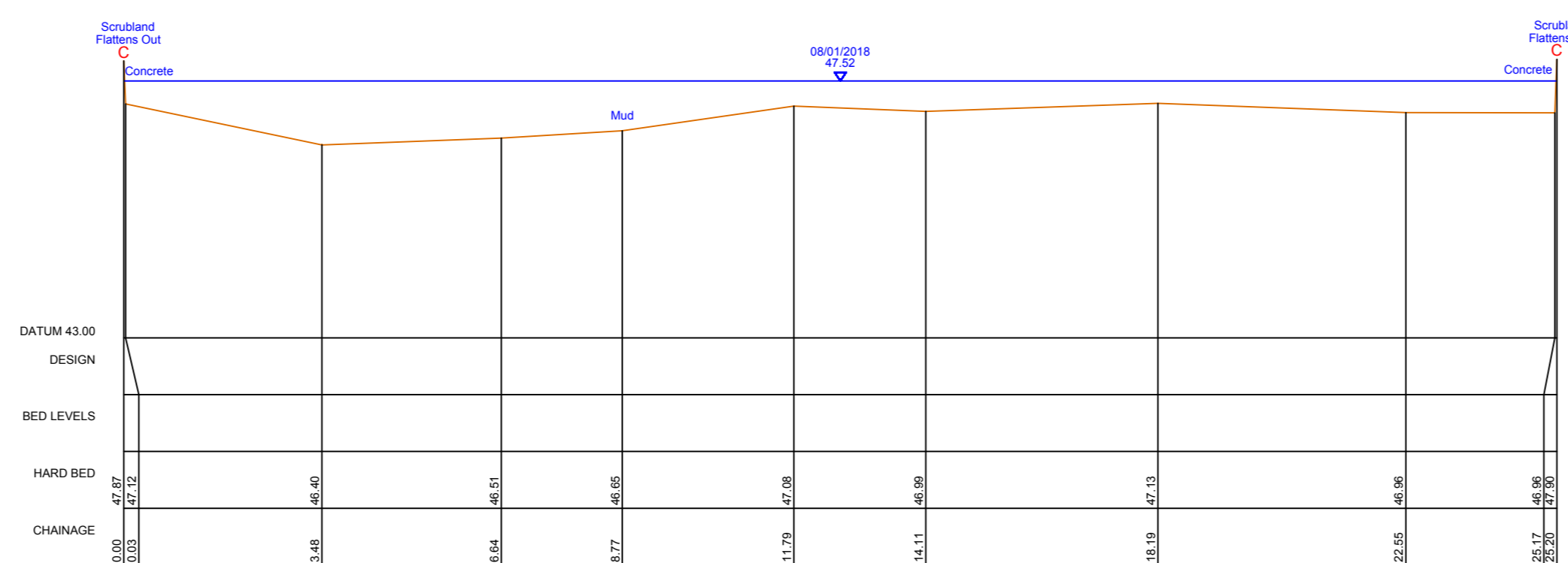
ESTO07_0970
606712.21mE 138128.89mN Brg 315
Weir Toe



ESTO07_0985
606736.02mE 138164.9mN Brg 225
Weir Crest



Through Section ESTO07_0985
Weir Crest



ESTO07_0986
606723.87mE 138152.92mN Brg 225
Weir Heel

NOTES:

1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
Blue line	WATER LEVEL	Red dashed line	VISBLE BED (TOP OF SILT AND GROUND)
Orange line	HARD BED (DETERMINED BY PROBING)	C	BANK CREST
Green dashed line	LEFT BANK CREST	Red dashed line	RIGHT BANK CREST

AMENDMENT

NO.	DESCRIPTION	DRN	CHWD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E-0730012	TR 0103 4107	35.975
E-0730403	TR 0229 4277	38.480
E-0730404	TR 0229 4277	38.480
E-0730019	TR 0109 4202	38.480
E-0730020	TR 0109 4202	38.480
E-0730021	TR 0109 4202	38.480
E-0730022	TR 0109 4202	38.480
E-0730023	TR 0109 4202	38.480
E-0730024	TR 0109 4202	38.480
E-0730025	TR 0109 4202	38.480
E-0730026	TR 0109 4202	38.480
E-0730027	TR 0109 4202	38.480
E-0730028	TR 0109 4202	38.480
E-0730029	TR 0109 4202	38.480
E-0730030	TR 0109 4202	38.480
E-0730031	TR 0109 4202	38.480
E-0730032	TR 0109 4202	38.480
E-0730033	TR 0109 4202	38.480
E-0730034	TR 0109 4202	38.480
E-0730035	TR 0109 4202	38.480
E-0730036	TR 0109 4202	38.480
E-0730037	TR 0109 4202	38.480
E-0730038	TR 0109 4202	38.480
E-0730039	TR 0109 4202	38.480
E-0730040	TR 0109 4202	38.480

Environment Agency
KENT & SOUTH LONDON REGION

Ordnance Survey, Ordnance Survey, London Road, Addlestone, West Midlands, Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
EAST STOUR (ESTO07)
CROSS SECTIONS
ESTO07_0855 TO ESTO07_0986

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref: 12_157*
SURVEY DATE: DECEMBER 2017

SCALE: 1:100 | **DRN:** JB | **CHKD:** ITS
DATUM: OS GPS ACTIVE | **DATE:** JAN 18 | **DATE:** MAR 18

GRID: NATIONAL GRID | **DRAWING NO.:** X-J01058-38 | **REV.:** 1

GRID FILENAME: F-20108-37-28.dwg

SCALE = 1:100 H, 1:100 V

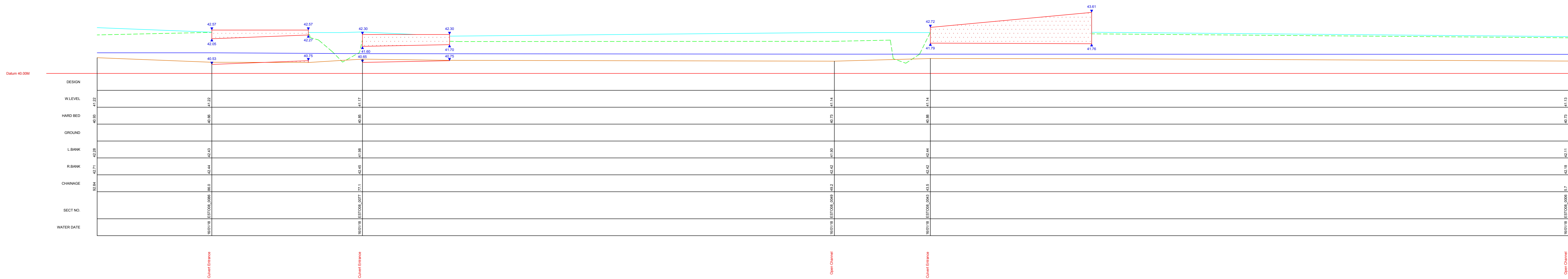
Original Drawing Size: A0

KEY TO SECTIONS:

- WATER LEVEL
- VISIBLE BED (TOP OF SILT) AND GROUND
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

KEY TO LONGITUDINAL SECTION ONLY:

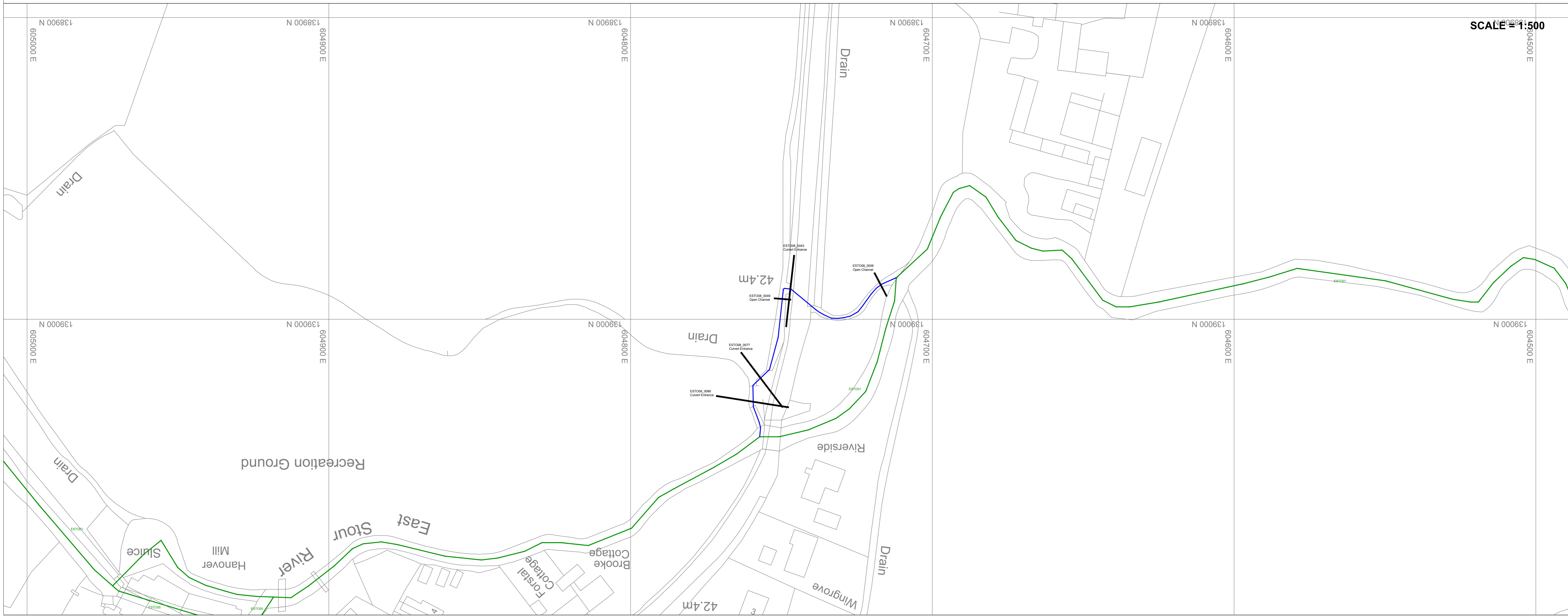
- LEFT BANK CREST
- RIGHT BANK CREST
- GAUGE BOARD
- POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS



LOCATION PLAN ORIENTATION



SCALE = 1:500



NOTES:

1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
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3. UNLESS OTHERWISE STATED ALL SECTIONS ARE VIEWED DOWNSTREAM.

SURVEY LEGEND

SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
[Symbol]	Water Level	[Symbol]	Right Bank Crest
[Symbol]	Visible Bed	[Symbol]	Points Indicated by 'C'
[Symbol]	Hard Bed	[Symbol]	Cross Sections
[Symbol]	Bank Crest	[Symbol]	Gauge Board

AMN	DESCRIPTION	DRN	CHD	DATE

CONTROL USED:

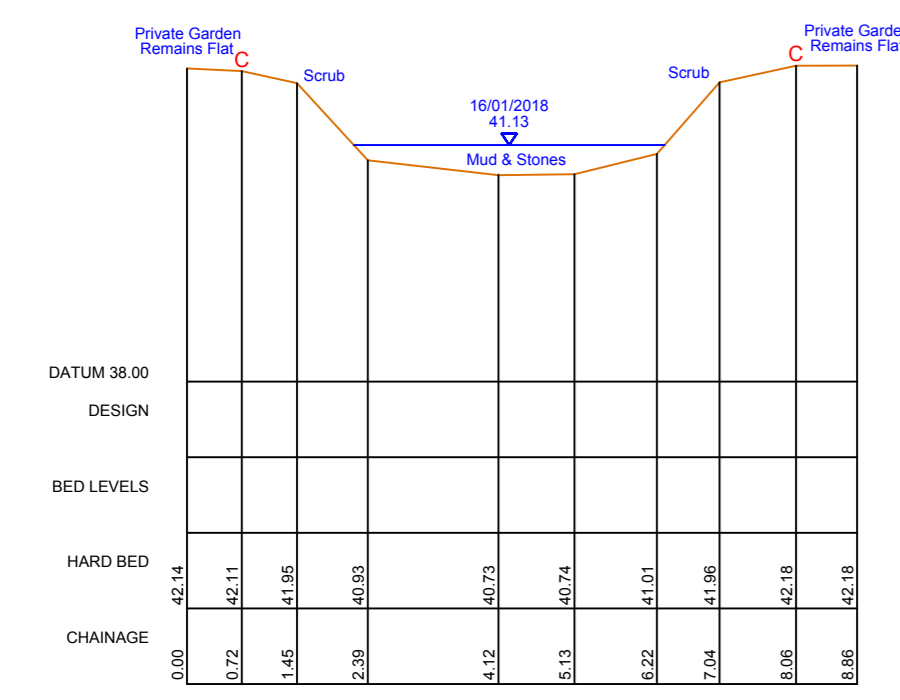
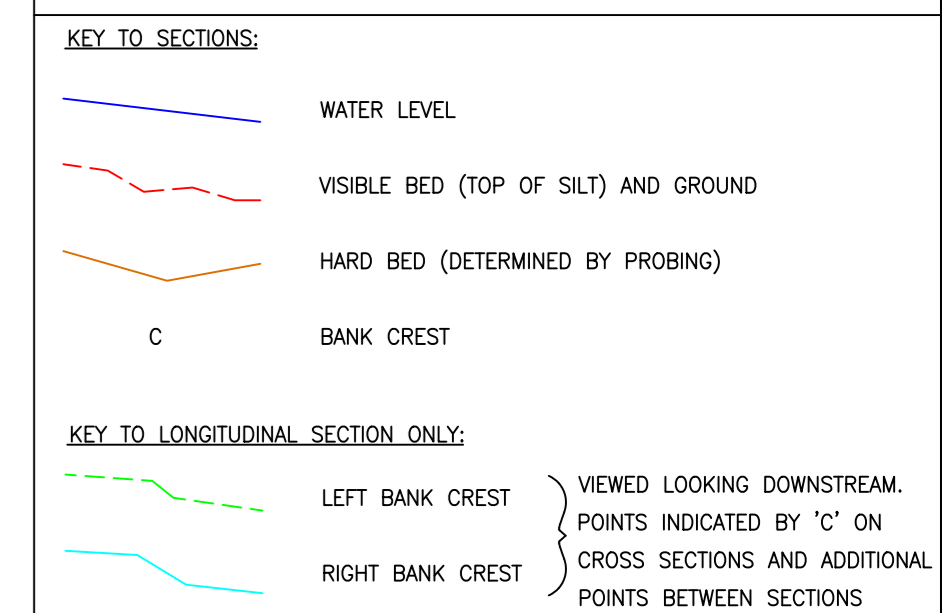
TYPE	DESCRIPTION	LEVEL
TR	0103 4107	35.925
TR	0223 4227	38.489
TR	0223 4227	38.489
TR	0192 4202	38.489
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TR	0192 4202	38.489
TR	0192 4202	38.489
TR	0192 4202	38.489
TR	0192 4202	38.489
TR	0192 4202	38.489
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TR	0192 4202	38.489
TR	0192 4202	38.489
TR	0192 4202	38.489
TR	0192 4202	38.489
TR	0192 4202	38.489
TR	0192 4202	38.489

Environment Agency
 KENT & SOUTH LONDON REGION
 Orchard House, Endeavour Park, London Road, Ashford, Kent, ME19 5DH

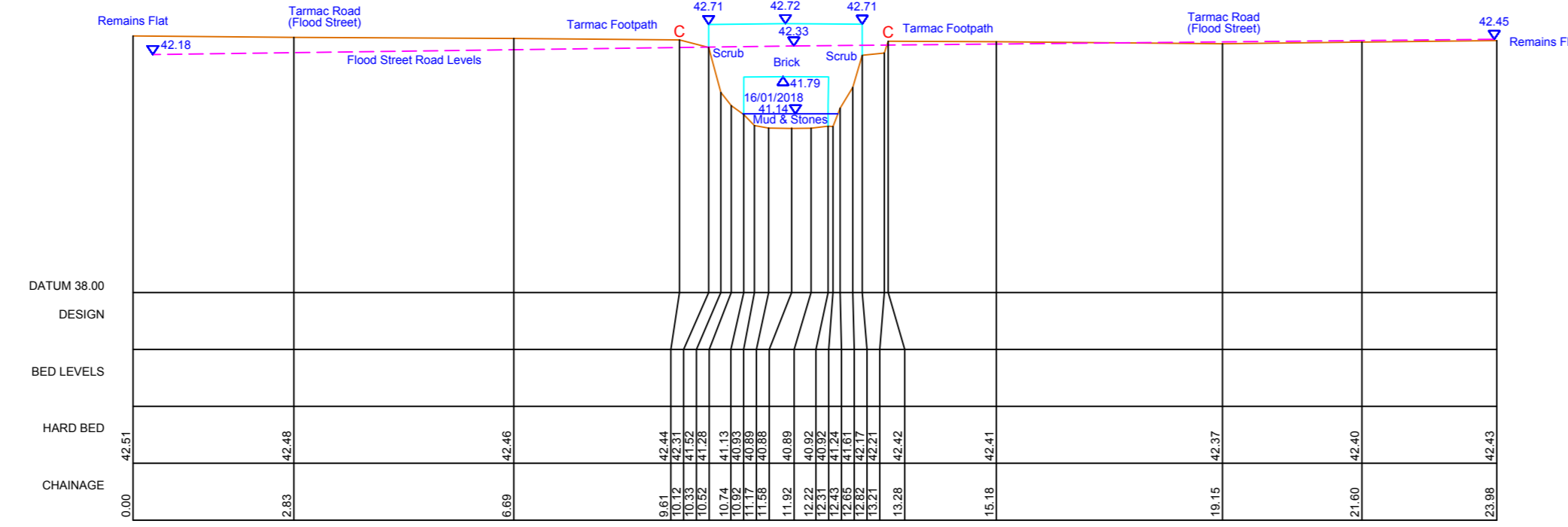
PROJECT/WATERCOURSE
 EAST STOUR, ASHFORD TO STANFORD

SITE/UMTS
 EAST STOUR (EST008)
 LONG SECTION & LOCATION PLAN
 EST008_0006 TO EST008_0008

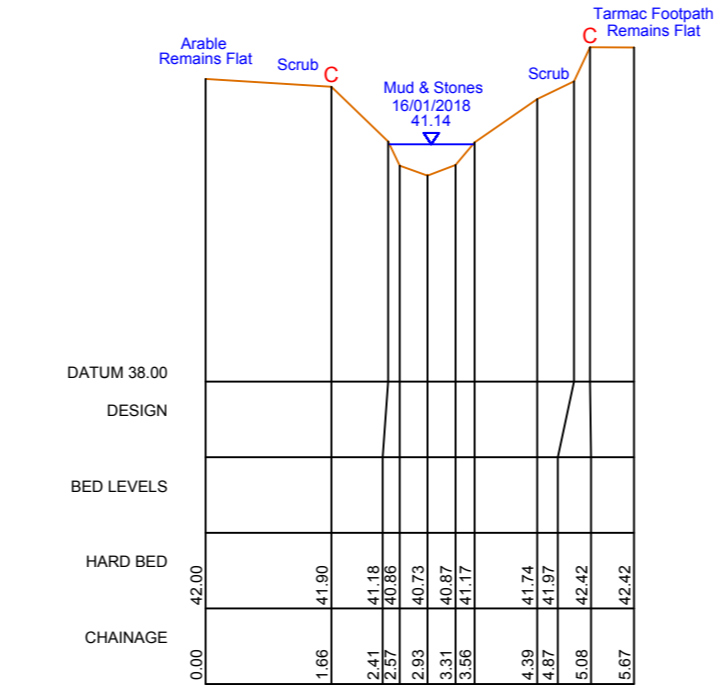
SURVEYED BY: MALTBY LAND SURVEYS LTD		Rev: 12_150	
SURVEY DATE: JANUARY 2018			
SCALE: AS SHOWN	DRN: RC	CHKD: ITS	
DATUM: OS GPS ACTIVE	DATE: JAN 18	DATE: MAR 18	
GRID: NATIONAL GRID	DRAWING NO:		REV:
	L-J01058-15		



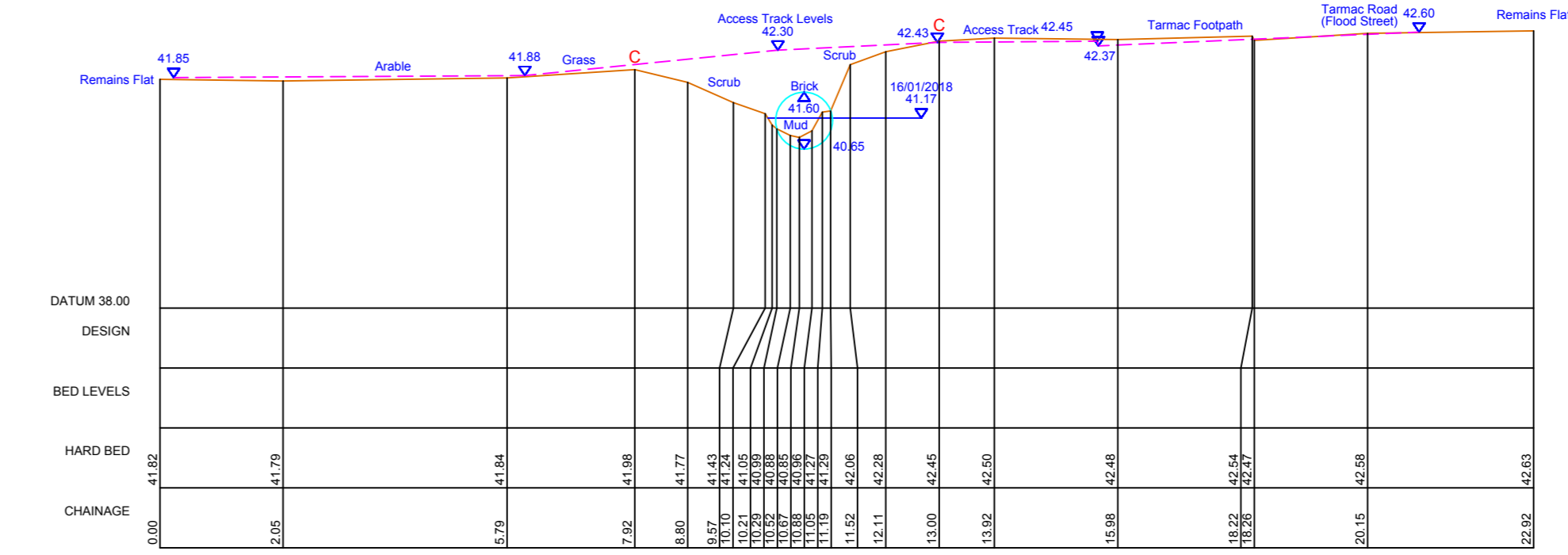
ESTO08_0006
604719.21mE 138984.56mN Brg 332
Open Channel



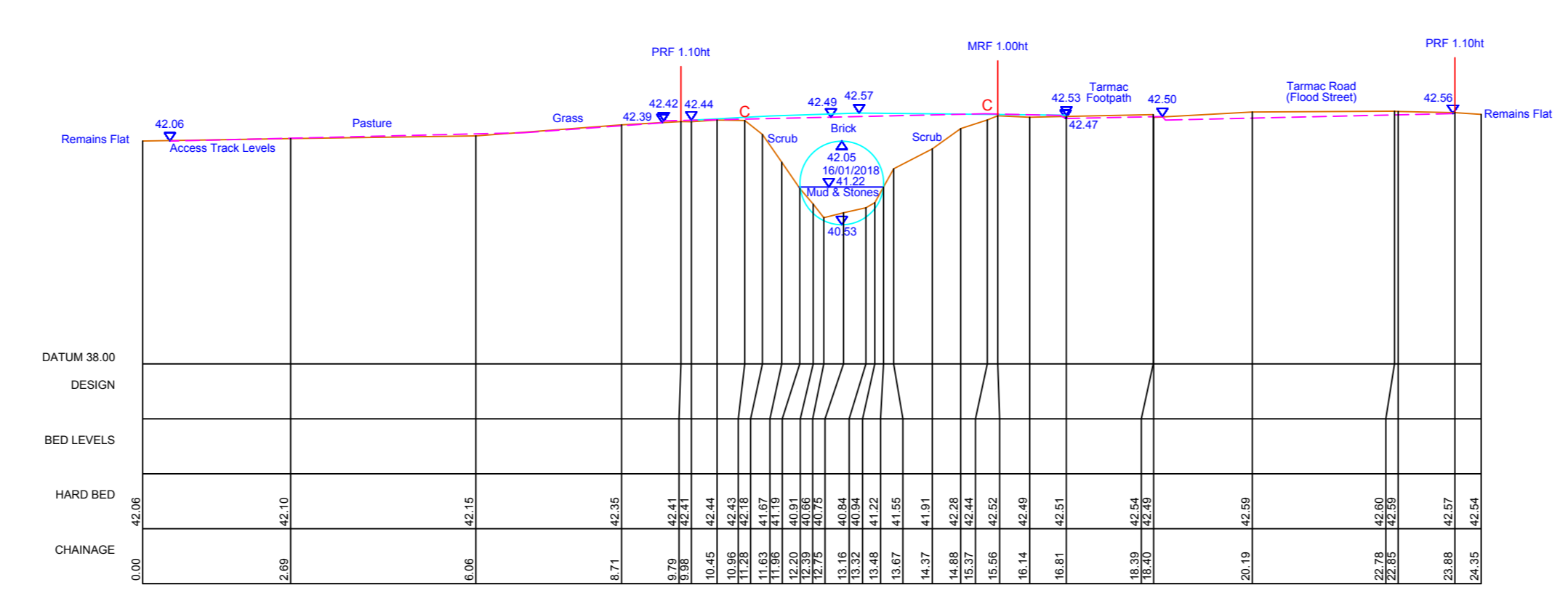
ESTO08_0043
604745.77mE 138978.73mN Brg 6
Culvert Entrance
Tunnel Length = 9.55m



ESTO08_0049
604752.47mE 138993.05mN Brg 275
Open Channel



ESTO08_0077
604763.41mE 139010.95mN Brg 323
Culvert Entrance
Tunnel Length = 5.16m



ESTO08_0086
604771.63mE 139025.41mN Brg 279
Culvert Entrance
Tunnel Length = 5.72m

NOTES:

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SURVEY LEGEND

SYMBOL	DESCRIPTION	SYMBOL	DESCRIPTION
(Symbol)	AS BENCH	(Symbol)	SPRINT WALK
(Symbol)	AV MARK	(Symbol)	TEMP MARK
(Symbol)	BE BENCH	(Symbol)	TOPTOP
(Symbol)	BU BENCH	(Symbol)	UNIDENTIFIED
(Symbol)	CL	(Symbol)	...

AMENDMENT	DRN	CHD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
E50730012	TR 0103 4107	35.975
E50730403	TR 0229 4227	36.489
E50730404	TR 0229 4227	36.489
E50730415	TR 0199 4202	36.489
E50730501	TR 0199 4202	36.489
E50730502	TR 0199 4202	36.489
E50730503	TR 0199 4202	36.489
E50730504	TR 0126 4199	37.237
E50730505	TR 0543 3936	40.289
E50730506	TR 0543 3936	40.289
E50730507	TR 0543 3936	40.289
E50730508	TR 0543 3936	40.289
E50730509	TR 0543 3936	40.289
E50730510	TR 0543 3936	40.289
E50730511	TR 0543 3936	40.289
E50730512	TR 0543 3936	40.289
E50730513	TR 0543 3936	40.289
E50730514	TR 0543 3936	40.289
E50730515	TR 0543 3936	40.289
E50730516	TR 0543 3936	40.289
E50730517	TR 0543 3936	40.289
E50730518	TR 0543 3936	40.289
E50730519	TR 0543 3936	40.289
E50730520	TR 0543 3936	40.289
E50730521	TR 0543 3936	40.289
E50730522	TR 0543 3936	40.289
E50730523	TR 0543 3936	40.289
E50730524	TR 0543 3936	40.289
E50730525	TR 0543 3936	40.289
E50730526	TR 0543 3936	40.289
E50730527	TR 0543 3936	40.289
E50730528	TR 0543 3936	40.289
E50730529	TR 0543 3936	40.289
E50730530	TR 0543 3936	40.289

Environment Agency
KENT & SOUTH LONDON REGION
Orchard House, Endeavour Park, London Road, Ashford, Kent, ME19 5PH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

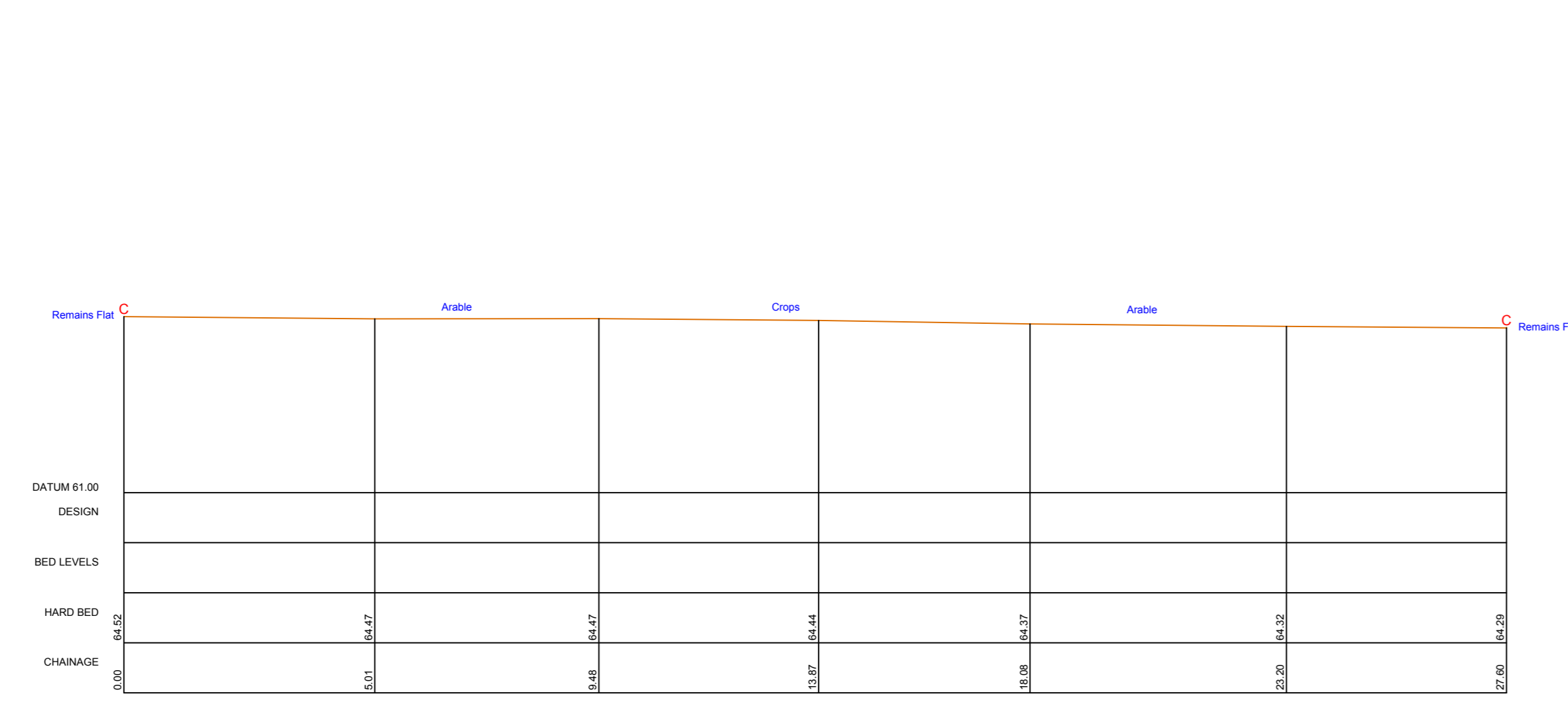
SITE/UMPS
**EAST STOUR (EST008)
CROSS SECTIONS
EST008_0006 TO EST008_0086**

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref 12_137*
SURVEY DATE: JANUARY 2018

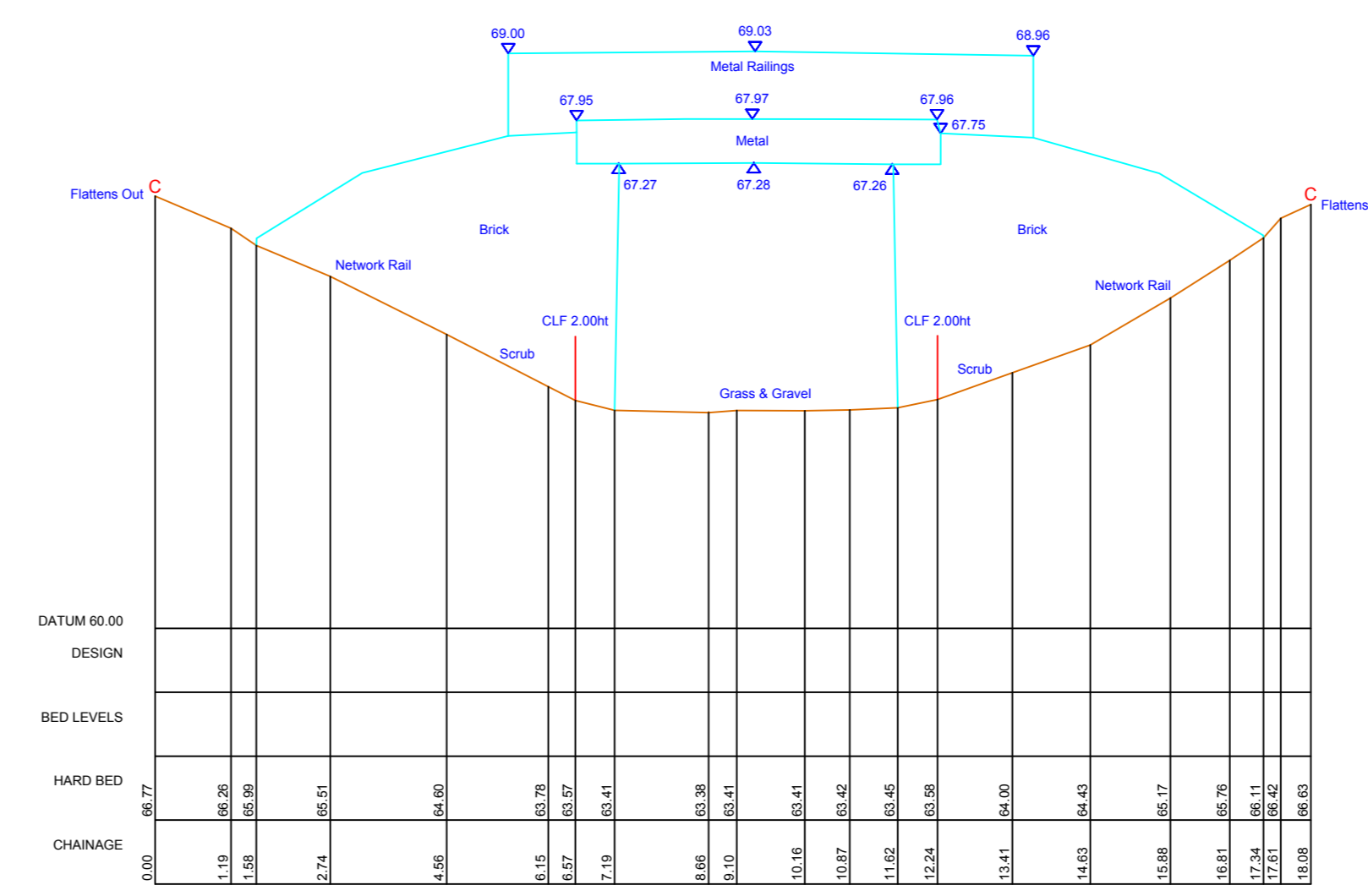
SCALE: 1:100 | DRN: RC | CHKD: ITS
DATUM: OS GPS ACTIVE | DATE: JAN 18 | DATE: MAR 18

GRID: NATIONAL GRID | DRAWING NO: | REV. |
DWG FILENAME: E-2018-39.dwg | **X-J01058-39**

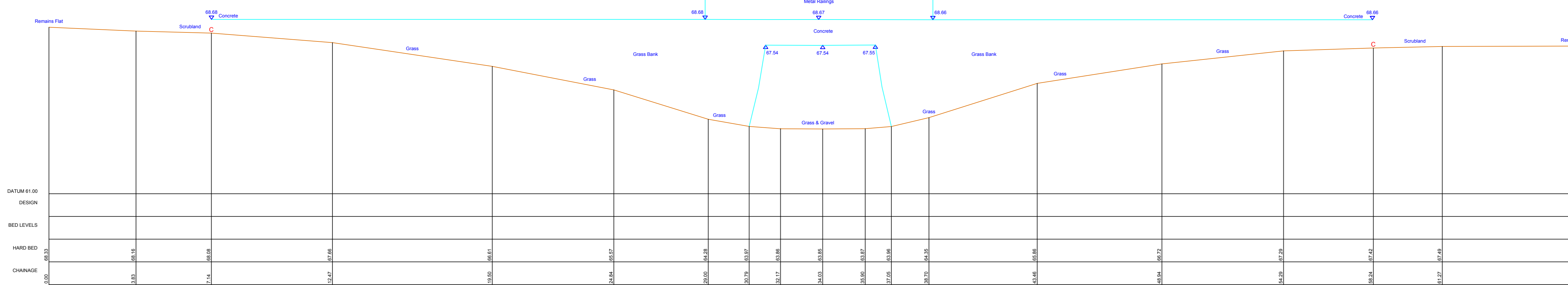
SCALE = 1:100



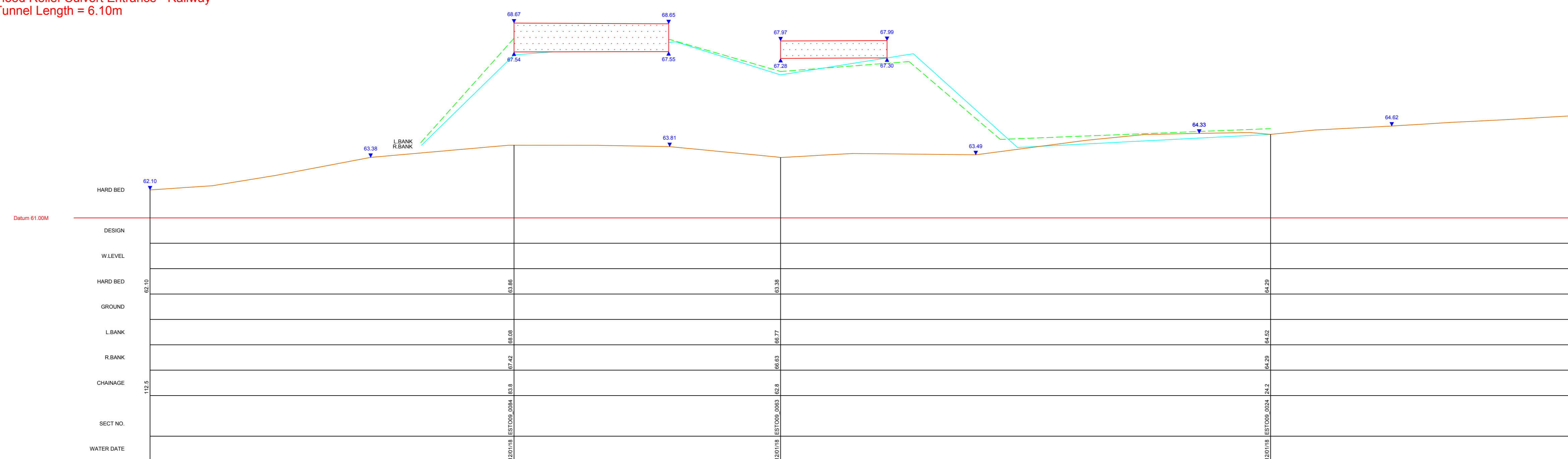
EST009_0024
611168.34mE 137562.38mN Brg 279
Outlet Spill Levels



EST009_0063
611170.11mE 137600.83mN Brg 281
Flood Relief Culvert - Railway
Tunnel Length = 4.20m



EST009_0084
611198.11mE 137616.55mN Brg 281
Flood Relief Culvert Entrance - Railway
Tunnel Length = 6.10m



SCALE = 1:200 H, 1:100 V

KEY TO SECTIONS:

- WATER LEVEL
- VISIBLE BED (TOP OF SILT) AND GROUND
- HARD BED (DETERMINED BY PROBING)
- BANK CREST

KEY TO LONGITUDINAL SECTION ONLY:

- VIEWED LOOKING DOWNSTREAM
- POINTS INDICATED BY 'C' ON CROSS SECTIONS AND ADDITIONAL POINTS BETWEEN SECTIONS

LOCATION PLAN ORIENTATION

NOTES:

1. A REPORT HAS BEEN PRODUCED FOR THIS SURVEY.
2. THIS MAP IS REPRODUCED FROM THE OS MAP BY THE ENVIRONMENT AGENCY WITH PERMISSION OF HER MAJESTY'S STATISTICAL OFFICE. © CROWN COPYRIGHT LICENCE. ALL RIGHTS RESERVED. UNAUTHORISED REPRODUCTION INFRINGES CROWN COPYRIGHT AND MAY LEAD TO PROSECUTION OR CIVIL PROCEEDINGS. LICENCE NO. 100026380.
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SURVEY LEGEND

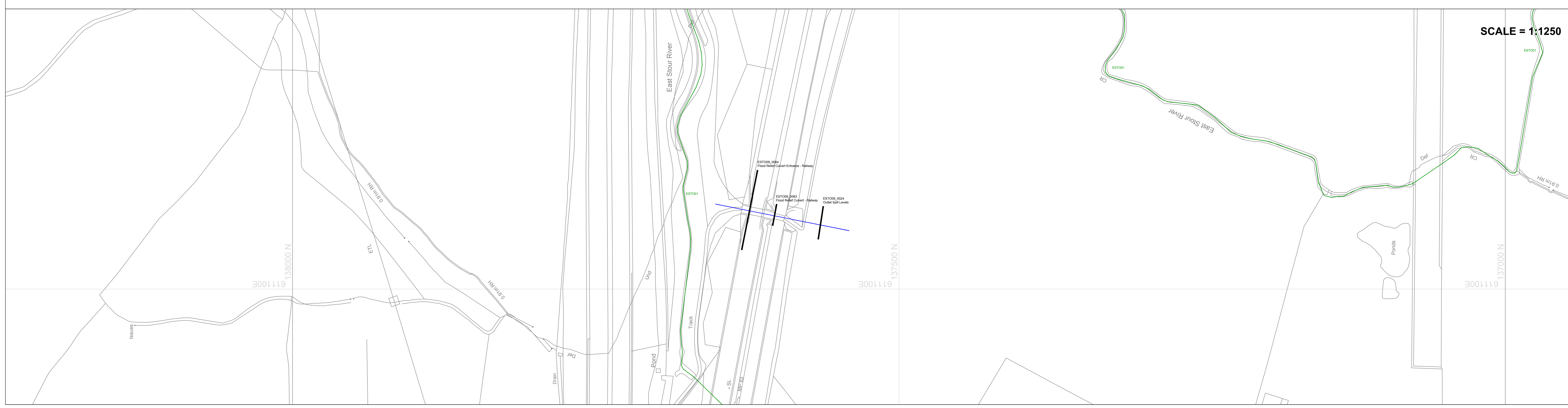
AS BECK	TR	TRAIL	TR	TRAIL	TR
...

AMENDMENT

NO	DESCRIPTION	DRN	CHKD	DATE

CONTROL USED:

TYPE	DESCRIPTION	LEVEL
TR 0103	4107	35.925
TR 0229	4227	36.480
TR 0195	4202	36.480
TR 0126	4199	37.327
TR 0543	4205	40.280
TR 0671	3642	63.022
TR 0504	3900	43.712
TR 0121	4070	62.816
TR 0671	3628	62.251



SCALE = 1:1250

Environment Agency
KENT & SOUTH LONDON REGION
Ordnance Survey, Endonour Park, London Road, Addlestone, West Mids., Kent, ME19 5QH

PROJECT/WATERCOURSE
EAST STOUR, ASHFORD TO STANFORD

SITE/LIMITS
EAST STOUR FLOOD RELIEF CULVERTS (EST009)
CROSS SECTIONS, LONG SECTION
& LOCATION PLAN
EST009_0024 TO EST009_0084

SURVEYED BY: MALTBY LAND SURVEYS LTD *Ref: 12_157*
SURVEY DATE: JANUARY 2018

SCALE: AS SHOWN **DRN:** RC **CHKD:** ITS

DATUM: OS GPS ACTIVE **DATE:** MAR 18 **DATE:** MAR 18

GRID: NATIONAL GRID **DRAWING NO.:** X-J01058-40 **REV.:** 1

DATE: MAR 18

Annex C

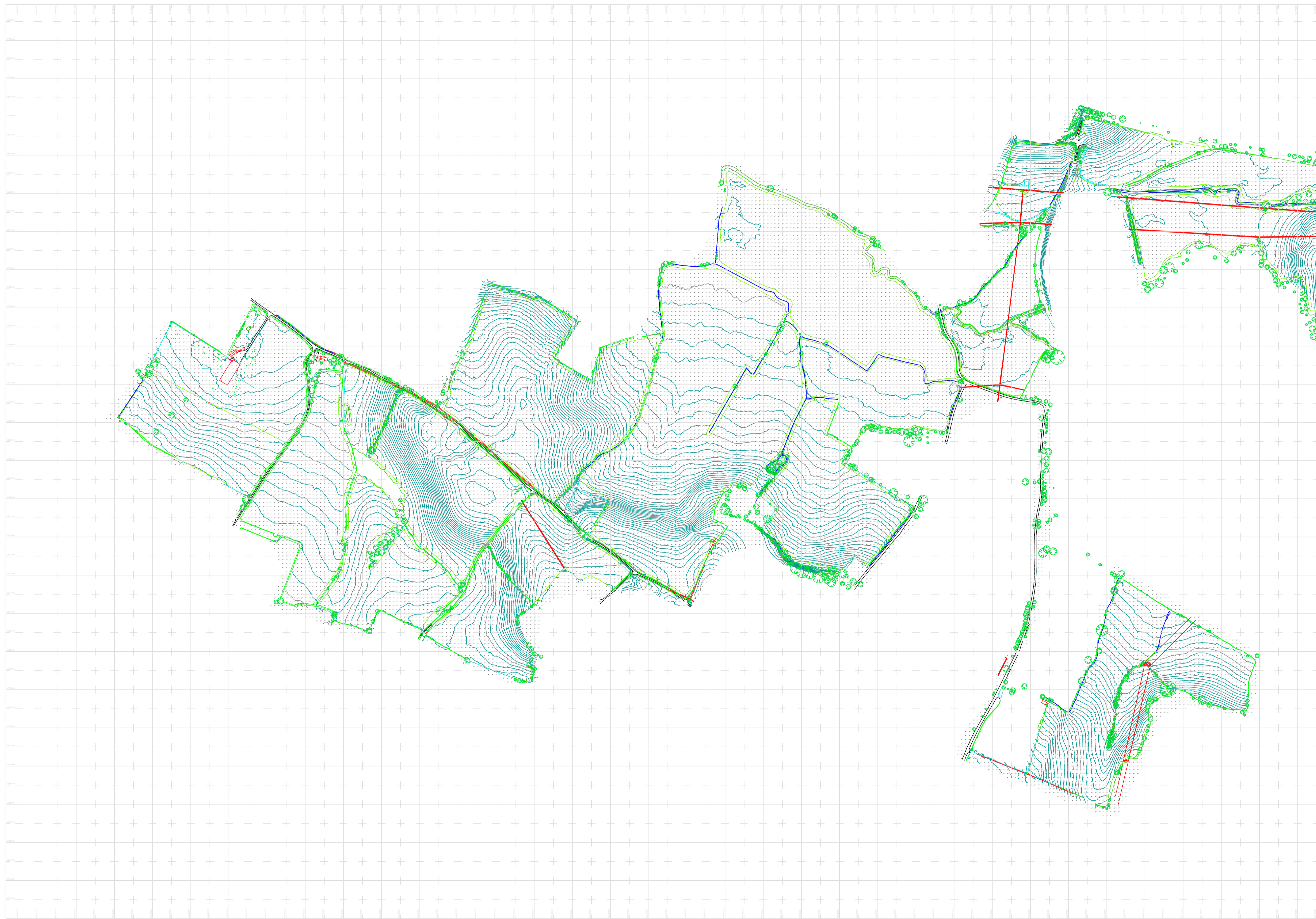
Sensat Topographic Survey

Annex B: East Stour Hydraulic Modelling Report

Stonestreet Green Solar Farm

EPL 001 Limited





Legend

- Building
- Bottom of Bank
- Concrete
- Drainage Ditch
- Fence
- Foot Path
- Grass Edge
- Hedge (Edge)
- Hedge Polyline
- KerbChannel
- KerbTop
- Overhead Wires
- Tarmac
- Top of Bank
- Edge of Tree Canopy Left
- Edge of Tree Canopy Right
- Verge
- Wall
- Waters Edge
- White Line

- ⏏ Gate
- Tree 2Pt Canopy
- ⊕ Electricity Pole

01	28/01/22	CW	BC	CW
----	----------	----	----	----

Issue	Date	By	Chkd	Appd
-------	------	----	------	------

sensat[®]
 Sensat, 160 Old St, London EC1V 9BP
 www.sensat.co.uk
 contact@sensat.co.uk
 +44 (0) 20 3488 2645

Client
 EPL 001 Ltd

Job Title
 Stonestreet Solar Farm

Drawing Title
 Topographic Survey derived from
 UAV Photogrammetry & Lidar
 Point Clouds
 Grid: OSGB36NG & OSGM15

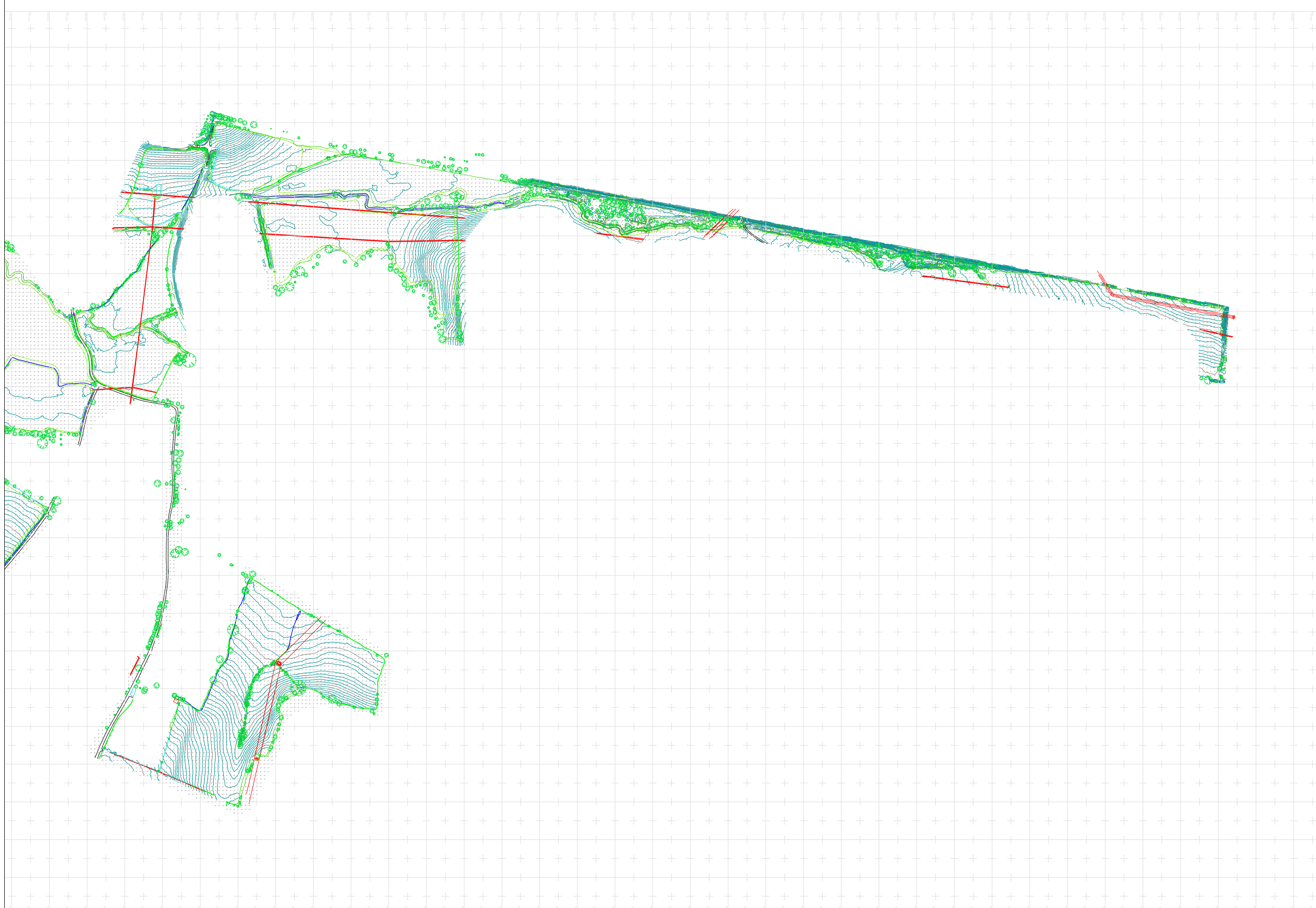
Scale at A1 1:5000

Plot ID

Drawing Status

For Information

Job No	Drawing No	Issue
S22213-00	Sheet 1 of 2	01



Legend

- Building
- Bottom of Bank
- Concrete
- Drainage Ditch
- Fence
- Foot Path
- Grass Edge
- Hedge (Edge)
- Hedge Polyline
- KerbChannel
- KerbTop
- Overhead Wires
- Tarmac
- Top of Bank
- Edge of Tree Canopy Left
- Edge of Tree Canopy Right
- Verge
- Wall
- Waters Edge
- White Line
- ⏏ Gate
- Tree 2Pt Canopy
- ⊙ Electricity Pole

01	28/01/22	CW	BC	CW
Issue	Date	By	Chkd	Appd

sensat[®]
 Sensat, 160 Old St, London EC1V 9BP
 www.sensat.co.uk
 contact@sensat.co.uk
 +44 (0) 20 3488 2645

Client
 EPL 001 Ltd

Job Title
 Stonestreet Solar Farm

Drawing Title
 Topographic Survey derived from
 UAV Photogrammetry & Lidar
 Point Clouds
 Grid: OSGB36NG & OSGM15
 Scale at A1 1:5000
 Plot ID

Drawing Status
For Information

Job No	Drawing No	Issue
S22213-00	Sheet 2 of 2	01

Annex D

Hydrology Report

(provided as standalone report)

Annex B: East Stour Hydraulic Modelling Report

Stonestreet Green Solar Farm

EPL 001 Limited





Hydrology Report

Stonestreet Green Solar Farm

EPL 001 Limited

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24 January 2024

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Method Statement	24 August 2023	██████████	██████████	██████████
Calculations Revision 1	25 January 2024	██████████	██████████	██████████

Basis of Report

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Table of Contents

1.0 Summary of Assessment	1
1.1 Introduction	1
1.2 Flood Frequencies	2
2.0 Method Statement	3
2.1 Requirements for Flood Estimates	3
2.2 The Catchment	3
2.3 Hydrometric Data	4
2.4 Hydrological Understanding of the Catchment	8
2.5 Initial Choice of Approach	15
3.0 Locations Where Flood Estimates are Required	16
3.1 Summary of Subject Sites	16
3.2 Catchment Descriptors	17
4.0 Stationary Statistical Methods	19
4.1 Method Overview	19
4.2 Estimating QMED	19
4.3 Estimating Growth Curves	22
4.4 Final Choice of QMED and Growth Curves	25
5.0 Revitalised Flood Hydrograph 2 (ReFH2) Method	27
5.1 Method Overview	27
5.2 Model Parameters	27
5.3 Model Inputs for Design Events	28
5.4 Final Choice of ReFH2 Flow Estimates	32
6.0 Discussion and Summary of Results	34
6.1 Comparison of Results from Different Methods	34
6.2 Final Choice of Method	34
6.3 Final Results	35
6.4 Checks	35
6.5 Assumptions, limitations, and uncertainty	39

Tables in Text

Table 2-1: Gauging stations (flow and level)	4
Table 2-2: Data available at each flow gauging station	5
Table 2-3: Rating Equations	5
Table 2-4: Other data available and how it has been obtained	6
Table 2-5: Conclusions of hydrometric data review:	7



Table 3-1: Flood Estimate Subject Sites	16
Table 4-1: Donor sites chosen and QMED adjustment factors	21
Table 4-2: Derivation of growth curves at subject sites.....	22
Table 4-3: Derivation of pooling groups.....	24
Table 4-4: Pooling group composition	24
Table 4-5: Method choice and reasons	25
Table 4-6: Final flood estimates from stationary statistical methods	26
Table 5-1: Summary of model parameters	27
Table 5-2: Design events for lumped catchments.....	28
Table 5-3: Design events for subcatchments and intervening areas.....	28
Table 5-4: Method choice and reasons	32
Table 5-5: Final flood estimates from ReFH2 method	32
Table 6-1: Growth factor checks	35
Table 6-2: Specific discharge	36
Table 6-3: Uncertainty.....	41

Figures in Text

Figure 2-1: South Willesborough plots of flow data and interpretation (Dec 1979-Jan 1980). 8	8
Figure 2-2: South Willesborough plots of flow data and interpretation (Feb 2001).....	9
Figure 2-3: Aylesford plots of flood peak data and interpretation (Chart).....	9
Figure 2-4: Aylesford plots of flood peak data and interpretation (Graph).....	10
Figure 2-5: Chart Leacon plots of flood peak data and interpretation (Chart)	10
Figure 2-6: Chart Leacon plots of flood peak data and interpretation (Graph)	11
Figure 2-7: Horton plots of flood peak and flow data and interpretation (Chart).....	11
Figure 2-8: Horton plots of flood peak and flow data and interpretation (Graph).....	12
Figure 2-9: Horton plots of flood peak and flow data and interpretation (Jan 1969).....	12
Figure 2-10: Horton plots of flood peak and flow data and interpretation (March 1975).....	13
Figure 2-11: Horton plots of flood peak and flow data and interpretation (Feb 2001).....	13
Figure 2-12: Hopemill plots of flood peak data and interpretation (Chart).....	14
Figure 2-13: Hopemill plots of flood peak data and interpretation (Graph).....	14
Figure 4-1: ESUS Final Non-Flood Year Adjusted Growth Curve.....	23
Figure 4-2: TRIB1 Final Non-Flood Year Adjusted Growth Curve.....	23
Figure 4-3: TRIB2 Final Non-Flood Year Adjusted Growth Curve.....	23
Figure 4-4: TRIB3 Final Non-Flood Year Adjusted Growth Curve.....	23
Figure 4-5: ESDS Final Non-Flood Year Adjusted Growth Curve	24



Figures

- HR1 Local Hydrology
- HR2 Catchment and Gauging Stations
- HR3 Aldington Flood Storage Area Extent
- HR4 Catchments and Sub catchments

Appendices

Digital Files – Input Data

FEH_Catchment_Descriptors_609400_137700_v5_0_1.xml
FEH_Catchment_Descriptors_608350_138850_v5_0_1.xml
FEH_Catchment_Descriptors_608900_138550_v5_0_1.xml
FEH_Catchment_Descriptors_607250_137150_v5_0_1.xml
FEH_Catchment_Descriptors_605300_138600_v5_0_1.xml
FEH_Point_Descriptors_605897_137543_v5_0_1.xml

Digital File – Project or Calculation Files

ReFH2_and_WINFAP_Design_Flow_Estimation_Summary_Sheet_v2.xlsx
Stat – WINFAP Projects and Non Flood Years Adjustment Spreadsheets
ReFH – ReFH Projects and CSV outputs

Digital Files – Output Data

ESUS Scaled Hydrographs.xlsx
ICHL Scaled Hydrographs.xlsx
ICULV1 Scaled Hydrographs.xlsx
ICULV2 Scaled Hydrographs.xlsx
IES1 Scaled Hydrographs.xlsx
IES2 Scaled Hydrographs.xlsx
ITRIB1a Scaled Hydrographs.xlsx
ITRIB1b Scaled Hydrographs.xlsx
ITRIB2a Scaled Hydrographs.xlsx
ITRIB2b Scaled Hydrographs.xlsx
ITRIB3 Scaled Hydrographs.xlsx
TRIB1 Scaled Hydrographs.xlsx
TRIB2 Scaled Hydrographs.xlsx
TRIB3 Scaled Hydrographs.xlsx

Other Supporting Information

Upper Stour hydrological assessment by continuous simulation. Draft V3. JBA, May 2023
RPs_DesignFlows20230512.xlsx (JBA, 2023)



Autumn 2000 Great Stour flood rarity. JBA, July 2014.

2013-2014 Post Flood Analysis: Kent & South London Area. JBA, December 2014.



Acronyms and Abbreviations

AEP	Annual exceedance probability
AMAX	Annual Maximum
AREA	Catchment area (km ²)
BFI	Base Flow Index
BFIHOST19	Base Flow Index derived using the HOST soil classification, revised in 2019
CSM	Continuous Simulation Modelling
FARL	FEH index of flood attenuation due to reservoirs and lakes
FEH	Flood Estimation Handbook
FSA	Flood Storage Area
GEV	Generalised Extreme Value
GLO	Generalised Logistic
HOST	Hydrology of Soil Types
IF	Impervious Fraction
IRF	Impervious Runoff Factor
LF	Low flow statistics (flow duration curve)
LIDAR	Light Detecting and Ranging
NRFA	National River Flow Archive
POT	Peaks Over a Threshold
QMED	Median Annual Flood (with return period 2 years)
ReFH	Revitalised Flood Hydrograph method
ReFH2	Revitalised Flood Hydrograph 2 method
SAAR	Standard Average Annual Rainfall (mm)
T _p	Time to peak of the instantaneous unit hydrograph
URBAN	Flood Studies Report index of fractional urban extent
URBEXT1990	FEH index of fractional urban extent
URBEXT2000	Revised index of urban extent, measured differently from URBEXT1990
WINFAP	Windows Frequency Analysis Package (software that can be used for FEH statistical method)



1.0 Summary of Assessment

1.1 Introduction

This report has been prepared inline with the Environment Agency's Flood Estimation Guidelines (LIT 11832) and follows the format of the Environment Agency's Flood Estimation Report Template (LIT 65087). It provides a record of the hydrological context, the method statement, the calculations, the decisions made, and the results of flood estimation.

1.1.1 Catchment location

East Stour River and Tributaries.

Stonestreet Green, Ashford, Kent E 605285 N 138611.

1.1.2 Purpose of study and complexity

Inflows for hydraulic model of the East Stour River in Stonestreet Green, Kent in relation to the Development Consent Order (DCO) application for Stonestreet Green Solar ("the Project"). The land within the Order limits is known as the 'Site'. The Main Rivers and Ordinary Watercourses in the vicinity of the Site are shown in the accompanying **HR Figure 1**. Hydrologic estimation consists of;

- Downstream lumped catchment - East Stour River E 605300 N 138600;
- Four upstream lumped catchments - East Stour River E 609400 N 137700, Unnamed Tributary 1 E 608350 N 138850, Unnamed Tributary 2 E 608900 N 138550 and Unnamed Tributary 3 E 607250 N 137150;
- Ten intervening sub-catchments.

1.1.3 Key catchment features

Geology: Headwaters to the north at the border of the Gault and Lower Chalk formations, middle catchment underlain by intergranularly permeable sedimentary bedrocks consisting of Mudstone, Sandstone, Limestone and Siltstone. Lower catchment underlain by Weald Clay formation. Hydraulic model includes the Aldington Flood Storage Area (FSA).

1.1.4 Flooding mechanisms

Risk of fluvial flooding from the East Stour River.

1.1.5 Gauged / ungauged

Ungauged catchment. East Stour at South Willesborough is present downstream, however due to presence of FSA upstream not suitable for QMED or Pooling. The following local gauges have been reviewed:

- **East Stour at South Willesborough (NRFA - 40023)** 15-min flow data available. Upstream flood alleviation scheme truncates peaks.
- **Stage Gauge - Aldington Upstream (EA Gauge - E4351)** 15-min stage data available.
- **Old Mill Stream at Alyesford (NRFA - 40035)** Suitable for QMED, local catchment to the north of the East Stour River.
- **Great Stour at Chart Leacon (NRFA - 40022)** Suitable for QMED, local catchment to the west of Ashford with similar geology to the East Stour River.



- **Great Stour at Horton (NRFA – 40011)** Suitable for QMED and Pooling, catchment is downstream of site.
- **Hexden Channel at Hopemill (NRFA – 40021)** Suitable for QMED, local catchment with similar geology to Unnamed Tributary 3.
- **Rainfall Gauge – Sellindge (EA Gauge – E4510)** 15-min rainfall data. Within East Stour River catchment to the northeast of Aldington FSA.

1.1.6 Final choice of method

Hybrid method, Rainfall Runoff scaled to peak flow from Statistical method for lumped catchments up to and including the 1 in 100 year event. 1 in 1000 year peak flows scaled from Statistical 1 in 100 year peak flow using Rainfall Runoff 1 in 100 year to 1 in 1000 year peak flow uplift.

1.1.7 Key limitations / uncertainties in results

Lack of gauge data suitable for peak flow estimates for the East Stour.

1.2 Flood Frequencies

- The frequency of a flood can be quoted in terms of a return period, which is defined as the average time between years with at least one larger flood, or as an annual exceedance probability (AEP), which is the inverse of the return period.
- Return periods are output by the Flood Estimation Handbook (FEH) software and can be expressed more succinctly than AEP. However, AEP can be helpful when presenting results to members of the public who may associate the concept of return period with a regular occurrence rather than an average recurrence interval.
- Results tables in this document contain both return period and AEP titles.
- The table below is provided to enable quick conversion between return periods and annual exceedance probabilities.

AEP	50	20	10	5	3.33	2	1.33	1	0.5	01
AEP	0.5	0.2	0.1	0.05	0.033	0.02	0.013	0.01	0.005	0.001
Return period (yrs)	2	5	10	20	30	50	75	100	200	1,000



2.0 Method Statement

2.1 Requirements for Flood Estimates

2.1.1 Overview and Project Scope

The purpose of the study is to provide inflows to the hydraulic model of the reaches of the East Stour River and associated tributaries shown in the accompanying **HR Figure 2**. Both peak flow and hydrographs are required.

Design flows of the 5.0%, 3.3%, 1.0% and 0.1% annual exceedance probability (AEP) have been estimated to reflect Environment Agency (EA) Flood Zones¹. The 30% uplift for the higher central and 55% uplift for the upper end 2050s epoch climate change allowances² have been applied to the 1% AEP event to account for the impact of climate change during the 40 year lifetime of development (starting in 2026).

Five catchments have been analysed as part of this hydrological study:

- East Stour River at E 609400 N 137700 – Upstream lumped catchment.
- Unnamed Tributary 1 at E 608350 N 138850 – Upstream lumped catchment.
- Unnamed Tributary 2 at E 608900 N 138550 – Upstream lumped catchment.
- Unnamed Tributary 3 at E 607250 N 137150 – Upstream lumped catchment.
- East Stour at E 605300 N 138600 – Downstream lumped catchment.

The overall project aim is to provide an updated assessment of flood risk to the Site. Part of the Site is within the 2016 South Ashford flood study however new hydrological estimates are required to improve the understanding of flood risk to the Site.

2.2 The Catchment

2.2.1 Maps:

The catchment and gauging stations are shown in **HR Figure 2**.

2.2.2 Catchment Description

2.2.2.1 East Stour

Catchment geology consists of Lower Chalk (high permeability) formations and Gault (low permeability) formation consisting of Clay, Mudstone and Siltstone in the headwaters, Sedimentary bedrocks consisting of Mudstone, Sandstone, Limestone and Siltstone (mixed permeability) in the middle catchment and Weald Clay (low permeability) formation in the lower catchment. Includes the Aldington FSA, with two unnamed tributaries flowing in from the north, the East Stour headwaters are steep peaking in the north of unnamed Tributary 2 at approximately 180mAOD. The middle and lower catchment have a shallow gradient with the downstream extent of the catchment at approximately 40mAOD. The catchment is essentially rural with limited small areas of urbanisation.

¹ GOV.UK, <https://www.gov.uk/guidance/flood-risk-and-coastal-change> accessed 4th August 2023

² GOV.UK <https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances> accessed 4th August 2023



2.2.2.2 Unnamed Tributary 1

Catchment geology consists of Lower Chalk formations and Gault formation consisting of Clay, Mudstone and Siltstone in the upper catchment and Sedimentary bedrocks consisting of Sandstone, Siltstone and Mudstone in the lower catchment. The headwaters are steep peaking in the north at approximately 180mAOD. The downstream extent of the catchment is at approximately 50mAOD. The catchment is slightly urbanised containing the villages of Bradbourne, East Bradbourne, Bradbourne Lees and Lily Vale.

2.2.2.3 Unnamed Tributary 2

Catchment geology consists of Lower Chalk formations and Gault formation consisting of Clay, Mudstone and Siltstone in the upper catchment and Sedimentary bedrocks consisting of Sandstone, Siltstone and Mudstone in the lower catchment. The headwaters are steep peaking in the north at approximately 180mAOD. The catchment is essentially rural.

2.2.2.4 Unnamed Tributary 3

Catchment geology consists of the Hythe Formation interbedded Sandstone and Limestone in the upper catchment and Weald Clay formation in the lower catchment. The catchment is gently sloping, reaching approximately 100mAOD to the south east with the downstream extent of the catchment at approximately 50mAOD. The catchment is essentially rural.

2.3 Hydrometric Data

2.3.1 Source of flood peak data

NRFA peak flows dataset, Version 11.1, released March 2023. This contains data up to water year 2021.

2.3.2 Gauging stations (flow and level)

Table 2-1: Gauging stations (flow and level)

Watercourse	Station Name	Gauging Authority Number	NRFA Number	Catchment area (km ²)	Type (Rated/ ultrasonic/ level)	Start of Record and end (if station closed)
East Stour	South Willesborough	654210001	40023	58.8	Flat V	01/1979
Old Mill Stream	Aylesford	654220001	40035	18.0	Velocity-area	12/2003
Great Stour	Chart Leacon	654110001	40022	72.5	Flat V	01/1967
Great Stour	Horton	654400001	40011	345	Broad-crested weir/Velocity-area	01/1964
Hexden Channel	Hopemill	556710001	40021	32.4	Flume	01/1973



2.3.3 Data available at each flow gauging station

Table 2-2: Data available at each flow gauging station

Station Name	Start and end of flood peak record	Update for this study?	OK for QMED?	OK for pooling?	Data quality check needed?	Station and flow data quality summary
South Willesborough	1975 – 2022	No	No	No	No	Upstream flood alleviation scheme truncates peak flows therefore not suitable for QMED or Pooling estimate.
Aylesford	2003 – Present	No	Yes	No	No	Peak flow gauging's are good. Water year 2005/2006 and 2012/2012 rejected as AMAX likely occurred during period of missing data.
Chart Leacon	1980 – 2021	No	Yes	No	No	Theoretical rating at high flows. Rating extended to 1.8m using hydraulic modelling but low confidence beyond 1.13m as flow is out of bank.
Horton	1964 – 2021	No	Yes	Yes	No	Gauged beyond AMAX3. Rating is reliable.

2.3.4 Rating Equations

Table 2-3: Rating Equations

Station name	Type of rating e.g., theoretical, empirical; degree of extrapolation	Rating review needed?	Comments and link to any rating reviews
South Willesborough	Theoretical rating to 5.3 cumecs (under review)	No	Station not suitable for QMED or Pooling estimates. Almost all flows contained.
Aylesford	Rating derived from ultrasonic gauge data.	No	Peak flow data appears to be good.
Chart Leacon	Theoretical rating, velocity-area calibration for high flows.	No	Rating validated to QMED.
Horton	Theoretical rating from weir equation.	No	All flows within weir range. Rating is thought to be reliable.
Hopemill	Power law rating in three sections.	No	Gauged within 30% of QMED. Flows observed to be within bank.



2.3.5 Other data available and how it has been obtained

Table 2-4: Other data available and how it has been obtained

Type of data	Data relevant to this study?	Data available?	Source of data	Details
Check flow gaugings	No	Yes	NRFA	Ratings suitable for proposed uses.
Historical flood data	Yes	Yes	Autumn 2000 Great Stour flood rarity report, July 2014 and 2013-2014 Post Flood Analysis: Kent & South London Area, December 2014.	Analysis includes events where the Aldington FSA reached full capacity.
Flow or river level data for events	Yes	Yes	NRFA/EA Hydrology Data API	<p>South Willesborough: 15 minute flow data for 29th December 1979 and 6th February 2001 events.</p> <p>Horton: 15 minute flow data for 18th January 1969, March 1975 and 9th February 2001 events.</p> <p>Aylesford: Full 15 minute flow dataset has been requested from the EA.</p> <p>Chart Leacon: Full 15 minute flow dataset has been requested from the EA.</p> <p>Hopemill: Full 15 minute flow dataset has been requested from the EA.</p>



Type of data	Data relevant to this study?	Data available?	Source of data	Details
Rainfall data for events	Yes	Yes	NRFA/EA Hydrology Data API	<p>South Willesborough: Catchment daily rainfall data.</p> <p>Aylesford: Catchment daily rainfall data.</p> <p>Chart Leacon: Catchment daily rainfall data.</p> <p>Horton: Catchment daily rainfall data.</p> <p>Hopemill: Catchment daily rainfall data.</p> <p>Sellindge: 15 minute rainfall data for December 2003, February 2014, April/May 2018 and March 2020 events.</p>
Potential evaporation data	No	No		Evaporation data not thought to have a large impact on flooding at The Site due to peaks occurring during large winter storms.
Results from previous studies	Yes	Yes		<p>South Ashford 2016 flood risk mapping study.</p> <p>Upper Stour hydrological assessment by continuous simulation (JBA, 2023)</p>
Other data or information	No	No		

2.3.6 Conclusions of hydrometric data review

Table 2-5: Conclusions of hydrometric data review

Station name	Rating suitability	Suitability for flood estimation calculations
South Willesborough	Suitable	Not suitable



Station name	Rating suitability	Suitability for flood estimation calculations
Aylesford	Suitable	QMED only
Chart Leacon	Suitable	QMED only
Horton	Suitable	QMED and Pooling
Hopemill	Suitable	QMED only

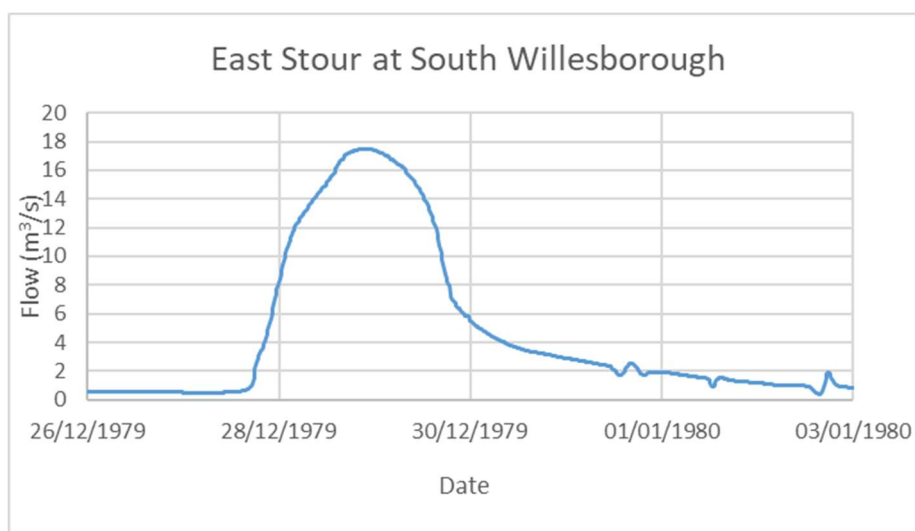
2.4 Hydrological Understanding of the Catchment

2.4.1 Plots of flood peak data and interpretation

2.4.1.1 East Stour at South Willesborough

Seasonality of peak flow events is skewed to winter floods.

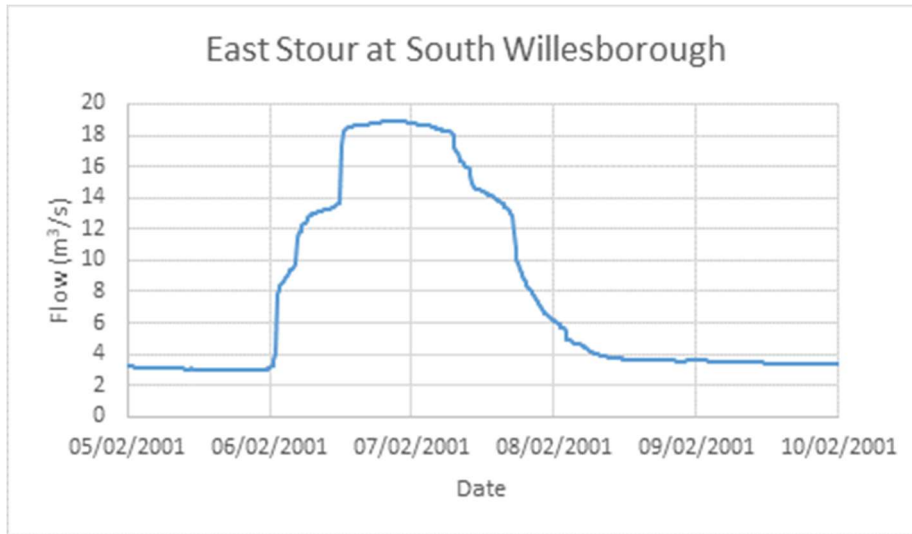
Figure 2-1: South Willesborough plots of flow data and interpretation (Dec 1979-Jan 1980)



Event starts at approximately 17:00 27/12/1979 reaching a peak flow of 17.5 m³/s at 20:00 28/12/1979. Data is from prior to construction of the Aldington FSA in 1989.



Figure 2-2: South Willesborough plots of flow data and interpretation (Feb 2001)



Event starts at approximately 00:00 06/02/2001 reaching a peak flow of 18.9 m³/s at 19:00 06/02/2001. Data is after the completion of the Aldington FSA. Note peak flow is beyond upper limit of rating.

2.4.1.2 Old Mill Stream at Aylesford

Max gauged flow of 7.189 m³/s. All AMAX events occur during winter.

Figure 2-3: Aylesford plots of flood peak data and interpretation (Chart)

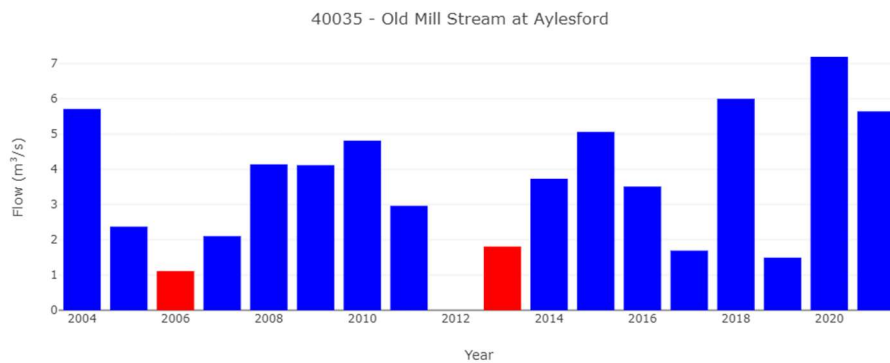
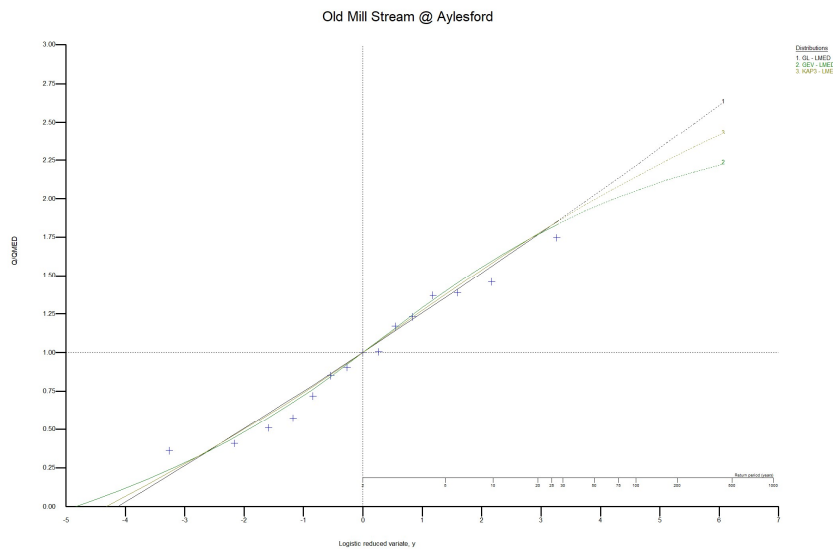


Figure 2-4: Aylesford plots of flood peak data and interpretation (Graph)



2.4.1.3 Great Stour at Chart Leacon

Max gauged flow of 20.316 m³/s. AMAX events are heavily skewed to winter events.

Figure 2-5: Chart Leacon plots of flood peak data and interpretation (Chart)

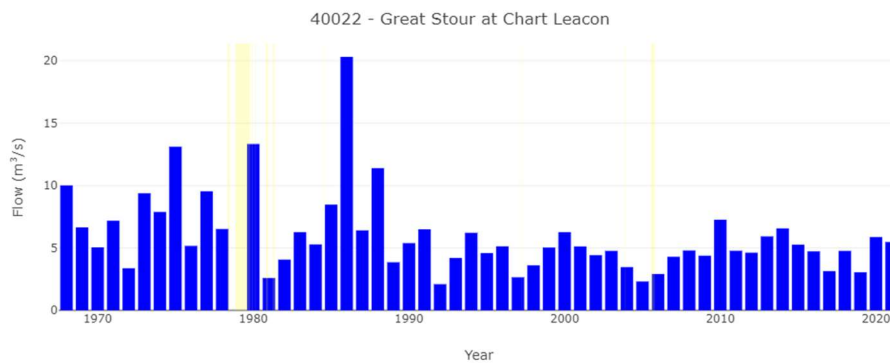
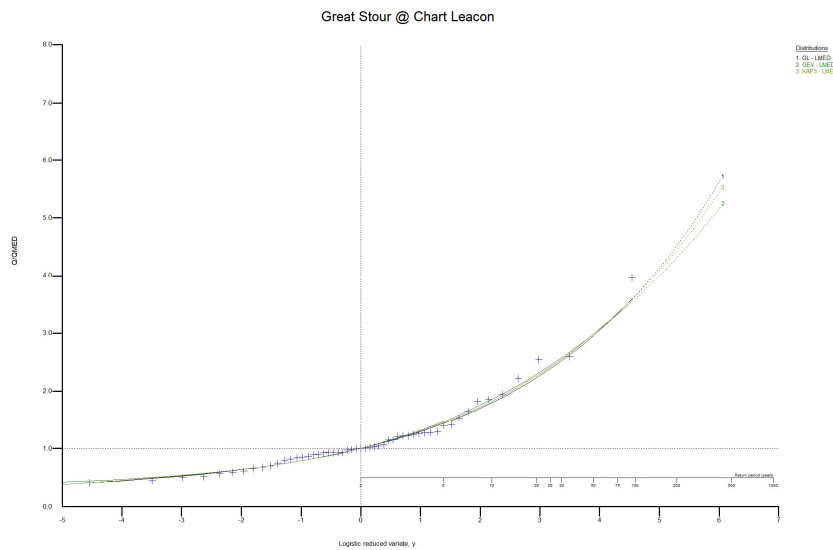


Figure 2-6: Chart Leacon plots of flood peak data and interpretation (Graph)



2.4.1.4 Great Stour at Horton

Max gauged flow of 33.658 m³/s. AMAX events are heavily skewed to winter events.

Figure 2-7: Horton plots of flood peak and flow data and interpretation (Chart)

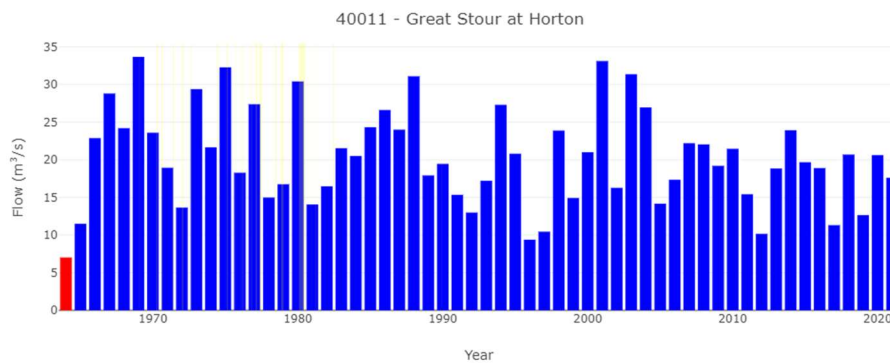


Figure 2-8: Horton plots of flood peak and flow data and interpretation (Graph)

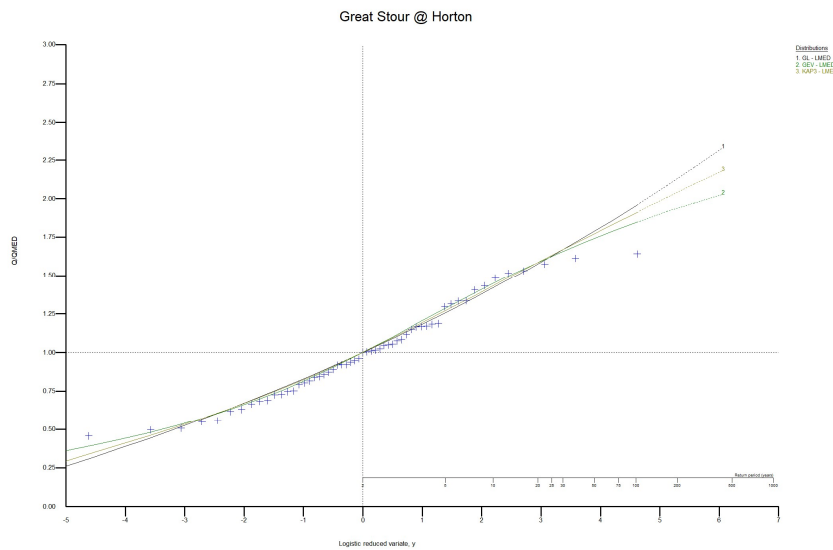
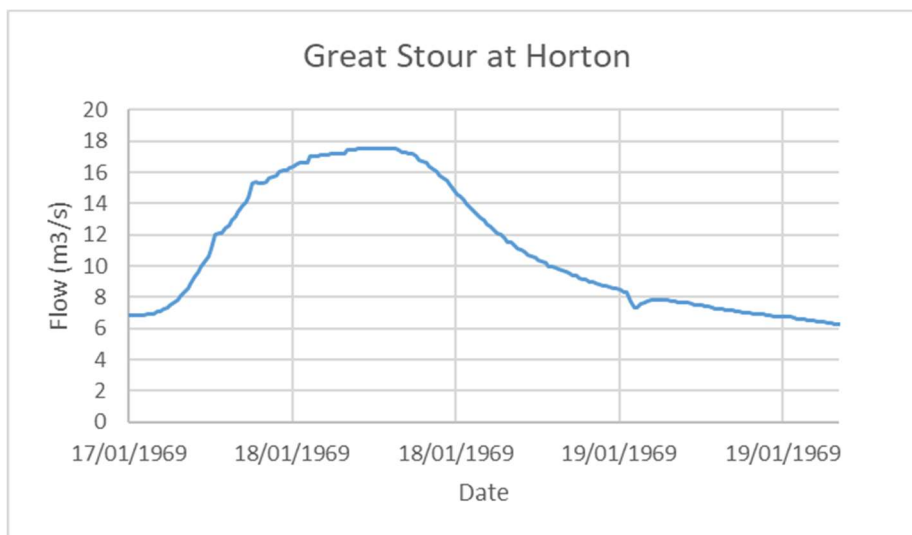


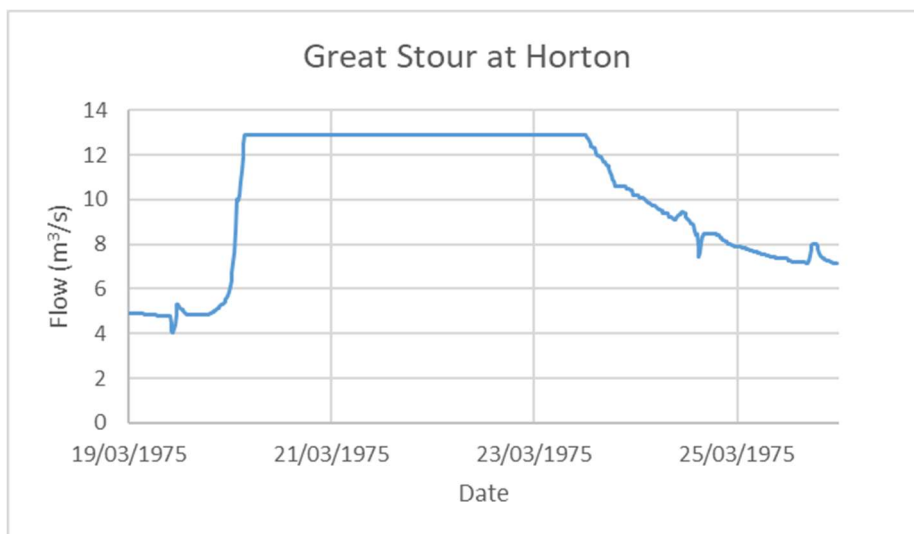
Figure 2-9: Horton plots of flood peak and flow data and interpretation (Jan 1969)



Event starts at approximately 00:00 18/01/1969 reaching a peak flow of 17.5 m³/s at 12:45 18/01/1969.



Figure 2-10: Horton plots of flood peak and flow data and interpretation (March 1975)



Peak flows appear to be truncated at 12.9 m³/s.

Figure 2-11: Horton plots of flood peak and flow data and interpretation (Feb 2001)



Event starts at approximately 00:00 08/02/2001 reaching a peak flow of 33.1 m³/s at 03:00 09/02/2001.



2.4.1.5 Hexden Channel at Hopemill

Max gauged flow of 28.88 m³/s. AMAX events are heavily skewed to winter events.

Figure 2-12: Hopemill plots of flood peak data and interpretation (Chart)

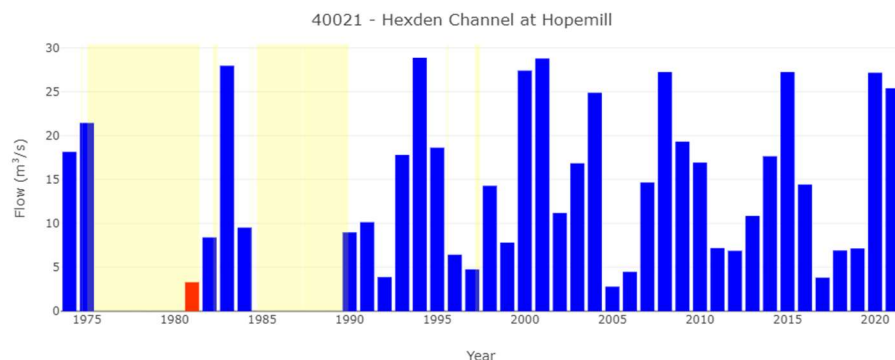
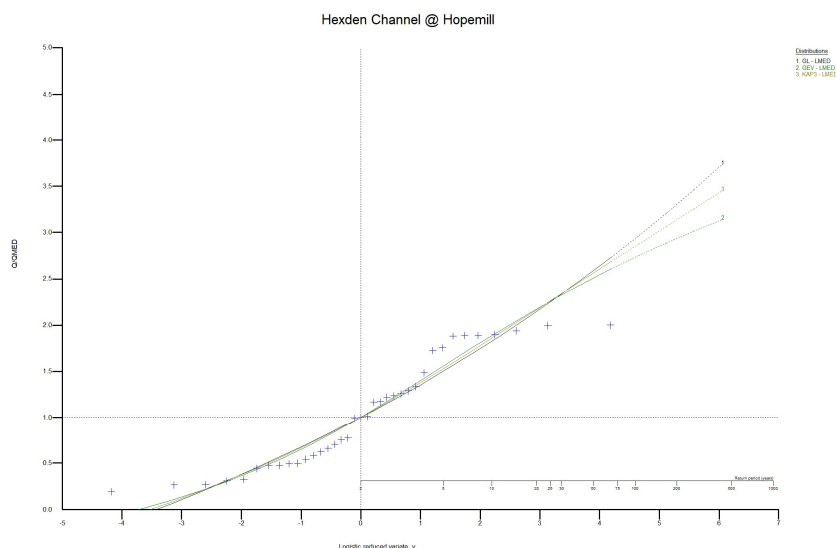


Figure 2-13: Hopemill plots of flood peak data and interpretation (Graph)



2.4.2 Conceptual model

The hydraulic model is volume driven model, particularly when considering the defended scenario due to the presence of the Aldington FSA. The East Stour River is most susceptible from winter rainfall events, due to the groundwater influence on flows and the attenuating affect of the Aldington FSA on peak flows during flashier summer rainfall events.

The Site is at risk of flooding from the East Stour River.

Blockages may be possible in the culverts through the railway embankment upstream of the Site, and at the inlet of to the control structures in the Aldington FSA embankment. Downstream of the site blockages are highly unlikely due the large opening of the bridges, and lack of potential debris sources.



2.4.3 Unusual catchment features

The Aldington FSA is within the catchment the approximate extent of which is shown by **HR Figure 3**.

The headwaters of the catchment are underlain by highly permeable Chalk bedrock, with the sources of the East Stour River and Unnamed Tributaries 1 and 2 to the north of the Site arising from where the Lower Chalk formations meet the Gault Formation outcrop and as such flows through the East Stour River are expected to be groundwater influenced.

2.5 Initial Choice of Approach

2.5.1 Are FEH methods appropriate?

Yes

2.5.2 Initial choice of method(s) and reasons

Due to the available local gauges suitable for QMED adjustment, the statistical method is deemed the most appropriate estimate of peak flows. The growth in flow rates up to and including the 1% AEP event is best estimated from a pooling group of hydrologically similar catchments. As the flow gauges on the East Stour River are not suitable for QMED or pooling estimates, a single site analysis would not be suitable. Therefore ungauged statistical methods will be used up to and including the 1% AEP event.

Due to the greater availability of long rainfall record and spatial consistency of extreme rainfall events and therefore greater confidence in rainfall frequency curves for large return periods. A hybrid approach will be used for the 0.1% AEP event, with the ReFH2 1% to 0.1% AEP event ratio applied to the statistical 1% AEP peak flow.

2.5.3 How will hydrograph shapes be derived if needed?

Hydrograph profiles will be derived from ReFH2, and scaled so that the critical duration winter peak flow matches peak flow estimates from the statistical (hybrid for 0.1% AEP) method. This is as the local AMAX events are shown to heavily skewed to winter events and the lack of suitable gauge data for a study of average hydrograph shapes. Duration sensitivity testing will be completed as part of the accompanying hydraulic modelling study.

2.5.4 Will the catchment be split into sub-catchments? If so, how?

The East Stour River downstream lumped catchment has been defined using the FEH webservice boundary and has been split into four upstream lumped catchments, the East Stour River, Unnamed Tributaries 1, 2 and 3, also defined using the FEH webservice boundaries. The remaining intervening area has been split into ten sub-catchments defined using interpretation of 1m resolution LiDAR data.

2.5.5 Software to be used

FEH Web Service³, WINFAP 5⁴ and ReFH2.3

³ CEH 2015. The Flood Estimation Handbook (FEH) Online Service, Centre for Ecology & Hydrology, Wallingford, UK.

⁴ WINFAP 5 © Wallingford HydroSolutions Limited 2023.



3.0 Locations Where Flood Estimates are Required

3.1 Summary of Subject Sites

The table below lists the locations of the subject sites. The site codes listed below are used in all subsequent tables to save space. The locations of the catchments are shown by **HR Figure 4**.

Table 3-1: Flood Estimate Subject Sites

Site code	Type of estimate: lumped (L) or sub-catchment (S)	Water-course	Site name / description	Easting	Northing	AREA on FEH Web Service (km ²)	Revised AREA (if altered) (km ²)
ESUS	L	East Stour	East Stour Upstream	609400	137700	19.49	N/A
TRIB1	L	Unnamed Tributary 1	Unnamed tributary of the East Stour A20 crossing near Water Farm.	608350	138850	7.16	N/A
TRIB2	L	Unnamed Tributary 2	Unnamed tributary of the East Stour at A20 crossing near Brindles.	608900	138550	12.85	N/A
TRIB3	L	Unnamed Tributary 3	Unnamed tributary of the East Stour at Harringe Bridge	609400	137700	3.72	N/A
ESDS	L	East Stour	East Stour Downstream	605300	138600	51.91	N/A
IES1	S	East Stour	Intervening area of the East Stour upstream of the FSA	606667	138139	3.72 (TRIB3 used as donor)	2.116
IES2	S	East Stour	Intervening area of East Stour downstream of FSA	605300	138600	3.72 (TRIB3 used as donor)	2.543
ITRIB3	S	Unnamed Tributary 3	Intervening area of Unnamed tributary 3	606659	137912	3.72 (TRIB3 used as donor)	1.206
ICULV1	S	Contributing area	Contributing area into western most culvert upstream of FSA	607105	138375	N/A (point descriptor used)	0.521
ICULV2	S	Contributing area	Contributing area into next culvert east of ICULV1	607425	138315	N/A (point descriptor used)	0.827



Site code	Type of estimate: lumped (L) or sub-catchment (S)	Water-course	Site name / description	Easting	Northing	AREA on FEH Web Service (km ²)	Revised AREA (if altered) (km ²)
ITRIB1a	S	Unnamed Tributary 1	Contributing area flowing into Unnamed tributary 1 between the A20 and M20 roads	608154	138697	N/A (point descriptor used)	0.532
ITRIB1b	S	Unnamed Tributary 1	Contributing area flowing into Unnamed tributary 1 between the M20 and railway embankment	607685	138266	N/A (point descriptor used)	0.290
ICHL	S	Contributing area	Contributing area flowing into Church Lane railway embankment underpass.	608121	138185	N/A (point descriptor used)	0.279
ITRIB2a	S	Unnamed Tributary 2	Contributing area flowing into Unnamed tributary 2 between the A20 and M20 roads	608661	138378	N/A (point descriptor used)	0.217
ITRIB2b	S	Unnamed Tributary 2	Contributing area flowing into Unnamed tributary 2 between the M20 and railway embankment	608579	138101	N/A (point descriptor used)	0.167

3.2 Catchment Descriptors

3.2.1 Final catchment descriptors at each subject site

Any catchment descriptor values that have been manually adjusted are shown in Red. Note for some catchments FARL, DPLBAR, DPSBAR, URBEX2000 and FPEXT are not applicable as point descriptors and plot scale equations have been used.

Site code	FARL	PROPWET	BFIHOST19	DPLBAR (km)	DPSBAR (m/km)	SAAR (mm)	URBEXT 2000	FPEXT
ESUS	0.992	0.34	0.688	5.33	43.3	775	0.0159	0.0904
TRIB1	1	0.34	0.600	2.95	50.5	769	0.0328	0.0473
TRIB2	0.996	0.34	0.749	4.72	61.8	793	0.0149	0.0528
TRIB3	1	0.34	0.427	1.88	44.1	745	0	0.0645
ESDS	0.996	0.34	0.625	8.22	46.7	771	0.0153	0.0938



Site code	FARL	PROPWET	BFIHOST19	DPLBAR (km)	DPSBAR (m/km)	SAAR (mm)	URBEXT 2000	FPEXT
IES1	1	0.34	0.427	1.43	44.1	745	0	0.0645
IES2	1	0.34	0.427	1.57	44.1	745	0	0.0645
ITRIB3	1	0.34	0.427	1.09	44.1	745	0	0.0645
ICUVL1	N/A	0.34	0.509	N/A	N/A	747	N/A	N/A
ICULV2	N/A	0.34	0.509	N/A	N/A	747	N/A	N/A
ITRIB1a	N/A	0.34	0.509	N/A	N/A	747	N/A	N/A
ITRIB1b	N/A	0.34	0.509	N/A	N/A	747	N/A	N/A
ICHL	N/A	0.34	0.509	N/A	N/A	747	N/A	N/A
ITRIB2a	N/A	0.34	0.509	N/A	N/A	747	N/A	N/A
ITRIB2b	N/A	0.34	0.509	N/A	N/A	747	N/A	N/A

3.2.2 Catchment boundary checks and revisions

FEH Webservice boundaries were compared to OpenTopoMap contours for the lumped catchments. FEH derived boundaries show good agreement with mapped contours. The subcatchments have been defined using 1m resolution LiDAR data.

3.2.3 URBEXT source and method for updating

URBEXT2000 was used and checked against aerial imagery and shown to be representative.

3.2.4 BFIHOST source, checks and updates

BFIHOST19 values were checked against BGS geology mapping, it is noted that the BFIHOST19 values are relatively high given the small extent of the catchments that are highly permeable. However, the relative difference in BFIHOST19 values match the underlying geology, and any underestimation of peak flows due to the unrealistically high BFIHOST19 values can be accounted for in the statistical analysis.

3.2.5 Checks and revisions to other catchment descriptors

FARL, DPSBAR and DPLBAR were checked against aerial imagery and shown to be representative.



4.0 Stationary Statistical Methods

4.1 Method Overview

4.1.1 What is the purpose of applying these methods?

Estimate of peak flows for the five study lumped catchments via the ungauged statistical FEH methods.

There are suitable gauges in the local vicinity with similar catchment descriptors and underlying geology.

4.1.2 What methods will be used to estimate QMED and growth curves?

Site code	Methods used for QMED	Methods used for growth curves
ESUS	Data transfer	Pooled
TRIB1	Data transfer	Pooled
TRIB2	Data transfer	Pooled
TRIB3	Data transfer	Pooled
ESDS	Data transfer	Pooled

4.2 Estimating QMED

4.2.1 QMED at ungauged subject sites

Generally QMED increases with catchment size. While TRIB2 is a larger catchment than TRIB1, it also has a higher BFIHOST19 value, and higher level of attenuation and therefore a lower QMED peak flow is plausible.

Site code	Method (CD/DT/BCW)	Initial QMED (rural) from CDs (m ³ /s)	Donors used (NRFA numbers)	Donor distances from subject centroid (km)	Individual donor weights	Combined and weighted donor adjustment factor	Urban adjustment factor	Final QMED (m ³ /s)
ESUS	DT	2.100	40035	7.15	1.22		1.028	2.561
TRIB1	DT	1.280	40035	3.08	1.30		1.047	1.669
TRIB2	DT	1.205	40035	5.27	1.24		1.032	1.499
TRIB3	DT	1.171	40021	31.44	1.11		1.000	1.297
ESDS	DT	6.006	40035	5.05	1.25	1.19	1.024	7.166
			40011	9.32	1.02			
			40022	18.57	1.02			

Methods: CD - Catchment descriptors alone; DT - catchment descriptors with donor transfer; BCW - catchment descriptors with bankfull channel width.



4.2.2 Urban adjustment of QMED

Urban adjustment was undertaken based on the methodology developed by Kjeldsen (2014)⁵ as part of WINFAP v5⁶. Urban adjustment was undertaken using the default parameters of:

- Impervious fraction for built-up areas, IF – 0.3
- Percentage runoff for impervious surfaces, PR_{imp} – 70%
- Urban cover taken from URBEXT2000.

4.2.3 Search for donor sites

4.2.3.1 Old Mill Stream at Aylesford – 40035

Is marked by NRFA as suitable for QMED but not pooling. The Aylesford gauge is the closest local gauge to the Site. The Old Mill Stream catchment at the location of the gauge is highly similar to the ESUS, TRIB1, TRIB2 and ESDS catchments. The catchment area is 17.96 km², with similar geology (Lower Chalk and Gault formation in the headwaters, Sedimentary bedrocks consisting of Sandstone, Siltstone and Mudstone in the lower catchment). Highly similar SAAR, BFIHOST19 and FARL values. The AMAX data suggests catchment descriptors underestimate QMED significantly, this maybe due to the BFIHOST19 value being high for a catchment predominantly underlain by intergranularly permeable bedrock. Given the similarities of the catchment and descriptors to ESUS, TRIB1, TRIB2 and ESDS it is expected that catchment descriptors underestimate QMED by a similar ratio.

4.2.3.2 Great Stour at Horton – 40011

Is marked by NRFA as suitable for QMED and pooling. Less similar to ESUS, TRIB1, TRIB2 and ESDS than 40035 due to a greater proportion and lower extent of the catchment being underlain by Chalk. However, suitable for use in a multiple donor approach.

4.2.3.3 Dour at Crabble Mill – 40033

Is marked by NRFA as suitable for QMED but not pooling. Catchment is a chalk stream with significantly higher SAAR and BFIHOST19 values and therefore is not considered suitable for donor transfer to the target catchments.

4.2.3.4 Great Stour at Chart Leacon – 40022

Is marked by NRFA as suitable for QMED but not pooling. Less similar to ESUS, TRIB1, TRIB2 and ESDS than 40035 as the catchment is expected to be more groundwater influenced. Similar geology although higher proportion underlain by highly permeable bedrocks. Similar catchment descriptors. Considered suitable for multiple donor approach.

4.2.3.5 Beult at Stilebridge – 40005

Is marked by NRFA as suitable for QMED and pooling. Catchment is underlain by clay and has a significantly lower BFIHOST19 value to the target catchments and therefore is not considered suitable for donor transfer.

⁵ Kjeldsen, T.R, Jones. D. A., and Morris, D. G. (2014). Using multiple donor sites for enhanced flood estimation in ungauged catchments, *Water Resour. Res.*, 50, 6646–6657, doi:10.1002/ 2013WR015203.

⁶ Wallingford HydroSolutions (2023). WINFAP 5 Urban adjustment procedures.



4.2.3.6 Hexden Channel at Hopemill – 40021

Is marked by NRFA as suitable for QMED but not pooling. Good match for SAAR and FARL to TRIB3 and ok match for BFIHOST19. Catchment is underlain by similar geology to TRIB3 consisting of predominantly clay bedrock with intergranularly permeable sedimentary bedrocks. Considered suitable for donor adjustment for TRIB3 only.

4.2.3.7 Teise at Stonebridge – 40009

Is marked by NRFA as suitable for QMED and pooling. Significant difference in FARL value to target catchments. Not considered suitable for donor transfer due to high level of attenuation compared to target catchments.

4.2.3.8 Rother at Udiam – 40004

Is marked by NRFA as suitable for QMED but not pooling. Significantly higher SAAR value to target catchments. Not considered suitable for donor transfer due to higher amount of rainfall in catchment.

4.2.3.9 Medway at Teaston/East Farleigh – 40003

Is marked by NRFA as suitable for QMED and pooling. Similar catchment to TRIB3 however less suitable than 40021, as the ‘small catchment⁷’ methodology will be applied to TRIB3 only one donor station will be used. Therefore not considered suitable for donor adjustment.

4.2.3.10 Cuckmere at Cowbeech – 41016

Is marked by NRFA as suitable for QMED but not pooling. Significantly higher SAAR value to target catchments. Not considered suitable for donor transfer due to higher amount of rainfall in catchment.

4.2.4 Donor sites chosen and QMED adjustment factors

Table 4-1: Donor sites chosen and QMED adjustment factors

NRFA no.	Method (AM/POT/LF)	Adjustment for climatic variation?	QMED from flow data (m ³ /s)	De-urbanised QMED from flow data (m ³ /s) (A)	QMED from catchment descriptors (m ³ /s) (B)	Adjustment ratio (A/B)
40035	AM	No	4.12	3.90	2.42	1.611
40021	AM	No	14.43	14.26	9.41	1.516
40011	AM	No	20.49	19.31	18.43	1.048
40022	AM	No	5.13	4.82	4.46	1.081

Methods: AM – Annual maxima; POT – Peaks over threshold; LF – Low flow (flow duration curve) statistics.

⁷ Stewart, L., Faulkner, D., Formetta, F., Griffin, A., Haxton, T., Prosdocimi, I., Vesuviano, G., Young, A. (2019). Estimating flood peaks and hydrograph for small catchments (Phase 2). Report SC090031/R0, Environment Agency.



4.3 Estimating Growth Curves

4.3.1 Derivation of growth curves at subject sites

Table 4-2: Derivation of growth curves at subject sites

Site code	Method (SS, P, ESS, H.)	If P or ESS, name of pooling group	Distribution used and reason for choice	Any urban or non-flood years adjustments	Parameters of distribution (location, scale and shape after adjustments)	Growth factor for 100-year return period
ESUS	P	ESUS	Generalised Logistic (GL) as the distribution is used in permeable adjustment	Permeable adjustment applied	Location: 1 Scale: 0.288 Shape: -0.214 Bound: -0.346	3.266
TRIB1	P	TRIB1	Generalised Logistic (GL) as the distribution is used in permeable adjustment	Permeable adjustment applied	Location: 1 Scale: 0.299 Shape: -0.233 Bound: -0.286	3.479
TRIB2	P	TRIB2	Generalised Logistic (GL) as the distribution is used in permeable adjustment	Permeable adjustment applied	Location: 1 Scale: 0.300 Shape: -0.219 Bound: -0.368	3.391
TRIB3	P	TRIB3	Generalised Logistic (GL) as the distribution is used in permeable adjustment	Permeable adjustment applied	Location: 1 Scale: 0.297 Shape: -0.251 Bound: -0.183	3.309
ESDS	P	ESDS	Generalised Logistic (GL) as the distribution is used in permeable adjustment	Permeable adjustment applied	Location: 1 Scale: 0.270 Shape: -0.128 Bound: -1.108	2.683

Methods: SS - Single Site; P - Pooled; ESS - Enhanced Single Site; H - Historical. Pooled and ESS growth curves were derived using the procedures from Science Report SC050050 (2008). Urban adjustments are carried out using the method of Kjeldsen (2010).



4.3.2 Flood frequency curve plots

Figure 4-1: ESUS Final Non-Flood Year Adjusted Growth Curve



Figure 4-2: TRIB1 Final Non-Flood Year Adjusted Growth Curve

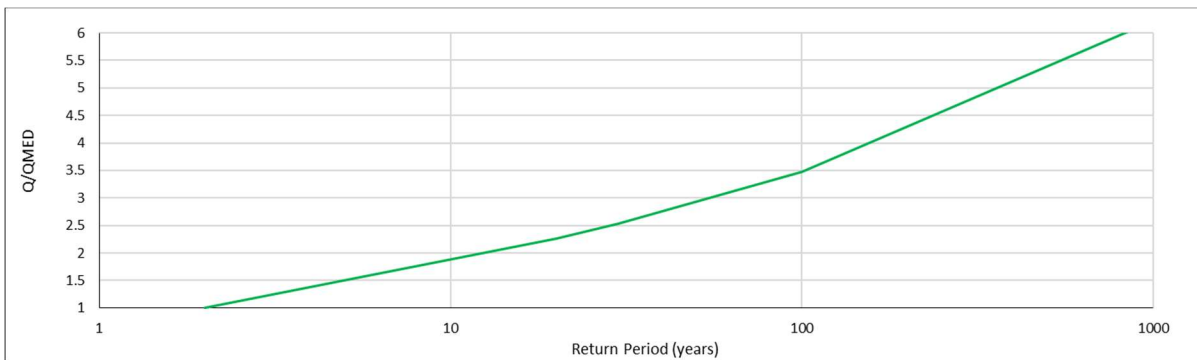


Figure 4-3: TRIB2 Final Non-Flood Year Adjusted Growth Curve

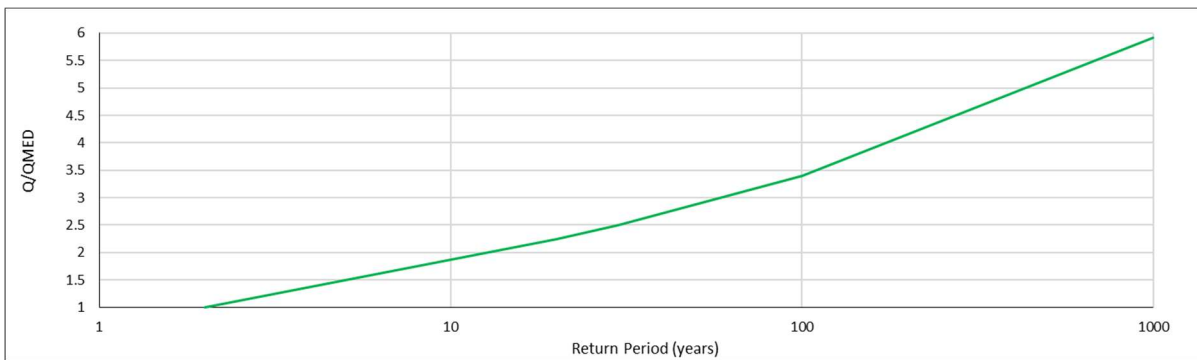


Figure 4-4: TRIB3 Final Non-Flood Year Adjusted Growth Curve

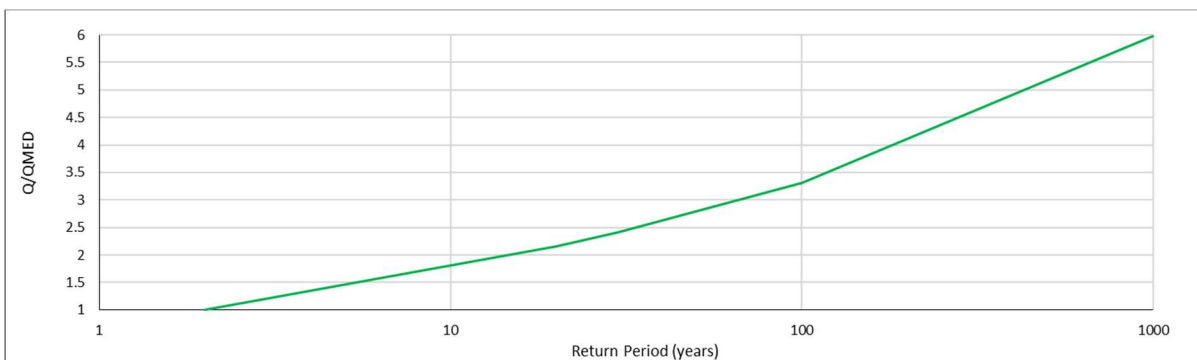
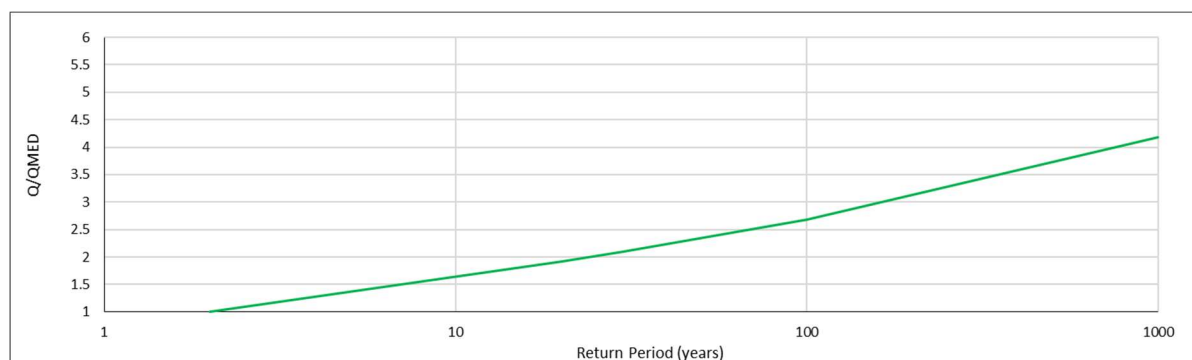


Figure 4-5: ESDS Final Non-Flood Year Adjusted Growth Curve



4.3.3 Derivation of pooling groups

Table 4-3: Derivation of pooling groups

Name of group	Site code from whose descriptors group was derived	Subject site treated as gauged? (ESS)	URBEXT2000 threshold applied to pooling group selection?	L-moments deurbanised (including subject site for ESS)?	Small catchment pooling procedure applied?
ESUS	ESUS	No	0.04	L-CV: 0.271 L-Skew: 0.233	Yes
TRIB1	TRIB1	No	0.04	L-CV: 0.291 L-Skew: 0.238	Yes
TRIB2	TRIB2	No	0.04	L-CV: 0.290 L-Skew: 0.224	Yes
TRIB3	TRIB3	No	0.04	L-CV: 0.264 L-Skew: 0.254	Yes
ESDS	ESDS	No	0.04	L-CV: 0.238 L-Skew: 0.169	No

Methods: Unless otherwise stated, pooling groups were derived using the procedures from Science Report SC050050 (2008). The small catchment pooling procedure is given in the report on Phase 2 of project SC090031 (2021) and implemented in WINFAP v5.

4.3.4 Pooling group composition

Table 4-4: Pooling group composition

Name of group	Changes made to default pooling group, with reasons	Weighted average L-moments
ESUS	Removed 28058 (Henmore Brook at Ashbourne) due to gauge having a negative L-Skew. Added 33054 (Babingley at Castle Rising) to ensure pooling group is still greater than 500 years.	L-CV: 0.271 L-Skew: 0.233



Name of group	Changes made to default pooling group, with reasons	Weighted average L-moments
TRIB1	Removed 28058 (Henmore Brook at Ashbourne) due to gauge having a negative L-Skew. Added 39033 (Winterbourne Stream at Bangor) to ensure pooling group is still greater than 500 years.	L-CV: 0.291 L-Skew: 0.238
TRIB2	Removed 28058 (Henmore Brook at Ashbourne) due to gauge having a negative L-Skew. Added 39033 (Winterbourne Stream at Bangor) to ensure pooling group is still greater than 500 years.	L-CV: 0.290 L-Skew: 0.224
TRIB3	Removed 28058 (Henmore Brook at Ashbourne) due to gauge having a negative L-Skew. Added 44013 (Piddle at Little Puddle) to ensure pooling group is still greater than 500 years.	L-CV: 0.264 L-Skew: 0.254
ESDS	Removed 26003 (Fosten Beck at Fosten Mill) due to gauge having a negative L-Skew. Added 36004 (Chad Brook at Long Melford) to ensure pooling group is still greater than 500 years.	L-CV: 0.238 L-Skew: 0.169

4.4 Final Choice of QMED and Growth Curves

4.4.1 Method choice and reasons

Table 4-5: Method choice and reasons

Site code	Final choice of QMED and reasons	Final choice of flood growth curve method and reasons
ESUS	Data Transfer – Site is ungauged and there are suitable gauges in the local vicinity.	Pooled – Site is ungauged
TRIB1	Data Transfer – Site is ungauged and there are suitable gauges in the local vicinity.	Pooled – Site is ungauged
TRIB2	Data Transfer – Site is ungauged and there are suitable gauges in the local vicinity.	Pooled – Site is ungauged
TRIB3	Data Transfer – Site is ungauged and there are suitable gauges in the local vicinity.	Pooled – Site is ungauged
ESDS	Data Transfer – Site is ungauged and there are suitable gauges in the local vicinity.	Pooled – Site is ungauged



4.4.2 Final flood estimates from stationary statistical methods

Table 4-6: Final flood estimates from stationary statistical methods

Site code	20 5%	30 3.3%	100 1%	1000 0.1%
ESUS	5.70	6.34	8.60	15.09
TRIB1	3.97	4.43	6.08	10.86
TRIB2	3.47	3.86	5.25	9.16
TRIB3	2.80	3.13	4.29	7.76
ESDS	14.12	15.40	19.69	30.62

Flood peak in m³/s for the return periods in years or AEP (%) events.



5.0 Revitalised Flood Hydrograph 2 (ReFH2) Method

5.1 Method Overview

5.1.1 What is the purpose of applying this method?

ReFH2 will be applied at all the study catchments to provide an estimate of hydrograph shape and comparative peak flows via the rainfall runoff method. Additionally, ReFH2 will be used to extend out the flood frequency curve for the extreme 0.1% AEP event.

5.1.2 Rural and urban catchment sub-divisions

All catchments are essentially rural, catchment URBEXT2000 descriptors were reviewed against aerial imagery and found to be representative. As such standard equations for urbanisation were applied.

5.1.3 Version of ReFH2 applied

ReFH2 V4.0

5.2 Model Parameters

5.2.1 Summary of model parameters

Table 5-1: Summary of model parameters

Site code	Method	Tp (hours) rural	Cmax (mm)	BL (hours)	Area modelled as urban (km ²)	TP urban scaling factor	IF	IRF	DS
ESUS	CD	5.16	665.65	58.31	0.486	0.75	0.4	0.7	0.5
TRIB1	CD	3.50	529.63	47.17	0.368	0.75	0.4	0.7	0.5
TRIB2	CD	4.30	779.94	59.78	0.300	0.75	0.4	0.7	0.5
TRIB3	CD	2.83	337.91	34.79	0.000	0.75	0.4	0.7	0.5
ESDS	CD	6.46	580.04	61.05	1.244	0.75	0.4	0.7	0.5
IES1	DT from TRIB3	2.42	337.91	32.77	0.000	0.75	0.4	0.7	0.5
IES2	DT from TRIB3	2.55	337.91	33.45	0.000	0.75	0.4	0.7	0.5
ITRIB3	DT from TRIB3	2.07	337.91	30.89	0.000	0.75	0.4	0.7	0.5
ICULV1	PD	1.87	418.13	39.86	0.000	0.75	0.4	0.7	0.5
ICULV2	PD	2.13	418.13	41.28	0.000	0.75	0.4	0.7	0.5
ITRIB1a	PD	1.88	418.13	39.92	0.000	0.75	0.4	0.7	0.5
ITRIB1b	PD	1.59	418.13	38.12	0.000	0.75	0.4	0.7	0.5
ICHL	PD	1.57	418.13	38.01	0.000	0.75	0.4	0.7	0.5
ITRIB2a	PD	1.46	418.13	37.29	0.000	0.75	0.4	0.7	0.5
ITRIB2b	PD	1.36	418.13	36.56	0.000	0.75	0.4	0.7	0.5



Methods: OPT: Optimisation from event analysis, BR: Baseflow recession fitting, LAG: TP from lag analysis, CD: Catchment descriptors, DT: Data transfer, CAL: model calibration, PD: Point descriptors (uses plot scale equations).

5.2.2 Analysis undertaken to derive model parameters

For the three subcatchments where data transfer was used from TRIB3, the catchment area was updated to match the subcatchment delineation which can be seen in **HR Figure 2**. Additionally, DPLBAR was adjusted so that $\log_{AREA}(DPLBAR)$ was the same for the subcatchments and TRIB3.

5.3 Model Inputs for Design Events

5.3.1 Design events for lumped catchments

Table 5-2: Design events for lumped catchments

Site Code	Rainfall DDF Model	Urban or rural	Highly permeable?	Season of design event	Storm duration (hrs)	Initial soil moisture Cini	Initial baseflow BFO
ESUS	FEH22	Urban	Yes	Winter	24.25	71.40	0.223
TRIB1	FEH22	Urban	Yes	Winter	17.25	82.92	0.132
TRIB2	FEH22	Urban	Yes	Winter	24.25	64.38	0.102
TRIB3	FEH22	Urban	No	Winter	8.25	111.24	0.131
ESDS	FEH22	Urban	Yes	Winter	27.25	78.13	0.805

5.3.2 Design events for subcatchments and intervening areas

Table 5-3: Design events for subcatchments and intervening areas

Site code(s)	Rainfall DDF model	Season of design event	Storm duration (hrs)	Areal reduction factor ARF	Reason for selecting storm
IES1	FEH22	Winter	7.25	0.975	To test critical duration storm
IES2				0.974	
ITRIB3				0.980	
ICULV1				0.985	
ICULV2				0.982	
ITRIB1a				0.985	
ITRIB1b				0.988	
ICHL				0.988	
ITRIB2a				0.989	
ITRIB2b				0.990	
IES1				FEH22	
IES2	0.975				
ITRIB3	0.981				
ICULV1	0.986				



Site code(s)	Rainfall DDF model	Season of design event	Storm duration (hrs)	Areal reduction factor ARF	Reason for selecting storm
ICULV2				0.983	
ITRIB1a				0.985	
ITRIB1b				0.988	
ICHL				0.988	
ITRIB2a				0.989	
ITRIB2b				0.990	
IES1	FEH22	Winter	17.25	0.982	Critical storm duration for TRIB1
IES2				0.981	
ITRIB3				0.985	
ICULV1				0.989	
ICULV2				0.987	
ITRIB1a				0.989	
ITRIB1b				0.991	
ICHL				0.991	
ITRIB2a				0.992	
ITRIB2b				0.993	
IES1				FEH22	
IES2	0.983				
ITRIB3	0.987				
ICULV1	0.990				
ICULV2	0.989				
ITRIB1a	0.990				
ITRIB1b	0.992				
ICHL	0.992				
ITRIB2a	0.993				
ITRIB2b	0.993				
IES1	FEH22	Winter	27.25		0.985
IES2				0.984	
ITRIB3				0.988	
ICULV1				0.991	
ICULV2				0.989	
ITRIB1a				0.991	
ITRIB1b				0.992	
ICHL				0.992	
ITRIB2a				0.993	
ITRIB2b				0.994	



Site code(s)	Rainfall DDF model	Season of design event	Storm duration (hrs)	Areal reduction factor ARF	Reason for selecting storm
IES1	FEH22	Winter	36.25	0.986	To test critical duration of defended scenario
IES2				0.986	
ITRIB3				0.989	
ICULV1				0.992	
ICULV2				0.990	
ITRIB1a				0.992	
ITRIB1b				0.993	
ICHL				0.993	
ITRIB2a				0.994	
ITRIB2b				0.994	
IES1				FEH22	
IES2	0.987				
ITRIB3	0.990				
ICULV1	0.992				
ICULV2	0.991				
ITRIB1a	0.992				
ITRIB1b	0.994				
ICHL	0.994				
ITRIB2a	0.994				
ITRIB2b	0.995				
IES1	FEH22	Winter	60.25		0.989
IES2				0.988	
ITRIB3				0.991	
ICULV1				0.993	
ICULV2				0.992	
ITRIB1a				0.993	
ITRIB1b				0.994	
ICHL				0.994	
ITRIB2a				0.995	
ITRIB2b				0.995	
IES1				FEH22	Winter
IES2	0.989				
ITRIB3	0.991				
ICULV1	0.993				
ICULV2	0.992				
ITRIB1a	0.993				



Site code(s)	Rainfall DDF model	Season of design event	Storm duration (hrs)	Areal reduction factor ARF	Reason for selecting storm				
ITRIB1b				0.995					
ICHL				0.995					
ITRIB2a				0.995					
ITRIB2b				0.996					
IES1	FEH22	Winter	84.25	0.990	To test critical duration of defended scenario				
IES2				0.990					
ITRIB3				0.992					
ICULV1				0.994					
ICULV2				0.993					
ITRIB1a				0.994					
ITRIB1b				0.995					
ICHL				0.995					
ITRIB2a				0.995					
ITRIB2b				0.996					
IES1				FEH22		Winter	96.25	0.991	To test critical duration of defended scenario
IES2								0.990	
ITRIB3	0.992								
ICULV1	0.994								
ICULV2	0.993								
ITRIB1a	0.994								
ITRIB1b	0.995								
ICHL	0.995								
ITRIB2a	0.996								
ITRIB2b	0.996								

5.3.3 Storm duration testing

Storm duration testing was completed on the hydraulic model. The results of which are summarised below:

5.3.3.1 Defended Scenario

- 5% AEP (20yr RP) – 96.25hr event critical.
- 3.3% AEP (30yr RP) – 96.25hr event critical.
- 1% AEP (100yr RP) – 60.25hr event critical.
- 1% AEP + 30% CC (higher central) – 48.25hr event critical.
- 1% AEP + 55% CC (upper end) – 36.25hr event critical.
- 0.1% AEP (1000yr RP) – 27.25hr event critical.



5.3.3.2 Undefended Scenario

- 5% AEP (20yr RP) – 17.25hr event critical.
- 1% AEP (100yr RP) – 24.25hr event critical.
- 0.1% AEP (1000yr RP) – 24.25hr event critical.

5.4 Final Choice of ReFH2 Flow Estimates

5.4.1 Method choice and reasons

Table 5-4: Method choice and reasons

Site code	Final choice of design inputs and model parameters
ESUS	Catchment descriptors, default model parameters.
TRIB1	Catchment descriptors, default model parameters.
TRIB2	Catchment descriptors, default model parameters.
TRIB3	Catchment descriptors, default model parameters.
ESDS	Catchment descriptors, default model parameters.
IES1	Donor transfer from TRIB3, Area and DPLBAR adjusted.
IES2	Donor transfer from TRIB3, Area and DPLBAR adjusted.
ITRIB3	Donor transfer from TRIB3, Area and DPLBAR adjusted.
ICULV1	Point descriptor, catchment area manually derived using LiDAR data
ICULV2	Point descriptor, catchment area manually derived using LiDAR data
ITRIB1a	Point descriptor, catchment area manually derived using LiDAR data
ITRIB1b	Point descriptor, catchment area manually derived using LiDAR data
ICHL	Point descriptor, catchment area manually derived using LiDAR data
ITRIB2a	Point descriptor, catchment area manually derived using LiDAR data
ITRIB2b	Point descriptor, catchment area manually derived using LiDAR data

5.4.2 Final flood estimates from ReFH2 method

Table 5-5: Final flood estimates from ReFH2 method

Site code	20 5%	30 3.3%	100 1%	1000 0.1%
ESUS	3.59	3.99	6.14	12.68
TRIB1	2.30	2.54	3.75	7.54
TRIB2	2.10	2.34	3.69	7.69
TRIB3	2.59	2.82	3.71	6.95
IES1	1.56	1.69	2.15	3.73
IES2	1.80	1.95	2.47	4.30
ITRIB3	0.94	1.03	1.29	2.17
ICULV1	0.32	0.35	0.44	0.75



Site code	20 5%	30 3.3%	100 1%	1000 0.1%
ICULV2	0.46	0.50	0.64	1.10
ITRIB1a	0.33	0.35	0.45	0.76
ITRIB1b	0.19	0.21	0.27	0.45
ICHL	0.19	0.21	0.26	0.43
ITRIB2a	0.15	0.17	0.21	0.35
ITRIB2b	0.12	0.13	0.17	0.27
ESDS	10.13	11.29	17.54	35.16

Flood peak in m³/s for the return periods in years or AEP (%) events.



6.0 Discussion and Summary of Results

6.1 Comparison of Results from Different Methods

Site code	Ratio of stationary statistical peak to ReFH2, 50% AEP	Ratio of stationary statistical peak to ReFH2, 1% AEP	Ratio of ReFH2 0.1% AEP to ReFH2 1% AEP
ESUS	1.589	1.401	2.064
TRIB1	1.727	1.620	2.007
TRIB2	1.653	1.423	2.084
TRIB3	1.083	1.158	1.876
ESDS	1.394	1.122	2.005

6.2 Final Choice of Method

6.2.1 Choice of method and reasons

6.2.1.1 Lumped Catchments

Due to the availability of gauge data for hydrologically similar catchments in the proximity of the study area which are suitable for QMED adjustment, the statistical method is deemed the most appropriate estimate of peak flows for return periods up to and including the 1% AEP event.

For the permeable catchments within the study, the catchment descriptors BFIHOST19 values are relatively high given the predominant underlying geology, however the same relationship is shown by the donor catchment (Old Mill Stream at Aylesford – 40035) used for the QMED adjustment and as such accounts for this uncertainty.

As hydrographs are required for the hydraulic model, the ReFH2 hydrograph shapes will be scaled so that the urbanised winter critical storm duration matches the final peak flow estimates from the statistical method.

6.2.1.2 Subcatchments and intervening areas

Due to the subcatchments and intervening areas having different catchment properties in particular geology to the upstream lumped catchments, the flow estimates using ReFH2 are considered most suitable. The final flows at the downstream extent of the hydraulic model have been reviewed against the final peak flow estimates for ESDS.

6.2.1.3 How will the 0.1% AEP flows be estimated?

For the lumped catchments, due to the greater availability of long rainfall records and spatial consistency of extreme rainfall events and therefore greater confidence in rainfall frequency curves for large return periods. A hybrid approach will be used for the 0.1% AEP event, with the ReFH2 0.1% to 1% AEP event ratio applied to the statistical 1% AEP event peak flow.

6.2.1.4 How will the flows be applied to a hydraulic model?

ESUS, TRIB1, TRIB2 and TRIB3 lumped flow estimates will be applied at the upstream extents of the model for their respective watercourses, the subcatchment flow estimates will be applied via distributed lateral inflows. Further details to the application of the inflow boundaries in the hydraulic model is provided in the hydraulic modelling report.



6.3 Final Results

Site code	20 5%	30 3.3%	100 1%	1000 0.1%
ESUS	5.70	6.34	8.60	17.76
TRIB1	3.97	4.43	6.08	12.21
TRIB2	3.47	3.86	5.25	10.93
TRIB3	2.80	3.13	4.29	8.05
IES1	1.56	1.69	2.15	3.73
IES2	1.80	1.95	2.47	4.30
ITRIB3	0.94	1.03	1.29	2.17
ICULV1	0.32	0.35	0.44	0.75
ICULV2	0.46	0.50	0.64	1.10
ITRIB1a	0.33	0.35	0.45	0.76
ITRIB1b	0.19	0.21	0.27	0.45
ICHL	0.19	0.21	0.26	0.43
ITRIB2a	0.15	0.17	0.21	0.35
ITRIB2b	0.12	0.13	0.17	0.27
ESDS	14.12	15.40	19.69	39.46

Critical duration storm flood peak in m³/s for the return periods in years or AEP (%) events.

6.3.1 Climate change allowances

Given the Project will have an operational lifetime of 40 years. The peak river flow climate change allowances for the 2050s (2040-2069) epoch for the Stour Management Catchment have been applied. As the project is classified as essential infrastructure the 30% higher central allowance has been applied in line with the Environment Agencies guidance on climate change allowances for flood risk assessments⁸. Additionally, as the project is classified as nationally significant infrastructure project the 55% upper end allowance has been applied as a credible maximum climate change scenario.

Climate change allowances have been applied by scaling the ReFH2 1% AEP hydrographs to the peak flow estimate from the statistical 1% AEP plus the relevant climate change allowance.

6.4 Checks

6.4.1 Growth factor checks

Table 6-1: Growth factor checks

Site code	1% AEP growth factor	0.1% AEP / 1% AEP ratio
ESUS	3.266	2.064
TRIB1	3.479	2.007

⁸ <https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances>



Site code	1% AEP growth factor	0.1% AEP / 1% AEP ratio
TRIB2	3.391	2.084
TRIB3	4.291	1.876
ESDS	2.683	2.005

6.4.2 Specific discharge

Table 6-2: Specific discharge

Site code	20 5%	30 3.3%	100 1%	1000 0.1%
ESUS	2.93	3.25	4.41	9.11
TRIB1	5.54	6.19	8.50	17.06
TRIB2	2.70	3.00	4.08	8.50
TRIB3	7.54	8.42	11.55	21.67
IES1	7.37	7.99	10.16	17.63
IES2	7.08	7.67	9.71	16.91
ITRIB3	7.79	8.54	10.70	17.99
ICULV1	6.16	6.68	8.46	14.41
ICULV2	5.60	6.08	7.71	13.30
ITRIB1a	6.13	6.65	8.42	14.34
ITRIB1b	6.69	7.28	9.21	15.45
ICHL	6.74	7.35	9.28	15.56
ITRIB2a	7.05	7.70	9.72	16.27
ITRIB2b	7.19	7.84	9.88	16.29
ESDS	2.72	2.97	3.79	7.60

Flood peak in l/s/ha for the return periods in years or AEP (%) events.

6.4.3 Spatial consistency of results

In general, for the lumped catchments the growth factors are shown to be spatially consistent, with a higher growth factor seen for the 1% AEP event for TRIB3. Given the lower permeability of TRIB3 a higher growth factor is to be expected due to the higher runoff ratio. For the 0.1% AEP to 1% AEP ratio, again spatial consistency is observed for the lumped catchments, with a lower ratio seen for TRIB3. This too is to be expected for a lower permeability catchment with soils likely to be closer to fully saturated during a 1% AEP event and as such a lower difference in runoff ratio between the 1% AEP and 0.1% AEP events. The specific discharge shows similar variation with permeability, with the less permeable TRIB1 and TRIB3 catchments having higher specific discharges.

For the subcatchments the specific discharges are shown to be spatially consistent with little variation, this is to be expected given the similar geology of these catchments. Additionally, the specific discharges for the subcatchments are consistent with the lower permeability lumped catchments which also have similar geology.



6.4.4 Return periods for notable historic floods

Post event analysis has been undertaken by JBA in 2014 for the Autumn 2000⁹ and Winter 2013 – 2014¹⁰ events.

During the 4 – 9 November 2000 flood event, where the Aldington FSA overtopped the peak flow of 32.1m³/s at the Great Stour at Wye gauging station located approximately 16km downstream of the Site, was assessed to have an annual probability of occurrence of 3.23% (1 in 31 years). The peak flow at the South Willesborough gauge reached 17.3m³/s, which is located approximately 5km downstream of the Site. Adjusting for the approximate 13% increase in catchment area between the gauge and the downstream boundary of the model, a peak flow of 15.1m³/s is expected for a similar event.

During the Winter 2013 – 2014 events, where the Aldington FSA overtopped peak flood levels were gauged to reach approximately 50.3mAOD (100mm above the spillway crest level) on the 15th February with the spill level overtopped for approximately 15 hours. The event was assessed to have an annual probability of occurrence between 3.3% (1 in 30 years) and 6.7% (1 in 15 years) along the East Stour. The peak flow at the South Willesborough gauge reached 11.5m³/s on the 15th February. Adjusting for the increase in catchment area a peak flow of 10.15m³/s is expected at the downstream extent of the model for a similar event.

During the defended scenario modelling of the 3.3% AEP event for the critical duration 96.25 hour event, flood levels at the Aldington FSA reached approximately 50.3mAOD, with a peak flow of 10.15m³/s at the downstream extent of the model, showing good agreement with the historic flood data for the February 2014 event which was assessed to have a similar annual probability of occurrence.

6.4.5 Compatibility with longer-term flood history

The defended scenario modelling indicates that the Aldington FSA spillway is overtopped during longer duration (84.25 hours and longer) flood events with 5% (1 in 20 years) AEP, this shows good agreement with the longer-term flood history with the Aldington FSA overtopping during long duration winter storms in 2000 and 2014 which were assessed to have annual probabilities of occurrence between approximately 3.3% (1 in 30 years) and 6.6% (1 in 15 years).

6.4.6 Comparisons with previous studies

Preliminary hydrological analysis of the Upper Stour catchment undertaken by JBA in 2023¹¹ has been provided by the Environment Agency and includes flows into and out of the Aldington FSA. Design flows were calculated using Continuous Simulation Modelling (CSM) and compared below.

6.4.6.1 Defended Scenario

For the defended scenario, the peak flow estimates from JBA's CSM analysis into the Aldington FSA closest matched the peak flow estimates for the 27.25 hour duration event from this study and are compared in the table below.

As can be seen in the table below, the flows into the Aldington FSA for this study's hydraulic model are between 1.5% and 4.6% higher than those derived in JBA's CSM analysis for the

⁹ Autumn 2000 Great Stour Flood Rarity report (JBA, 2014)

¹⁰ 2013-2014 Post Flood Analysis: Kent and South London Area report (JBA, 2014)

¹¹ Upper Stour hydrological assessment by continuous simulation. Draft v3. JBA, May 2023.



27.25 hour duration event. This indicates that the flows into the Aldington FSA closely match the CSM analysis undertaken by JBA.

RP	AEP	JBA CSM analysis (m ³ /s)	SLR Hydraulic Model (m ³ /s)	Difference
20	5%	14.74	15.46	+4.6%
30	3.3%	16.54	17.32	+4.5%
100	1%	23.48	24.51	+4.2%
1000	0.1%	46.91	47.61	+1.5%

Peak flow estimates into the Aldington FSA from JBA’s CSM analysis and SLR’s hydraulic model constructed for this study.

Peak flow estimates out from JBA’s CSM analysis out of the Aldington FSA closest matched the peak flow estimates from the critical duration events from this study and are compared in the table below.

As can be seen in the table below, the flows out of the Aldington FSA for this study’s hydraulic model are 2.0% and 3.2% lower for the 0.1% AEP and 1% AEP events respectively. For the 5% AEP and 3.3% AEP events, the study’s hydraulic model flows are 17.7% and 12.1% lower respectively. This indicates that the 1% AEP and 0.1% AEP flows for the critical duration flood event closely match the CSM analysis undertaken by JBA. For the 5% AEP and 3.3% AEP event, the critical duration 96.25 hour rainfall event was the longest duration event tested. Given that flooding within the catchment is highly sensitive to flood volumes and the historic flooding issues associated with long duration rainfall events this is likely an indicator that the critical duration event for the more probable 5% AEP and 3.3% AEP is longer than the maximum 96.25 hour rainfall event tested.

RP	AEP	SLR Storm Duration (Hours)	JBA CSM Analysis (m ³ /s)	SLR Hydraulic Model (m ³ /s)	Difference
20	5%	96.25	8.88	7.547	-17.7%
30	3.3%	96.25	11.03	9.841	-12.1%
100	1%	60.25	18.53	17.95	-3.2%
1000	0.1%	27.25	45.77	44.89	-2.0%

Peak flow estimates out of the Aldington FSA from JBA’s CSM analysis and SLR’s hydraulic model constructed for this study.

6.4.6.2 undefended Scenario

For the undefended scenario flows at the downstream extent of the model for the 24.25 hour rainfall event have been compared to JBA’s CSM flow estimates at the South Willesborough gauge. To account for the 11.7% reduction in catchment area at the downstream extent of the model compared to the South Willesborough gauge site the same reduction has been applied to JBA’s CSM flows at South Willesborough in the table below.

As can be seen in the table below, the SLR’s hydraulic model flows are between 4.8% lower to 7.3% higher than the flows estimated for the same location based on JBA’s CSM analysis. This indicates that the flows for the undefended scenario are broadly inline with the CSM analysis undertaken by JBA.



RP	AEP	JBA CSM Analysis (m ³ /s)	SLR Hydraulic Model (m ³ /s)	Difference
20	5%	17.04	17.92	+4.9%
100	1%	25.95	28.00	+7.3%
1000	0.1%	53.41	50.96	-4.8%

Peak flow at the downstream extent of SLR’s hydraulic model compared to estimated peak flow based on JBA’s CSM analysis.

6.4.7 Checks on hydraulic model results

The peak flows from the undefended scenario for the 24.25 hour rainfall event have been compared to the lumped flow estimate at ESDS in the table below.

RP	AEP	ESDS (m ³ /s)	SLR Hydraulic Model (m ³ /s)	Difference
20	5%	14.12	17.92	+21.2%
100	1%	19.69	28.00	+29.7%
1000	0.1%	39.46	50.96	+22.6%

Peak flow at the downstream extent of SLR’s hydraulic model compared to estimated peak flow based on JBA’s CSM analysis.

As can be seen, the peak flows in the hydraulic model are substantially higher than those estimated at ESDS. However, CSM analysis is generally considered to be more robust particularly for catchments which contain a mixture of low and high permeability geology and flood storage areas. Additionally, when deriving the inflow catchments into the hydraulic model, these were broken down into areas with similar geology, which better represents the variable permeability of the whole catchment. There is also good agreement between the hydraulic model peak flows to historic flood events assessed to have similar annual probabilities of occurrence. As such, there is greater confidence in the peak flows from the hydraulic model than those estimated at ESDS and no adjustment was undertaken to reconcile flows at the downstream extent of the model to the lumped estimate at ESDS.

6.5 Assumptions, limitations, and uncertainty

6.5.1 Assumptions

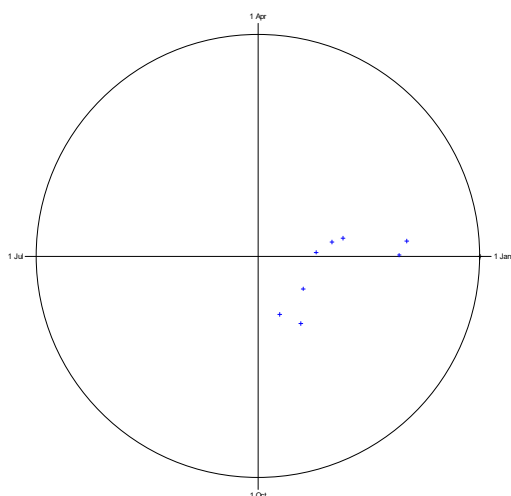
The main assumptions made are as follows:

- The FEH catchment descriptors for the permeable catchments significantly underestimate QMED due to the unusually high BFIHOST19 values for the predominant underlying geology.
- The seasonality of flooding events in the catchment area predominantly winter. The seasonality plots for the ESUS, TRIB1, TRIB2 and TRIB3 pooling groups can be seen below.
- The 0.1% AEP growth factors are best estimated for the rainfall-runoff approach, given that the confidence is greater in rainfall growth curves than in flood growth curves for longer return periods.



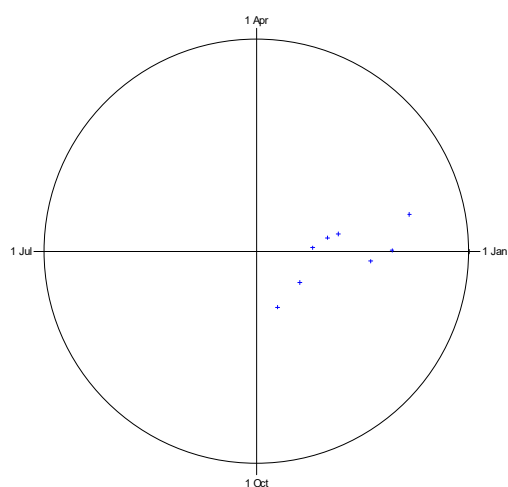
ESUS

Flood seasonality: TR 09400 37700 (27-07-2023 11:36) - ur...



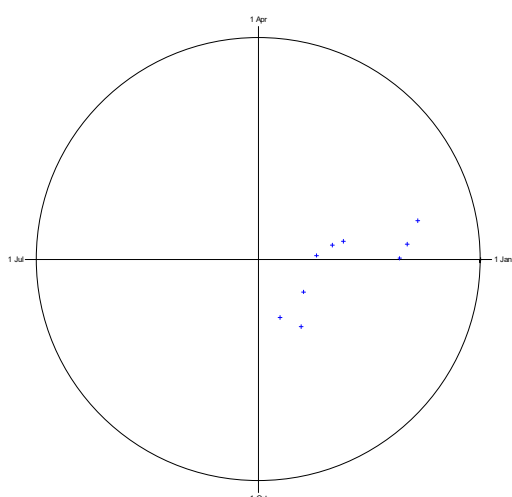
TRIB1

Flood seasonality: TR 08350 38850 (28-07-2023 09:23) - ru...



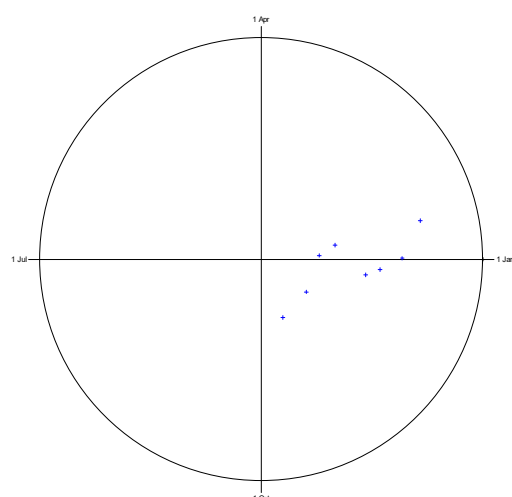
TRIB2

Flood seasonality: TR 08900 38550 (28-07-2023 10:08) - ur...



TRIB3

Flood seasonality: TR 07250 37150 (28-07-2023 10:31) - ur...



6.5.2 Limitations

Peak flow estimates do not account for the affect of the Aldington FSA and as such the flow estimates at ESDS are representative of the undefended scenario. Inline with this the gauging stations along the East Stour are not suitable for validation of peak flow estimates.

The longest duration rainfall event tested was the 96.25 hour event, with the longest duration event which ReFH2 can generate a hydrograph for being approximately 100 hours.

Information from historical flood events and JBA's CSM analysis indicate that the critical duration event for the 5% AEP and 3.3% AEP events are likely longer than the 96.25 hour event assessed.



6.5.3 Uncertainty

Table 6-3: Uncertainty

Site code	50% AEP Lower 95%	50% AEP Upper 95%	5% AEP Lower 95%	5% AEP Upper 95%	1% AEP Lower 95%	1% AEP Upper 95%	0.1% AEP Lower 95%	0.1% AEP Upper 95%
ESUS	1.84	3.74	3.93	8.21	5.94	12.56	11.90	26.46
TRIB1	1.10	2.76	2.38	6.58	3.53	10.46	6.59	22.71
TRIB2	1.08	2.20	2.39	4.99	3.62	7.66	7.32	16.29
TRIB3	0.91	1.84	1.93	4.04	2.96	6.27	5.39	11.99
ESDS	5.14	10.42	9.74	20.33	13.58	28.74	26.44	58.80

Upper and lower 95% confidence bounds for the flood peak in m³/s for the AEP (%) events.

6.5.4 Suitability of results for future studies

Suitable for the Site only as the hydrology has been targeted towards flows at the Site, and not locations upstream or downstream.

6.5.5 Recommendations for future work

The estimates are suitable for use in the study and no further work is recommended at this stage.





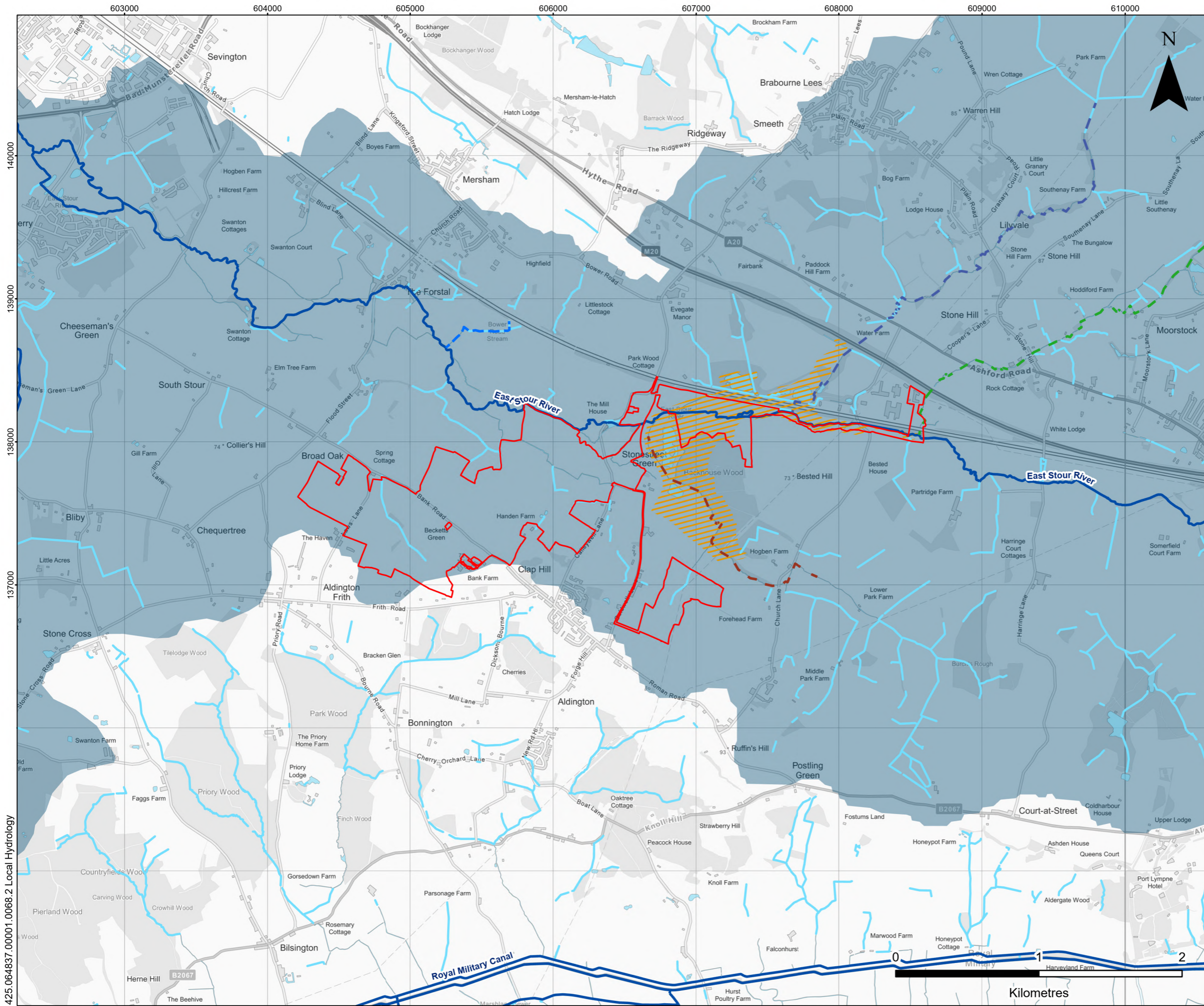
Figures

- HR1 Local Hydrology
- HR2 Catchment and Gauging Stations
- HR3 Aldington Flood Storage Area Extent
- HR4 Catchments and Sub catchments

Hydrology Report

Stonestreet Green Solar Farm

EPL 001 Limited

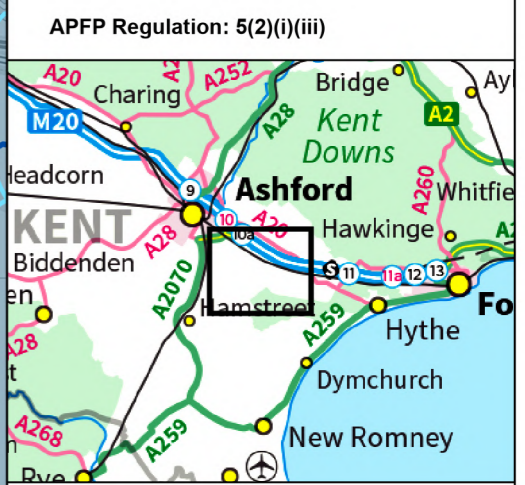


LEGEND

- Order limits
- Statutory Main River
- Watercourse
- Bower Road Stream
- Waterbody
- East Stour Catchment Area
- Environment Agency River and Sea Flood Storage Area

East Stour Internal Drainage Board - Drains

- Unnamed Tributary 1
- Unnamed Tributary 2
- Unnamed Tributary 3



STONESTREET GREEN SOLAR

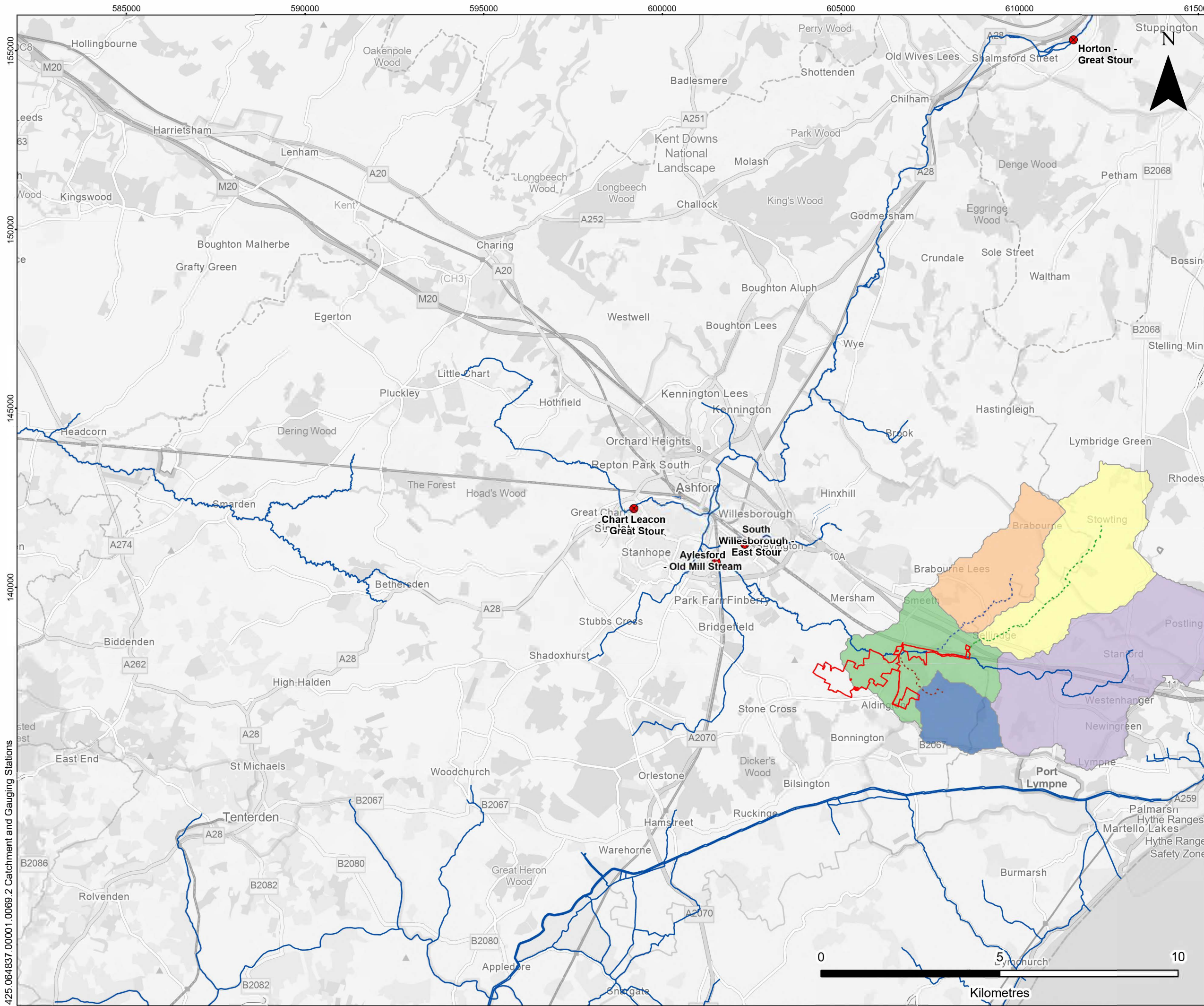
HYDROLOGY REPORT

LOCAL HYDROLOGY

HR Figure 1

Scale: 1:25,000 @ A3 Date: MAY 2024

425.064837.00001.0068.2 Local Hydrology



LEGEND

- Order limits
- Gauging Stations
- Statutory Main River

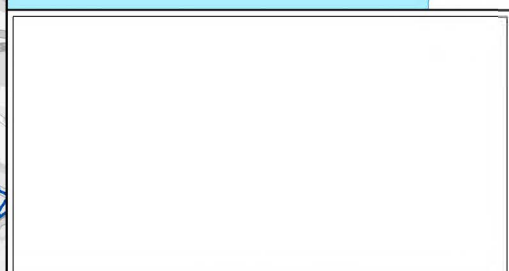
East Stour Internal Drainage Board - Drains

- Unnamed Tributary 1
- Unnamed Tributary 2
- Unnamed Tributary 3

Lumped Catchment

- ESDS
- ESUS
- TRIB1
- TRIB2
- TRIB3

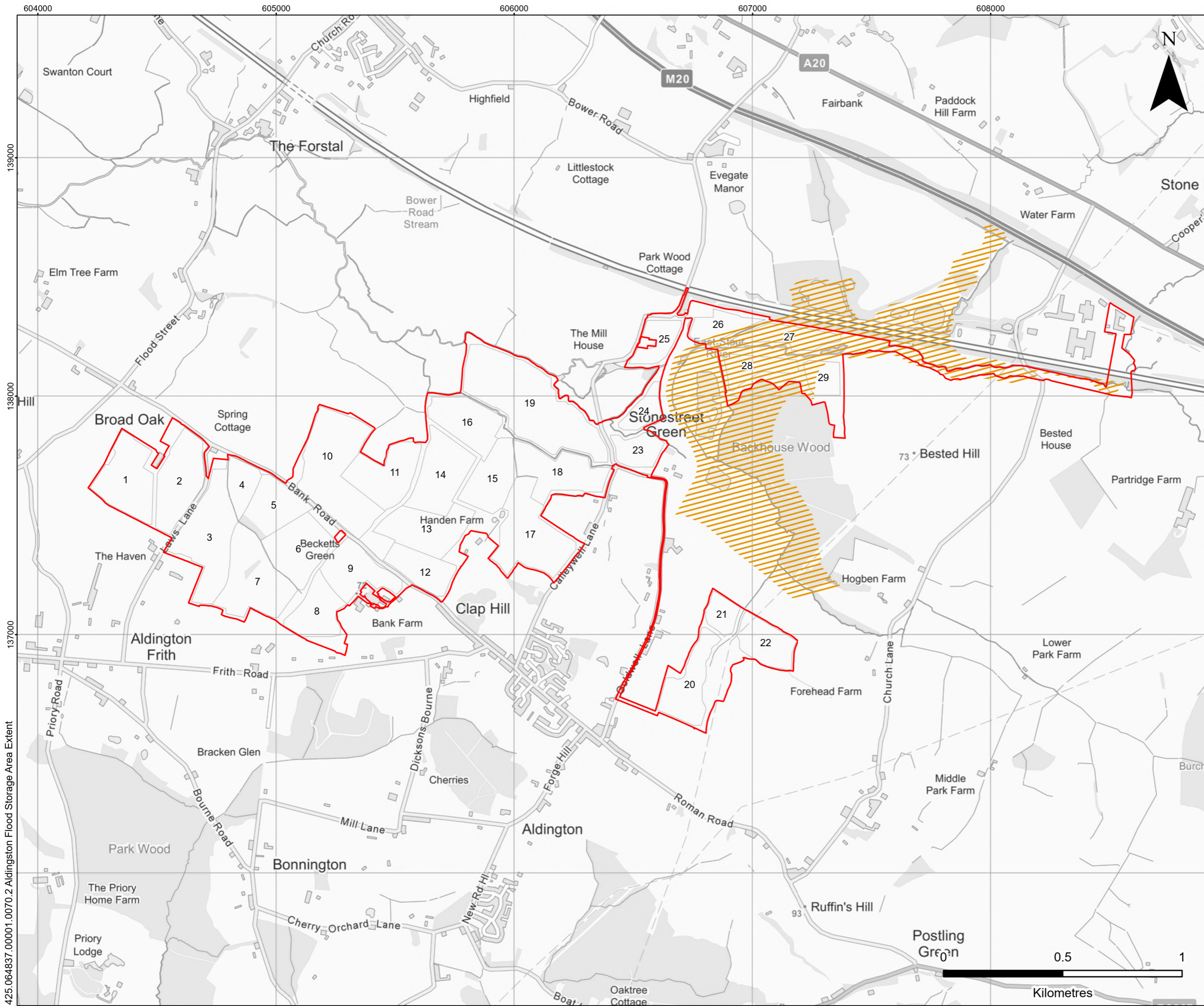
APFP Regulation: 5(2)(i)(iii)



STONESTREET GREEN SOLAR
HYDROLOGY REPORT
CATCHMENT AND GAUGING STATIONS

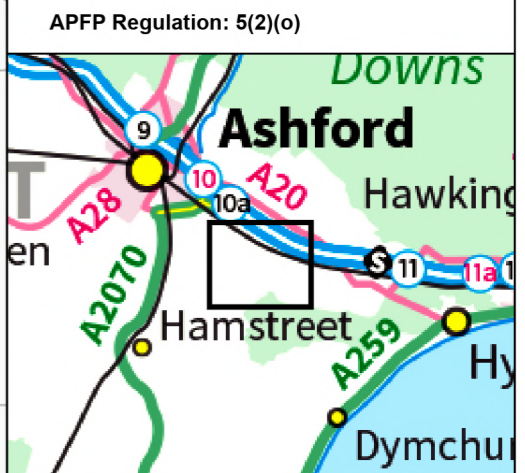
HR Figure 2

Scale 1:100,000 @ A3	Date MAY 2024
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LEGEND

- Order limits
- Field
- Environment Agency River and Sea Flood Storage Area

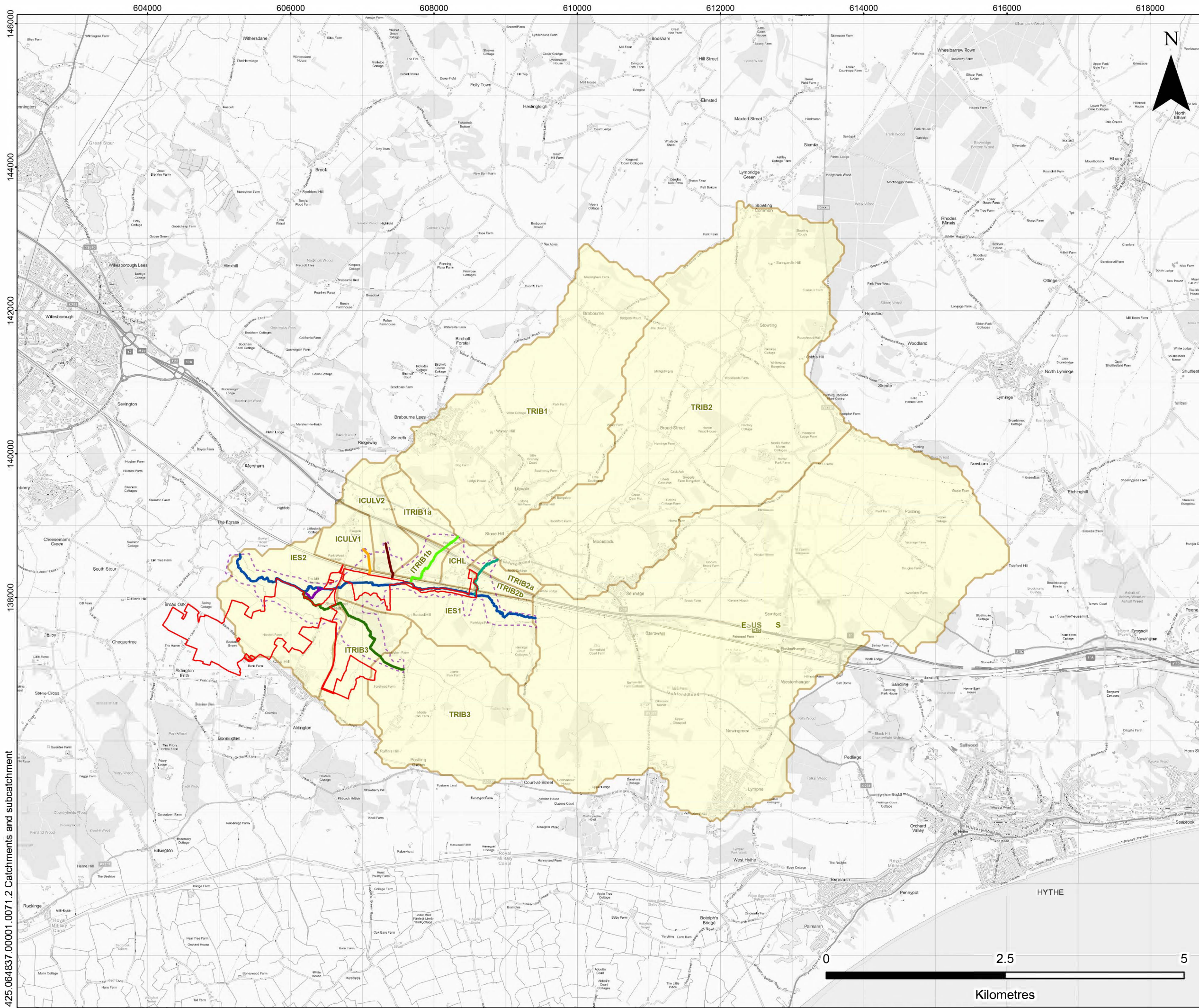


STONESTREET GREEN SOLAR
 HYDROLOGY REPORT
ALDINGTON FLOOD STORAGE AREA EXTENT

HR Figure 3

Scale: 1:15,000 @ A3 Date: MAY 2024

425.064837.00001.0070.2 Aldington Flood Storage Area Extent

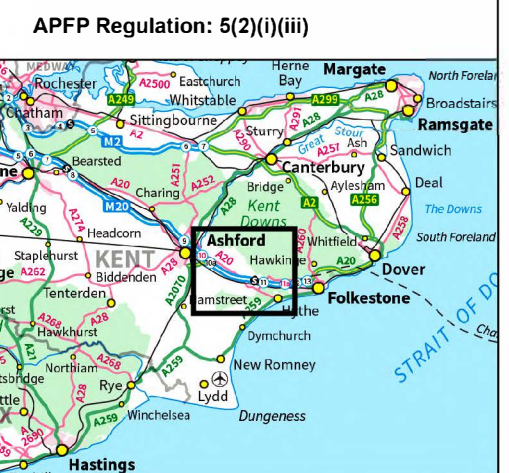


LEGEND

- Order limits
- Catchment
- Model Extent

Modelled Water Courses

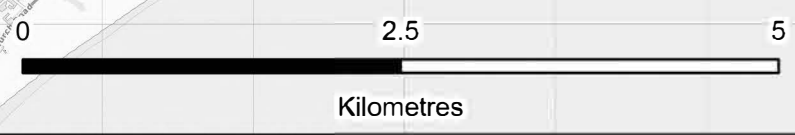
- Culvert 1
- Culvert 2
- East Stour River
- Mill Race
- Unnamed Tributary 1
- Unnamed Tributary 2
- Unnamed Tributary 3



**STONESTREET GREEN SOLAR
HYDROLOGY REPORT
CATCHMENT AND SUB-CATCHMENT**

HR Figure 4

Scale 1:50,000 @ A3 Date MAY 2024



425 064837.00001.0071.2 Catchments and subcatchment



Digital Files – Input Data

FEH_Catchment_Descriptors_609400_137700_v5_0_1.xml

FEH_Catchment_Descriptors_608350_138850_v5_0_1.xml

FEH_Catchment_Descriptors_608900_138550_v5_0_1.xml

FEH_Catchment_Descriptors_607250_137150_v5_0_1.xml

FEH_Catchment_Descriptors_605300_138600_v5_0_1.xml

FEH_Point_Descriptors_605897_137543_v5_0_1.xml

Hydrology Report

Stonestreet Green Solar Farm

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Digital Files – Project or Calculation Files

ReFH2_and_WINFAP_Design_Flow_Estimation_Summary_Sheet_v2.xlsx

Stat – WINFAP Projects and Non Flood Years Adjustment Spreadsheets

ReFH – ReFH Projects and CSV outputs

Hydrology Report

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Digital Files – Output Data

ESUS Scaled Hydrographs.xlsx
ICHL Scaled Hydrographs.xlsx
ICULV1 Scaled Hydrographs.xlsx
ICULV2 Scaled Hydrographs.xlsx
IES1 Scaled Hydrographs.xlsx
IES2 Scaled Hydrographs.xlsx
ITRIB1a Scaled Hydrographs.xlsx
ITRIB1b Scaled Hydrographs.xlsx
ITRIB2a Scaled Hydrographs.xlsx
ITRIB2b Scaled Hydrographs.xlsx
ITRIB3 Scaled Hydrographs.xlsx
TRIB1 Scaled Hydrographs.xlsx
TRIB2 Scaled Hydrographs.xlsx
TRIB3 Scaled Hydrographs.xlsx

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Other Supporting Information

Autumn 2000 Great Stour flood rarity. JBA, July 2014.

East Stour Flood Event Photo Reconnaissance. Environment Agency, November 2000.

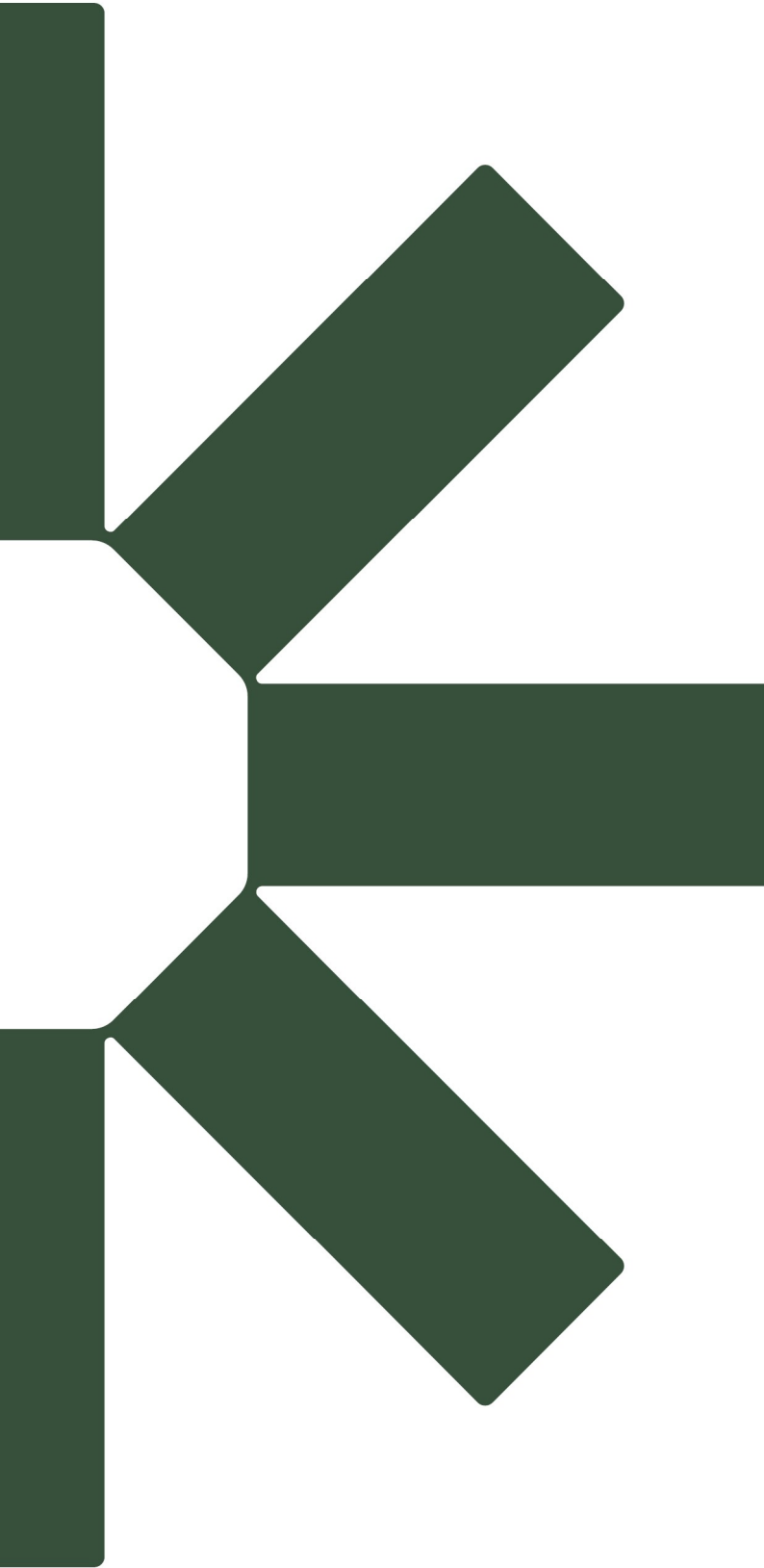
2013-2014 Post Flood Analysis: Kent & South London Area. JBA, December 2014.

East Stour Flood Event Photo Reconnaissance. Environment Agency, February 2014.

Hydrology Report

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Making Sustainability Happen

Annex E

Environment Agency Aldington FSA Information

Annex B: East Stour Hydraulic Modelling Report

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Aldington FSA - Modelling arrangements for JBA 2023 Ashford model update

Key notes regarding the Aldington FSA modelling arrangements for JBA 2023 Ashford model update:

FSA Outlets

- Compensation Outlet to Evegate Mill

1997 record drawings and inspection reports show an orifice plate (300mm dia) in a 600mm pipe, which has an upstream IL 46.6mAOD. The stage discharge curve with a max discharge of 0.34m³/s is shown on the chart titled "ALDINGTON FLOOD DETENTION STORAGE - Stage / Discharge and Stage / Storage Curves" with footnote "1966/X - Rofe, Kennard and Lapworth - Feb 1991".

- Main outlet (1500mm dia pipe) with hydrobrake

The hydrobrake design stage discharge curve is shown on the chart titled "ALDINGTON FLOOD DETENTION STORAGE - Stage / Discharge and Stage / Storage Curves" with footnote "1966/X - Rofe, Kennard and Lapworth - Feb 1991".

The file 'Aldington & Hothfield FSRs - Gaugings & hydrobrakes performance r1' includes the hydrobrake design stage discharge curve, spot flow gaugings downstream of the hydrobrake and an estimated actual stage discharge curve for the hydrobrake, which takes account of the spot flow gaugings and the inlet structure low weir at 44.65mAOD. The model update uses the estimated actual stage discharge curve, but there is uncertainty in it due to the limited spot flow gaugings.

Overflow spillway

The record drawings provide the co-ords and chainages for the spillway crest and transitions to the embankment flanks:-

Ch 370.6 - 380.6 10m transition from embankment flank to spillway crest at 50.2mAOD

Ch 380.6 - 680.6 300m spillway crest at 50.2mAOD

Ch 680.6 - 690.6 10m transition from spillway crest at 50.2mAOD to embankment flank

For modelling, the spillway design crest level at 50.2mAOD is used.

Potential future options for Ashford flood risk to keep pace with climate change

Further spot flow gaugings during FSA operation will be needed to refine the hydrobrake stage discharge curve. Potential future options for Ashford flood risk to keep pace with climate change could include modifying the FSA arrangements, including the outlet and overflow spillway arrangements, and also raising the embankment flanks.

Aldington Flood Storage Reservoir Information

Table 3-1 Key dimensions relating to reservoir and dam

Feature	Unit	Dimension	Source
Reservoir capacity	Mm ³	1.3	Reservoir Record
Reservoir area at TWL	Mm ²	0.77	
Invert of upstream channel	mOD	44.65	Drg RD10
Level of East Stour side weir	mOD	47.5 to 47.4	Drg RD9
Level to which floodwater may be “temporarily stored”, as certified in Final Certificate	mOD	50.2	Final Certificate
Embankment crest (non overflow)	mOD	51.3	Drg RD7
Downstream toe of embankment	mOD	46.8	Drg RD8
Invert of downstream channel	mOD	44.3	

3.1.2 Original Construction

The reservoir, in conjunction with the flood detention reservoir at Hothfield on the Great Stour west of Ashford, forms the Ashford Flood Alleviation Scheme. This scheme was completed in 1989 to reduce the frequency and intensity of flooding in both the rural and urban areas of the town and nearby villages. The works at Aldington were designed to reduce the flood flows from a peak of about 19m³/s for the 100-year event to just in excess of 4m³/s for the peak outflow by means of a hydrobrake with temporary detention of the additional volume for discharge over a larger timescale. The scheme was designed for floods in excess of the nominal 100-year event to overtop the embankment in a controlled manner.

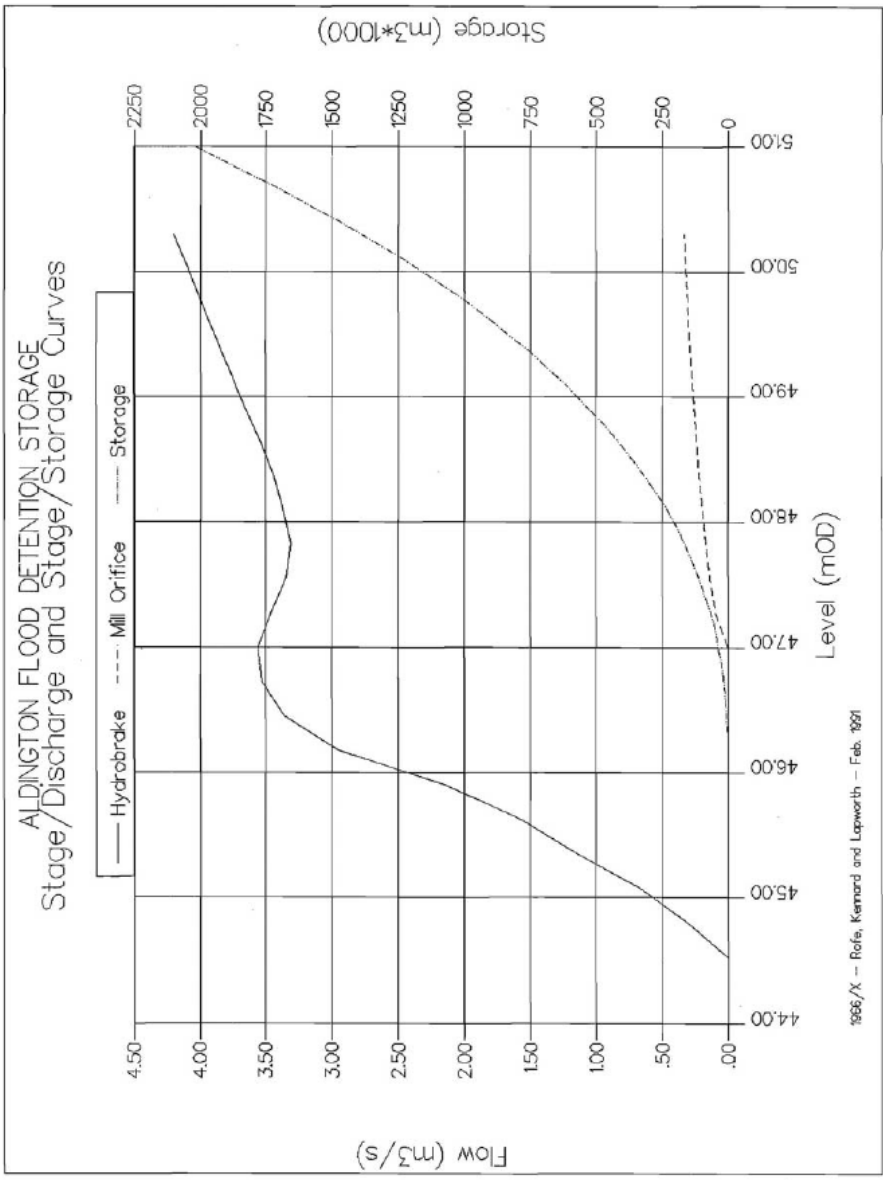
The reservoir is situated on the East Stour River and forms a flood detention area adjacent to the river and allied watercourses. The reservoir area is laid to pasture and is normally dry with flow through the area in the East Stour and Aldington Dyke watercourses. When full the reservoir occupies an area of about 1.5km x 1.5km.

3.7.1 General

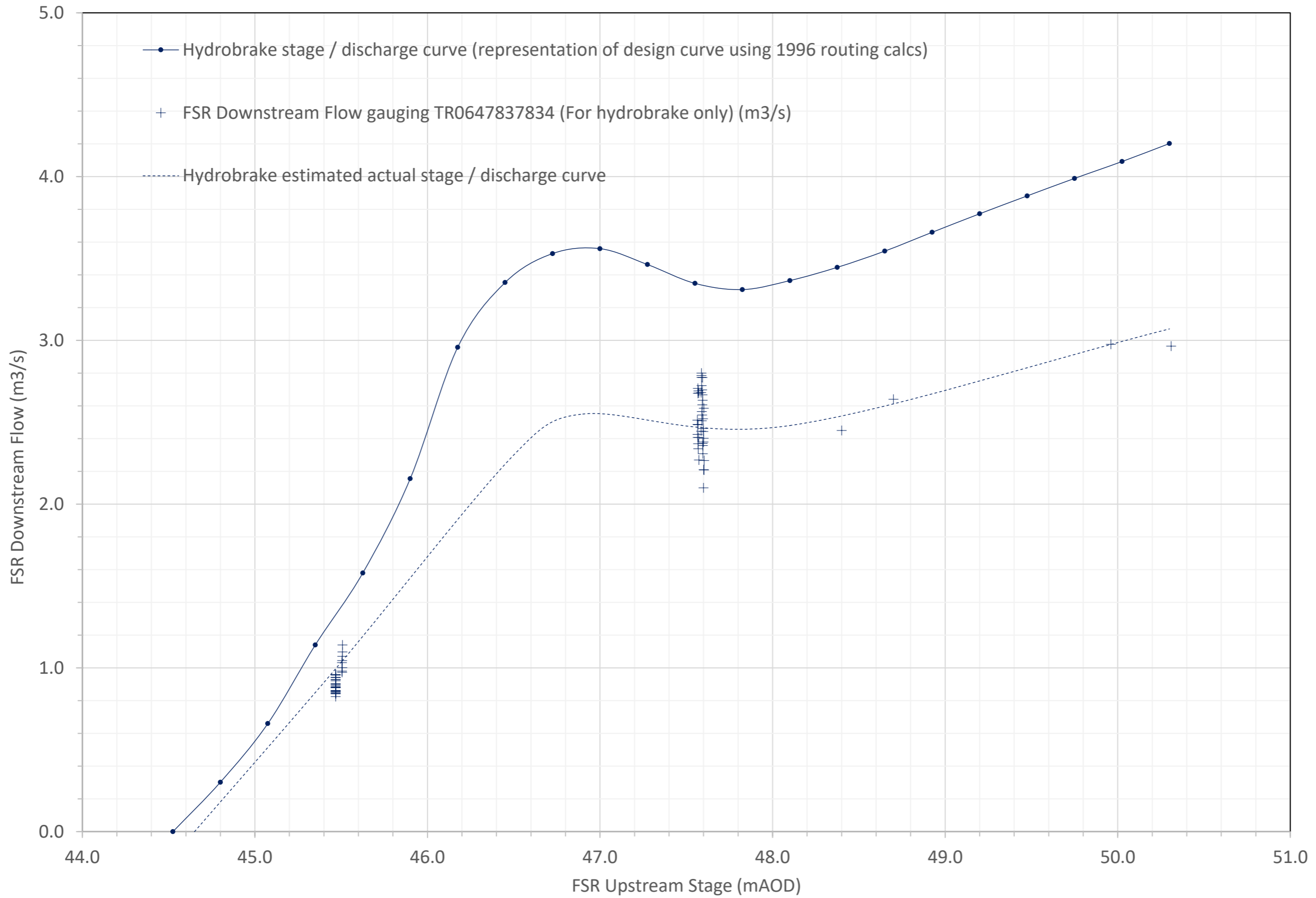
There are two outlets from the reservoir and one hydraulic structure to control inflows within the reservoir, as summarised in Table 3.6.

Table 3-6 Summary of outlets

Outlet	Control	Upstream invert level (m OD)	Other features which may control discharge (invert level)	Discharge (m ³ /s)
Main outlet (Aldington Dyke)	Hydrobrake	44.65	None	4.16 at WL of 50.2mOD – see figure in App. A
Compensation pipe & fish pass; East Stour	300mm dia orifice in 600mm DI pipe	46.6	None	0.34
East Stour side weir	Weir	47.4 to 47.5	Crest level set to ensure that the mill wheel can be operated and that it starts to spill when inflows exceed 0.12m ³ /s.	



Aldington FSR - Hydrobrake stage / discharge curve



Hydrobrake stage / discharge curve (representation of

Values from Aldington reservoir flood routing calcs 13/11/96 for 1252mm hydrobrake.

Note 1997 record dwgs also show 1252mm Type C conical hydrobrake.

Note curve visually checked that it matches curve on chart titled "ALDINGTON FLOOD DETENTION STORAGE - Stage / Discharge and Stage / Storage Curves" with footnote "1966/X - Rofe, Kennard and Lapworth - Feb 1991"

FSR Upstream Stage (mAOD)	FSR Downstream Flow (m3/s)
44.525	0.000
44.800	0.302
45.075	0.660
45.350	1.140
45.625	1.579
45.900	2.155
46.175	2.957
46.450	3.353
46.725	3.530
47.000	3.560
47.275	3.463
47.550	3.348
47.825	3.310
48.100	3.365
48.375	3.446
48.650	3.545
48.925	3.660
49.200	3.773
49.475	3.882
49.750	3.989
50.025	4.092
50.300	4.202

Hydrobrake estimated actual stage /

Values selected to create best estimate of actual stage discharge curve taking account of inlet structure low weir at 44.65mAOD, available SFGs & the design curve shape

FSR Upstream Stage (mAOD)	FSR Downstream Flow (m3/s)
44.65	0
45.506	1.042
46.5	2.30
46.9	2.55
48.1	2.48
50.3	3.07

Notes

1) There is another FSR outlet which is a separate compensation pipe flow to Evegate Mill. 1997 record dwgs and inspection reports show a 300mm dia orifice in a 600mm pipe, which has an upstream IL 46.6mAOD. The stage discharge curve with a max discharge of 0.34m3/s is shown on the chart titled "ALDINGTON FLOOD DETENTION STORAGE - Stage / Discharge and Stage / Storage Curves" with footnote "1966/X - Rofe, Kennard and Lapworth - Feb 1991".

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[REDACTED]
Aldington and Hothfield area and volume checks

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consulting

1 Aldington and Hothfield area and volume checks

This note compares the area and volume calculations for the Aldington and Hothfield flood storage areas from the current modelling work (2022) with earlier versions from 2010 (previous hydraulic model) and 2000 (flood routing calculations). The calculated data are also compared to the key reservoir data from the 2019 Section 19 reports.

1.1 Aldington Reservoir

The reservoir dimensions have been updated for the latest modelling and these data compared against previous sources. The hydraulic model reservoir unit uses a level-area relationship which has been derived from LIDAR and this has also been extended to a volume calculation.

The spillway level is at 50.2mAOD which is between the increments used in 2000 and 2010 calculations.

The area and volume values around this level are slightly variable but all reasonably close to each other, within 10%.

Table 1-1: Level, area and volume from 2000,2010 and 2022 for Aldington

Level (mOAD)	Area (m2)			Volume (m3)		
	2000	2010	2022	2000	2010	2022
51	998000	998000	947560	2022000	2063750	1954651
50.5	858000	858000	821439	1558000	1599750	1512826
50.2	-	-	739821	-	-	1278511
50	730000	730000	682984	1161000	1202750	1136204
49.5	579000	579000	569101	833750	875500	824369
49	463000	463000	454466	573250	615000	566865
48.5	368000	368000	341454	365500	407250	365527
48	265000	265000	259583	207250	249000	213284
47.5	159000	159000	164531	101250	143000	109043
47	94000	94000	87716	38000	79750	45662
46.5	29000	61500	37468	7250	40875	14435
46	0	29000	4051	0	18250	1562
45.5	0	17000	1438	0	6750	519
45	0	5000	435	0	1250	62
44.5	0	0	0	0	0	0

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 [REDACTED]
 Aldington and Hothfield area and volume checks



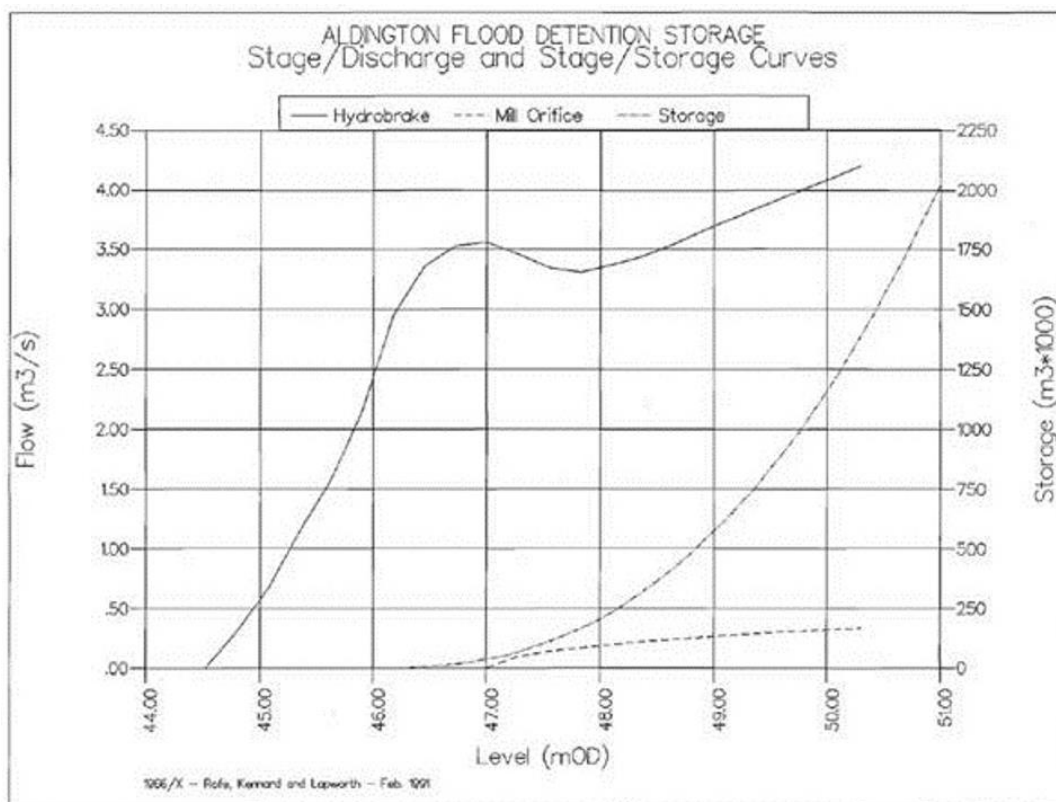
Table 1-2: Key reservoir dimensions from Aldington 2019 Section 10 report.

Feature	Unit	Dimension	Source
Reservoir capacity	Mm ³	1.3	Reservoir Record
Reservoir area at TWL	Mm ²	0.77	
Invert of upstream channel	m AOD	44.65	Drg RD10
Level of East Stour side weir	m AOD	47.5 to 47.4	Drg RD9
Level to which floodwater may be "temporarily stored", as certified in Final Certificate	m AOD	50.2	Final Certificate

The 2022 calculations of area and volume also agree closely with the data in the 2019 Section 10 report, comfortably with 5%.

Aldington (but not Hothfield) had a graph of the storage volume against the elevation supplied with the As-built records (date unknown). The volumes on this chart are also consistent with the more recently calculated values.

Figure 1-1: Aldington flood storage curve supplied with flood routing records



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[REDACTED]
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1.2 Hothfield Reservoir

The reservoir dimensions have been updated for the latest modelling and these data compared against previous sources. The hydraulic model reservoir unit uses a level-area relationship which has been derived from LIDAR and this has also been extended to a volume calculation.

The spillway level is at 48.5mOAD. The area and volume values at this level are slightly variable but all reasonably close to each other, within 10%.

Table 1-3: Level, area and volume from 2000,2010 and 2022 for Hothfield

Level (mOAD)	Area (m2)			Volume (m3)		
	2000	2010	2022	2000	2010	2022
49	1152000	1140000	1118000	2322500	2416158	2263074
48.5	1013000	994286	1000464	1781250	1882587	1733351
48	874000	873333	860695	1309500	1415682	1266387
47.5	733000	728000	708742	907750	1015349	874422
47	612000	580000	543452	571500	688349	560999
46.5	380000	426667	397341	323500	436682	326769
46	235000	340000	244006	169750	245016	164295
45.5	126000	320000	131345	79500	80016	73376
45	62000	10	64245	32500	13	25360
44.5	3000	10	17719	1500	8	7090
44	0	10	4357	0	3	253
43.5	0	0	0	0	0	0

Table 1-4: Key reservoir dimensions from Hothfield 2019 Section 10 report.

Feature	Value	Source / comment
Reservoir capacity	1.2Mm ³	Prescribed Form of Record
Reservoir area at TWL	1Mm ²	
Invert of upstream channel	44m AOD	Drawing RD5 & RD6
Invert of Pig Brook intake	44m AOD	
Low-level outlet	43.3m AOD	
Level to which floodwater may be "temporarily stored", as certified in Final Certificate	48.5m AOD	Final Certificate
Embankment crest	49.5m AOD	Drawing RD3
Downstream toe of embankment	44.4m AOD	Drawing RD4
Invert of downstream channel	43.57m AOD	

The 2022 calculation of area agrees closely with the data in the Section 10 report, within 1%.

The volume calculation for 2022 (and also 2000 and 2010) are all around 45% larger than the reservoir capacity given in the Section 10 report. Given the consistency of the

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Aldington and Hothfield area and volume
checks



calculated values from different data sources they would seem likely to be correct and the value in the Section 10 report the anomaly. There is no further information regarding how this value was derived.



Annex F

SLR Site Walkover Photos

Annex B: East Stour Hydraulic Modelling Report

Stonestreet Green Solar Farm

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SLR Site Walkover Photos








Photo 1: CH_Lane_P1 (25 July 2023)	Photo 2: CH_Lane_P2 (25 July 2023)	Photo 3: CULV1_D_P1 (25 July 2023)
		
Photo 4: CULV1_D_P2 (24 July 2023)	Photo 5: CULV2_D_P1 (25 July 2023)	Photo 6: DITCH_P1 (24 July 2023)
		



Photo 7: DITCH_P2 (24 July 2023)	Photo 8: ES_001_P1 (25 July 2023)	Photo 9: ES_002_P1 (25 July 2023)
		
Photo 10: ES_002_P2 (25 July 2023)	Photo 11: ES_002_P3 (25 July 2023)	Photo 12: ES_003_P1 (25 July 2023)
		



Photo 13: ES_003_P2 (25 July 2023)	Photo 14: ES_004_P1 (24 July 2023)	Photo 15: ES_004_P2 (24 July 2023)
		
Photo 16: ES_004_P3 (24 July 2023)	Photo 17: ES_005_P1 (24 July 2023)	Photo 18: ES_005_P2 (24 July 2023)
		



Photo 19: ES_005_P3 (24 July 2023)	Photo 20: ES_005_P4 (24 July 2023)	Photo 21: ES_006_P1 (24 July 2023)
		
Photo 22: ES_006_P2 (24 July 2023)	Photo 23: ES_006_P3 (24 July 2023)	Photo 24: ES_006_P4 (24 July 2023)
		



Photo 25: ES_006_P5 (24 July 2023)	Photo 26: ES_006-007_P1 (24 July 2023)	Photo 27: ES_006-007_P2 (24 July 2023)
		
Photo 28: ES_006-007_P3 (24 July 2023)	Photo 29: ES_007_P1 (24 July 2023)	Photo 30: ES_008_P1 (24 July 2023)
		



<p>Photo 31: ES_008_P2 (24 July 2023)</p>	<p>Photo 32: ES_008-009_P1 (24 July 2023)</p>	<p>Photo 33: ES_009_P1 (24 July 2023)</p>
		
<p>Photo 34: ES_009_P2 (24 July 2023)</p>	<p>Photo 35: ES_011_P1 (24 July 2023)</p>	<p>Photo 36: ES_011_P2 (24 July 2023)</p>
		









Photo 37: ES_011_P3 (24 July 2023)	Photo 38: ES_011_P4 (24 July 2023)	Photo 39: ES_011_P5 (24 July 2023)
		
Photo 40: ES_011_P6 (24 July 2023)	Photo 41: ES_011_P7 (24 July 2023)	Photo 42: ES_011_P8 (25 July 2023)
		



Photo 43: ES_012-013_P1 (24 July 2023)	Photo 44: ES_013_P1 (24 July 2023)	Photo 45: ES_013_P2 (24 July 2023)
		
Photo 46: ES_013_P3 (24 July 2023)	Photo 47: ES_013_P4 (24 July 2023)	Photo 48: ES_013-014_P1 (24 July 2023)
		



Photo 49: ES_013-014_P2 (24 July 2023)	Photo 50: ES_014_P1 (24 July 2023)	Photo 51: ES_014_P2 (24 July 2023)
		
Photo 52: ES_014_P3 (24 July 2023)	Photo 53: FSA_Embankment_P1 (24 July 2023)	Photo 54: FSA_Embankment_P2 (24 July 2023)
		






Photo 55: FSA_Embankment_P3 (24 July 2023)	Photo 56: FSA_Embankment_P4 (24 July 2023)	Photo 57: FSA_Embankment_P5 (24 July 2023)
		
Photo 58: FSA_South_P1 (24 July 2023)	Photo 59: FSA_South_P2 (24 July 2023)	Photo 60: FSA_South_P3 (24 July 2023)
		



Photo 61: MILL_003_P1 (24 July 2023)	Photo 62: MILL_003_P2 (24 July 2023)	Photo 63: MILL_003_P3 (24 July 2023)
		
Photo 64: MILL_003_P4 (24 July 2023)	Photo 65: MILL_003_P5 (24 July 2023)	Photo 66: MILL_003_P6 (24 July 2023)
		



Photo 67: MILL_004_P1 (24 July 2023)	Photo 68: MILL_004_P2 (24 July 2023)	Photo 69: TRIB1_001_P1 (25 July 2023)
		
Photo 70: TRIB1_001_P2 (25 July 2023)	Photo 71: TRIB1_001-002_P1 (25 July 2023)	Photo 72: TRIB1_001-002_P2 (25 July 2023)
		



Photo 73: TRIB1_001-002_P3 (25 July 2023)	Photo 74: TRIB1_002_P1 (25 July 2023)	Photo 75: TRIB1_002_P2 (25 July 2023)
		
Photo 76: TRIB1_002_P3 (25 July 2023)	Photo 77: TRIB1_002_P4 (25 July 2023)	Photo 78: TRIB1_002_P5 (25 July 2023)
		



Photo 79: TRIB1_002_P6 (25 July 2023)	Photo 80: TRIB1_003_P1 (25 July 2023)	Photo 81: TRIB1_003_P2 (25 July 2023)
		
Photo 82: TRIB1_003_P3 (25 July 2023)	Photo 83: TRIB1_004_P1 (25 July 2023)	Photo 84: TRIB1_004_P2 (25 July 2023)
		



Photo 85: TRIB1_004_P3 (25 July 2023)	Photo 86: TRIB1_004-005_P1 (25 July 2023)	Photo 87: TRIB1_005_P1 (24 July 2023)
		
Photo 88: TRIB1_005_P2 (24 July 2023)	Photo 89: TRIB1_005_P3 (25 July 2023)	Photo 90: TRIB1_006_P1 (24 July 2023)
		



Photo 91: TRIB1_Pond_P1 (25 July 2023)



Photo 92: TRIB1_Pond_P2 (25 July 2023)



Photo 93: TRIB2_001_P1 (25 July 2023)

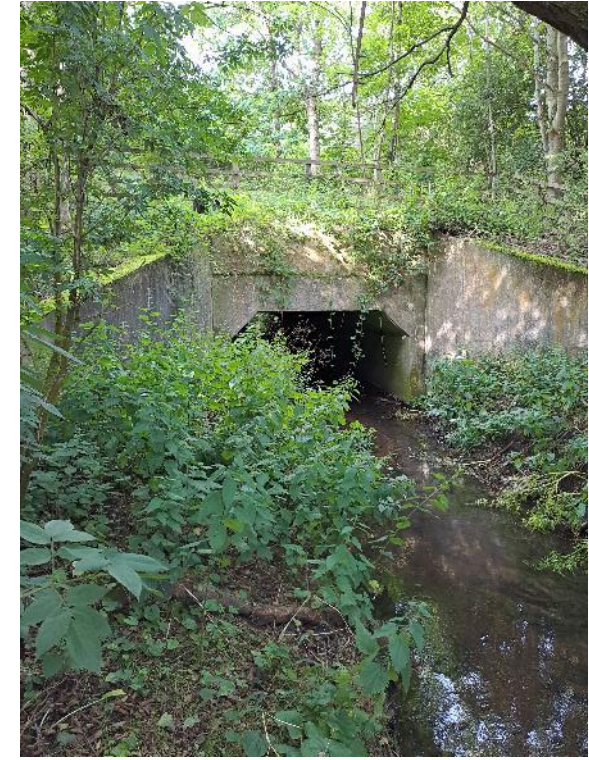


Photo 94: TRIB2_001_P2 (25 July 2023)



Photo 95: TRIB2_002b_P1 (25 July 2023)



Photo 96: TRIB3_001_P1 (24 July 2023)



Photo 97: TRIB3_001_P2 (24 July 2023)	Photo 98: TRIB3_001_P3 (24 July 2023)	Photo 99: TRIB3_002_P1 (24 July 2023)
		
Photo 100: TRIB3_002_P2 (24 July 2023)	Photo 101: TRIB3_002_P3 (24 July 2023)	Photo 102: TRIB3_003_P1 (24 July 2023)
		




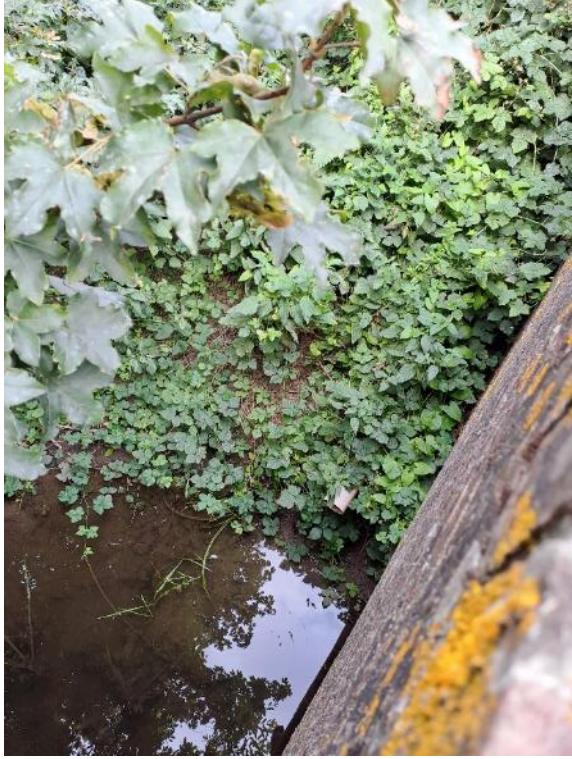




Photo 103: TRIB3_004_P1 (24 July 2023)	Photo 104: TRIB3_004_P2 (24 July 2023)	Photo 105: TRIB3_004_P3 (24 July 2023)
		
Photo 106: TRIB3_004_P4 (24 July 2023)	Photo 107: TRIB3_004_P5 (24 July 2023)	Photo 108: TRIB3_004_P6 (24 July 2023)
		



Photo 109: TRIB3_004_P7 (24 July 2023)	Photo 110: TRIB3_004_P8 (24 July 2023)	Photo 111: TRIB3_005_P1 (24 July 2023)
		
Photo 112: TRIB3_005_P2 (24 July 2023)	Photo 113: TRIB3_005_P3 (24 July 2023)	Photo 114: TRIB3_005_P4 (24 July 2023)
		



Photo 115: TRIB3_005_P5 (24 July 2023)		
		



Annex G

2D Bridge Loss Calculation Sheet

Annex B: East Stour Hydraulic Modelling Report

Stonestreet Green Solar Farm

EPL 001 Limited



From TuFLOW Guidance

Deck Height to Thickness Ratio	Peak Form Loss Coefficient	hB/T
Scenario A (hB/T) = 2	0.42	2
Scenario B (hB/T) = 4	0.28	4
Scenario C (hB/T) = 6	0.2	6

Scenario	Gradient [FLC/(hB/T)]
hB/T <= 4	-0.07 <4
hB/T > 4	-0.04 >4

Inputs

Parameter	Level (m aOD)
Bed Level	46.49
Soffit Level	47.84
Deck Level	48.13
Rail Level	48.13
Rail Blockage Ratio	10%

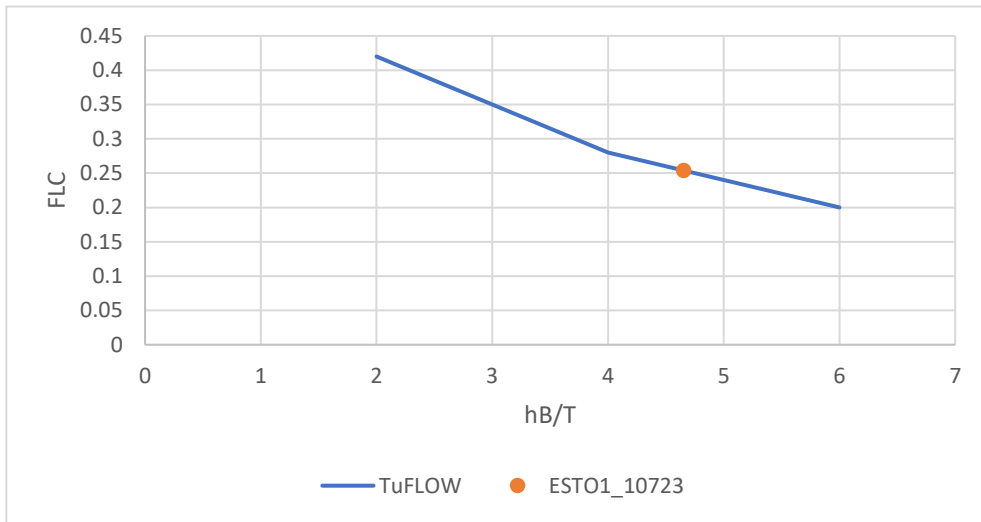
Calculations

Parameter	Value
hB (height of opening)	1.35
T	0.29
hB/T	4.655172414 >4
FLC	0.253793103 -0.04

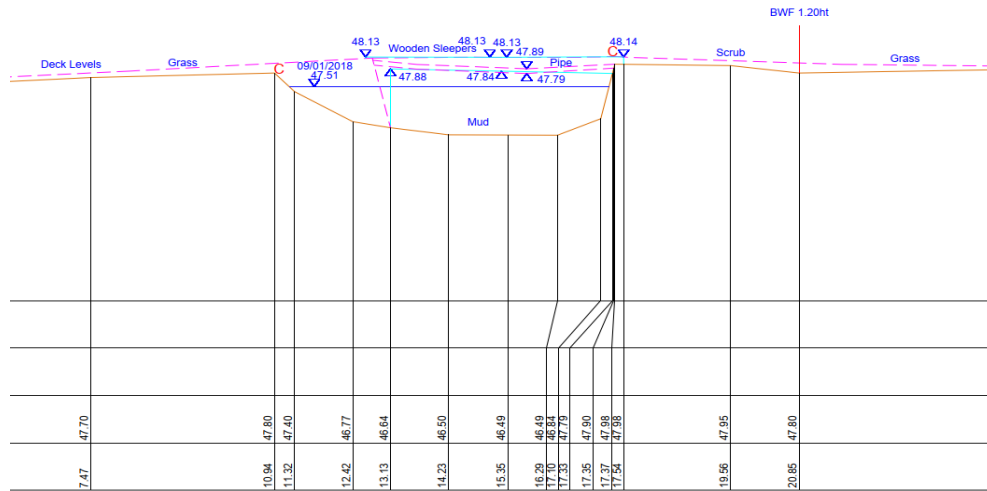
Outputs

Attribute	Value	
ID	ESTO1_10723	Enter unique name
Option	-	Not used (leave blank)
Pier_pBlockage	0.000	0 for no piers, 5 for 5% blockage etc.
Pier_FLC	0.000	0 if no pier. If piers use "Hydraulics of Bridge Waterways (Bradly, 1978)" to calculate loss coefficient
Deck_Soffit	47.840	Soffit level (top of opening)
Deck_Depth	0.290	Thickness of Bridge Deck (vertical)
Deck_Width	3.820	Deck width (In direction of flow)
Deck_pBlockage	100	100 assumes solid deck, update if required
Rail_Depth	0.000	Thickness of Rail (vertical)
Rail_pBlockage	10	% Blockage of rail
SuperS_FLC	0.254	Deck form loss coefficient
SuperS_lpf	1.6	Default value

ESTO01_10723 Bridge Loss Calculation Sheet



2m



From TuFLOW Guidance

Deck Height to Thickness Ratio	Peak Form Loss Coefficient	hB/T
Scenario A (hB/T) = 2	0.42	2
Scenario B (hB/T) = 4	0.28	4
Scenario C (hB/T) = 6	0.2	6

Scenario	Gradient [FLC/(hB/T)]
hB/T <= 4	-0.07 <4
hB/T > 4	-0.04 >4

Inputs

Parameter	Level (m aOD)
Bed Level	46.1
Soffit Level	48.63
Deck Level	49.07
Rail Level	50.25
Rail Blockage Ratio	10%

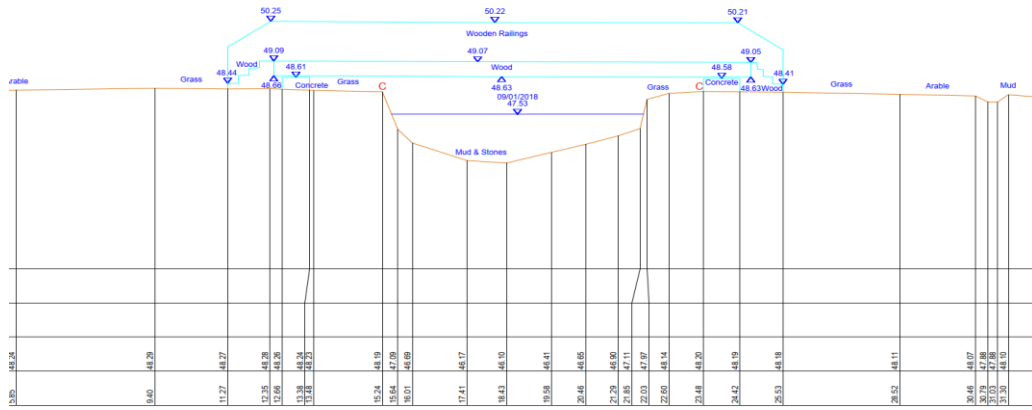
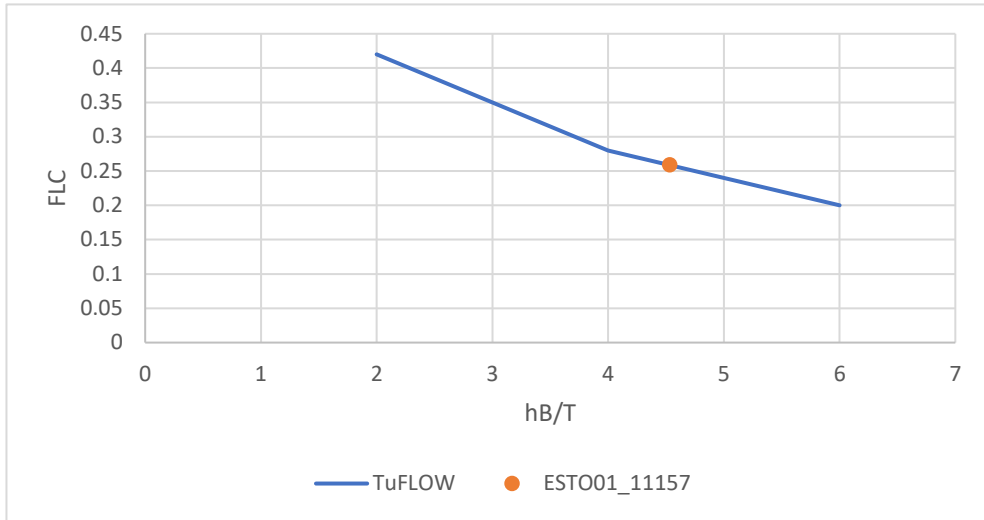
Calculations

Parameter	Value
hB (height of opening)	2.53
T	0.558
hB/T	4.534050179 >4
FLC	0.258637993 -0.04

Outputs

Attribute	Value	
ID	ESTO01_11157	Enter unique name
Option	-	Not used (leave blank)
Pier_pBlockage	0.000	0 for no piers, 5 for 5% blockage etc.
Pier_FLC	0.000	0 if no pier. If piers use "Hydraulics of Bridge Waterways (Bradly, 1978)" to calculate loss coefficient
Deck_Soffit	48.630	Soffit level (top of opening)
Deck_Depth	0.440	Thickness of Bridge Deck (vertical)
Deck_Width	1.140	Deck width (In direction of flow)
Deck_pBlockage	100	100 assumes solid deck, update if required
Rail_Depth	1.180	Thickness of Rail (vertical)
Rail_pBlockage	10	% Blockage of rail
SuperS_FLC	0.259	Deck form loss coefficient
SuperS_lpf	1.6	Default value

ESTO01_11157 Bridge Loss Calculation Sheet



From TuFLOW Guidance

Deck Height to Thickness Ratio	Peak Form Loss Coefficient	hB/T
Scenario A (hB/T) = 2	0.42	2
Scenario B (hB/T) = 4	0.28	4
Scenario C (hB/T) = 6	0.2	6

Scenario	Gradient [FLC/(hB/T)]
hB/T <= 4	-0.07 <4
hB/T > 4	-0.04 >4

Inputs

Parameter	Level (m aOD)
Bed Level	46.67
Soffit Level	48.74
Deck Level	49.09
Rail Level	50.06
Rail Blockage Ratio	10%

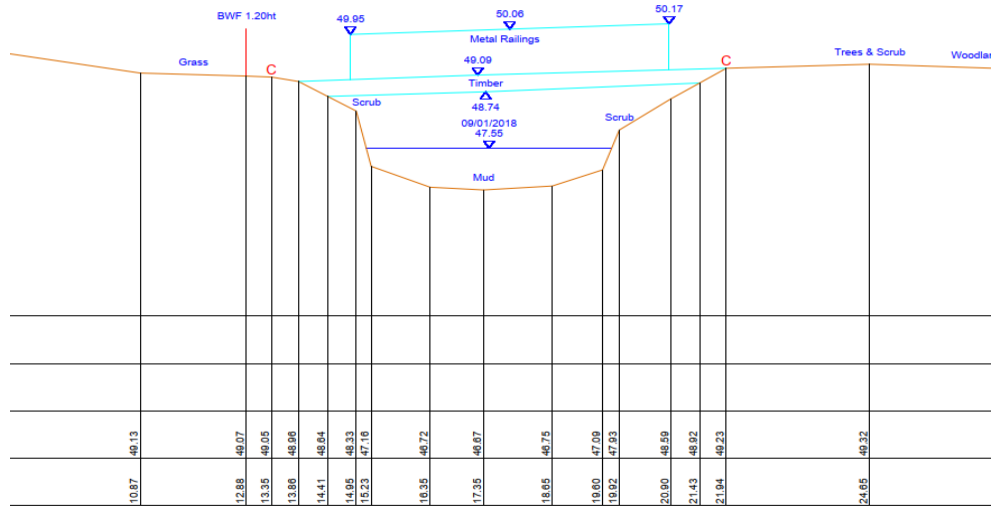
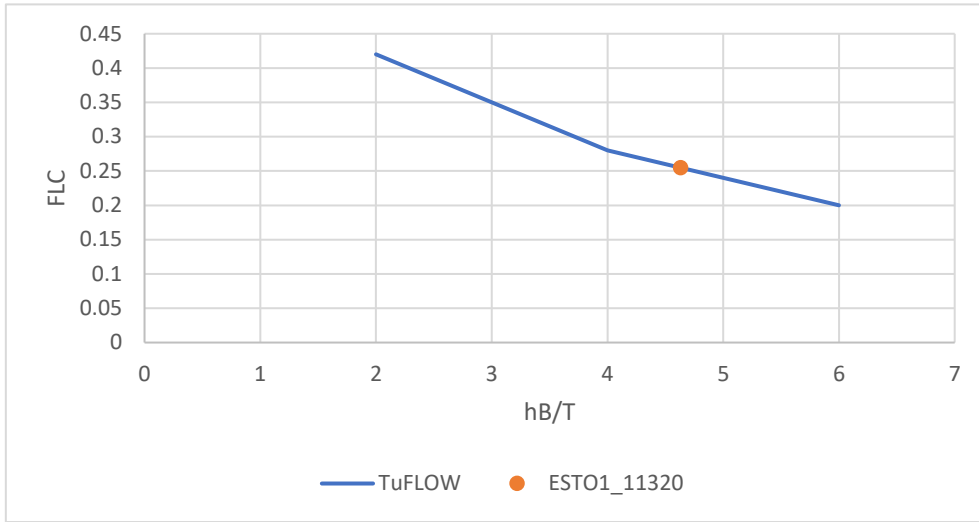
Calculations

Parameter	Value
hB (height of opening)	2.07
T	0.447
hB/T	4.630872483 >4
FLC	0.254765101 -0.04

Outputs

Attribute	Value	
ID	ESTO01_11320	Enter unique name
Option	-	Not used (leave blank)
Pier_pBlockage	0.000	0 for no piers, 5 for 5% blockage etc.
Pier_FLC	0.000	0 if no pier. If piers use "Hydraulics of Bridge Waterways (Bradly, 1978)" to calculate loss coefficient
Deck_Soffit	48.740	Soffit level (top of opening)
Deck_Depth	0.350	Thickness of Bridge Deck (vertical)
Deck_Width	0.690	Deck width (In direction of flow)
Deck_pBlockage	100	100 assumes solid deck, update if required
Rail_Depth	0.970	Thickness of Rail (vertical)
Rail_pBlockage	10	% Blockage of rail
SuperS_FLC	0.255	Deck form loss coefficient
SuperS_lpf	1.6	Default value

ESTO1_11320 Bridge Loss Calculation Sheet



From TuFLOW Guidance

Deck Height to Thickness Ratio	Peak Form Loss Coefficient	hB/T
Scenario A (hB/T) = 2	0.42	2
Scenario B (hB/T) = 4	0.28	4
Scenario C (hB/T) = 6	0.2	6

Scenario	Gradient [FLC/(hB/T)]
hB/T <= 4	-0.07 <4
hB/T > 4	-0.04 >4

Inputs

Parameter	Level (m aOD)
Bed Level	49.59
Soffit Level	51.84
Deck Level	52.22
Rail Level	53.42
Rail Blockage Ratio	10%

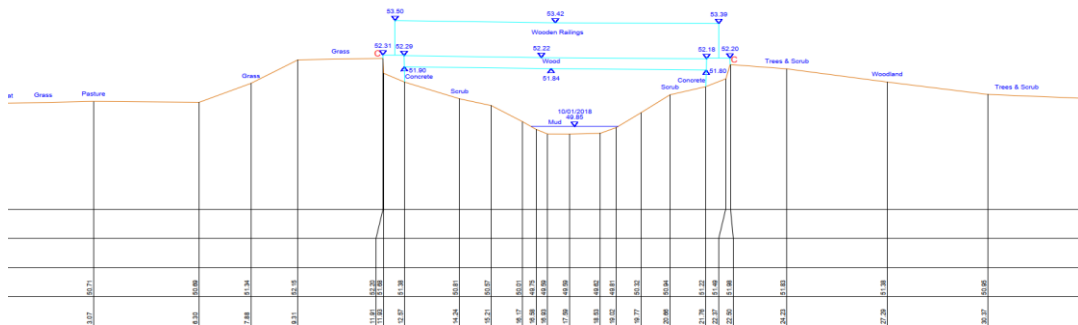
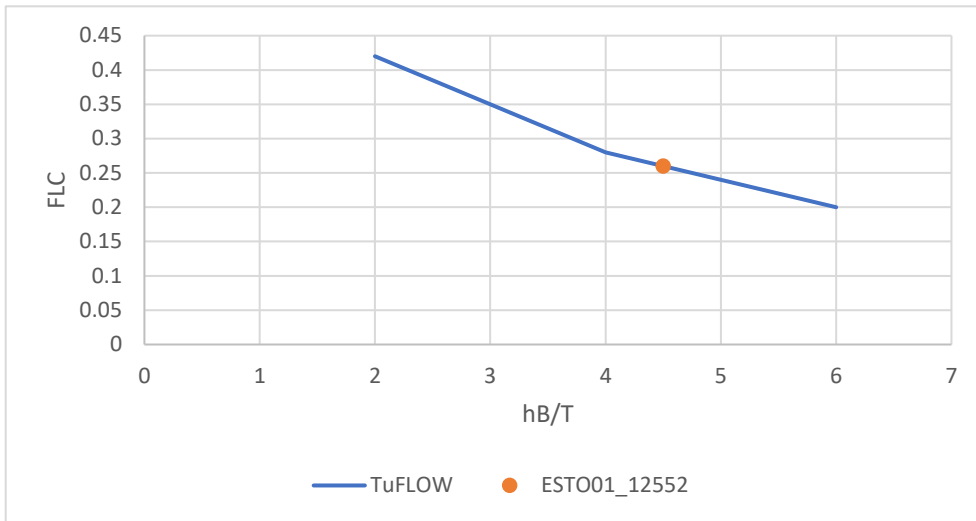
Calculations

Parameter	Value
hB (height of opening)	2.25
T	0.5
hB/T	4.5 >4
FLC	0.26 -0.04

Outputs

Attribute	Value	
ID	ESTO01_12552	Enter unique name
Option	-	Not used (leave blank)
Pier_pBlockage	0.000	0 for no piers, 5 for 5% blockage etc.
Pier_FLC	0.000	0 if no pier. If piers use "Hydraulics of Bridge Waterways (Bradly, 1978)" to calculate loss coefficient
Deck_Soffit	51.840	Soffit level (top of opening)
Deck_Depth	0.380	Thickness of Bridge Deck (vertical)
Deck_Width	1.840	Deck width (In direction of flow)
Deck_pBlockage	100	100 assumes solid deck, update if required
Rail_Depth	1.200	Thickness of Rail (vertical)
Rail_pBlockage	10	% Blockage of rail
SuperS_FLC	0.260	Deck form loss coefficient
SuperS_lpf	1.6	Default value

ESTO01_12552
Bridge Loss Calculation Sheet



STO01 12552
18504.16mE 138034.25mN Brg 335
notbridge
Innel Length = 1.84m

From TuFLOW Guidance

Deck Height to Thickness Ratio	Peak Form Loss Coefficient	hB/T
Scenario A (hB/T) = 2	0.42	2
Scenario B (hB/T) = 4	0.28	4
Scenario C (hB/T) = 6	0.2	6

Scenario	Gradient [FLC/(hB/T)]
hB/T <= 4	-0.07 <4
hB/T > 4	-0.04 >4

Inputs

Parameter	Level (m aOD)
Bed Level	49.87
Soffit Level	51.87
Deck Level	52.26
Rail Level	53.47
Rail Blockage Ratio	10%

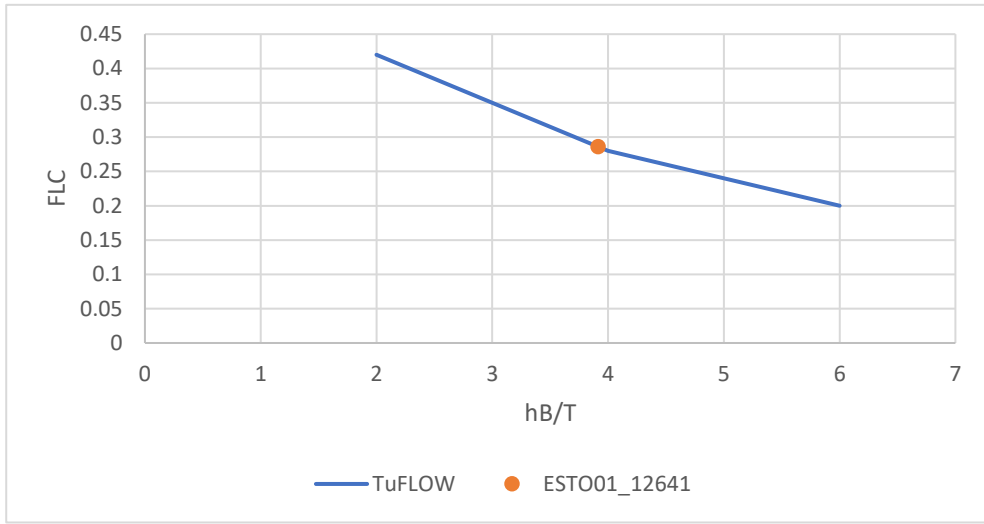
Calculations

Parameter	Value
hB (height of opening)	2
T	0.511
hB/T	3.913894325 <4
FLC	0.286027397 -0.07

Outputs

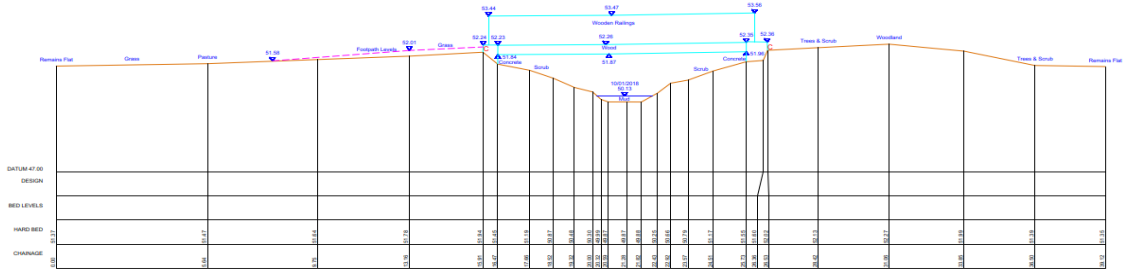
Attribute	Value	
ID	ESTO01_12641	Enter unique name
Option	-	Not used (leave blank)
Pier_pBlockage	0.000	0 for no piers, 5 for 5% blockage etc.
Pier_FLC	0.000	0 if no pier. If piers use "Hydraulics of Bridge Waterways (Bradly, 1978)" to calculate loss coefficient
Deck_Soffit	51.870	Soffit level (top of opening)
Deck_Depth	0.390	Thickness of Bridge Deck (vertical)
Deck_Width	1.780	Deck width (In direction of flow)
Deck_pBlockage	100	100 assumes solid deck, update if required
Rail_Depth	1.210	Thickness of Rail (vertical)
Rail_pBlockage	10	% Blockage of rail
SuperS_FLC	0.286	Deck form loss coefficient
SuperS_lpf	1.6	Default value

ESTO01_12641 Bridge Loss Calculation Sheet



608550.36mE 138025.77mN Brg 80

608550.36mE 138025.77mN Brg 80
Footbridge
Tunnel Length = 1.84m



ESTO01_12641
608550.36mE 138025.77mN Brg 80
Footbridge
Tunnel Length = 1.78m

From TuFLOW Guidance

Deck Height to Thickness Ratio	Peak Form Loss Coefficient	hB/T
Scenario A (hB/T) = 2	0.42	2
Scenario B (hB/T) = 4	0.28	4
Scenario C (hB/T) = 6	0.2	6

Scenario	Gradient [FLC/(hB/T)]
hB/T <= 4	-0.07 <4
hB/T > 4	-0.04 >4

Inputs

Parameter	Level (m aOD)
Bed Level	50.28
Soffit Level	52.13
Deck Level	52.5
Rail Level	53.44
Rail Blockage Ratio	10%

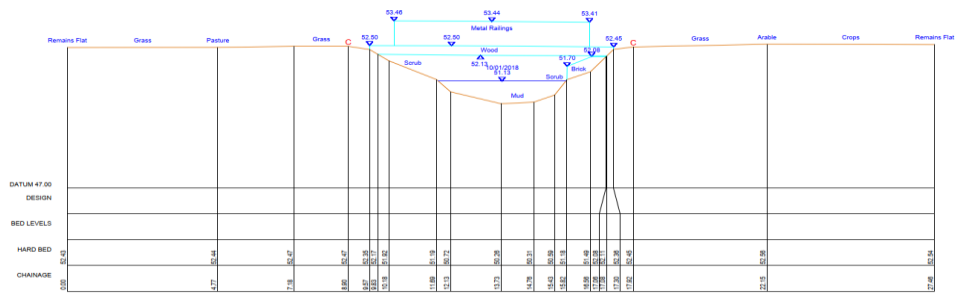
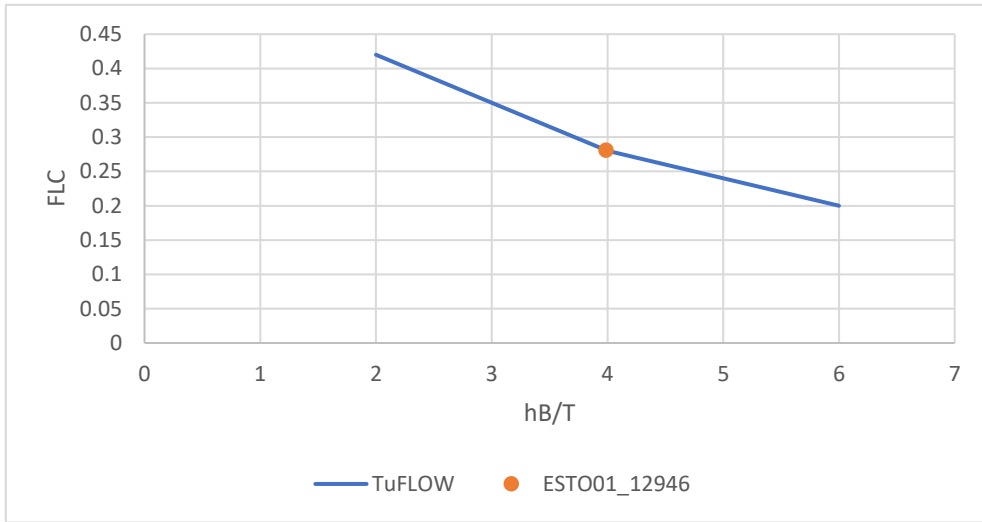
Calculations

Parameter	Value
hB (height of opening)	1.85
T	0.464
hB/T	3.987068966 <4
FLC	0.280905172 -0.07

Outputs

Attribute	Value	
ID	ESTO01_12946	Enter unique name
Option	-	Not used (leave blank)
Pier_pBlockage	0.000	0 for no piers, 5 for 5% blockage etc.
Pier_FLC	0.000	0 if no pier. If piers use "Hydraulics of Bridge Waterways (Bradly, 1978)" to calculate loss coefficient
Deck_Soffit	52.130	Soffit level (top of opening)
Deck_Depth	0.370	Thickness of Bridge Deck (vertical)
Deck_Width	0.810	Deck width (In direction of flow)
Deck_pBlockage	100	100 assumes solid deck, update if required
Rail_Depth	0.940	Thickness of Rail (vertical)
Rail_pBlockage	10	% Blockage of rail
SuperS_FLC	0.281	Deck form loss coefficient
SuperS_lpf	1.6	Default value

ESTO01_12946 Bridge Loss Calculation Sheet



ESTO01_12946
 608809.06mE 137913.52mN Brg 33
 Footbridge
 Tunnel Length = 0.81m